

A. GENERAL STRUCTURAL REQUIREMENTS

- ALL METHODS OF CONSTRUCTION, DETAILS, NOTES, ETC., INDICATED ON THE DRAWINGS ARE TO BE CONSIDERED TYPICAL FOR ALL SIMILAR CONDITIONS.
- CONSTRUCTION SHALL BE MADE FROM APPROVED SHOP DRAWINGS ONLY.
- ANY DISCREPANCIES ON THESE PLANS WITH REGARD TO DIMENSIONS OR CONDITIONS SHALL BE BROUGHT TO THE ATTENTION OF THE ENGINEER BEFORE PROCEEDING WITH THE AFFECTED PORTION OF WORK.
- ALL APPLICABLE FEDERAL, STATE, AND MUNICIPAL REGULATIONS SHALL BE FOLLOWED, INCLUDING THE FEDERAL DEPARTMENT OF LABOR OCCUPATIONAL SAFETY AND HEALTH ACT AND THE RHODE ISLAND STATE BUILDING CODE.
- THE LATEST EDITION OF THE FOLLOWING LISTED CODES SHALL APPLY. IN CASE OF CONFLICT, THE MORE RIGID REQUIREMENTS AND CODES SHALL GOVERN.
 - RHODE ISLAND STATE BUILDING CODE (STATE CODE): INTERNATIONAL BUILDING CODE, 2018 EDITION AND ITS APPLICABLE REFERENCED STANDARDS.
 - AMERICAN INSTITUTE OF STEEL CONSTRUCTION SPECIFICATIONS AND ITS CODE OF STANDARD PRACTICE (AISC).
 - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE, ACI 318.
 - AMERICAN CONCRETE INSTITUTE BUILDING CODE REQUIREMENTS FOR CONCRETE MASONRY STRUCTURES, ACI 530 AND ACI 530.1.
- THE DESIGN LOADS ARE RESISTED BY THE COMPLETED STRUCTURE ACTING AS A UNIT. THE CONTRACTOR SHALL DESIGN AND PROVIDE ANY AND ALL TEMPORARY BRACING, SHORING, OR ADDITIONAL REINFORCEMENT NECESSARY TO RESIST LOADS IMPOSED ON ANY PORTION OF THE STRUCTURE THROUGHOUT ALL STAGES OF CONSTRUCTION. THE SHORING SHALL BE DESIGNED TO RESIST ALL DEAD LOADS AND ANY APPLICABLE CONSTRUCTION LOADS.
- ALL SHORING DESIGNS AND PLANS SHALL BE STAMPED BY A RHODE ISLAND REGISTERED PROFESSIONAL ENGINEER.
- COLUMN ANCHOR RODS ARE NOT DESIGNED TO TEMPORARILY CANTILEVER FROM THE FOUNDATIONS. ALL STEEL MUST BE TEMPORARILY BRACED AND GUYED UNTIL THE BUILDING IS LATERALLY STABLE.
- NOTES AND TYPICAL DETAILS APPLY TO ALL STRUCTURAL WORK UNLESS OTHERWISE NOTED. FOR CONDITIONS NOT SPECIFICALLY SHOWN PROVIDE DETAILS OF SIMILAR NATURE. VERIFY APPLICABILITY BY SUBMITTING SHOP DRAWINGS FOR REVIEW.
- PLANS SHALL NOT BE SCALED FOR DIMENSIONS.
- ARCHITECTURAL AND MEP DRAWINGS MUST BE USED IN CONJUNCTION WITH THE STRUCTURAL DRAWINGS DURING ALL PHASES OF CONSTRUCTION.

B. DESIGN LOADS

- GENERAL
 - BUILDING RISK CATEGORY III
 - FROST DEPTH 4'-0"
- LIVE LOADS
 - CLASSROOMS/OFFICES 65 psf (50 psf + 15 psf PARTITIONS)
 - CORRIDORS ABOVE FIRST FLOOR 80 psf
 - FIRST FLOOR CORRIDORS, FLEXIBLE SPACES, OPEN CLASSROOMS, LABS, MEDIA CENTER, AUDITORIUM SEATING, ROOF TERRACE, STAIRS 100 psf
 - AUDITORIUM STAGE, FITNESS CENTER, MECHANICAL ROOMS*, CULINARY AREAS, STORAGE 150 psf
 - SLAB-ON-GRADE 250 psf

* OR ACTUAL WEIGHT OF EQUIPMENT, WHICHEVER IS GREATER
- ROOF LIVE LOADS (SNOW):
 - IMPORTANCE FACTOR 1.1
 - GROUND SNOW LOAD (Pg) 30 psf
 - FLAT ROOF SNOW LOAD (Pf) 30 psf (MINIMUM PER CODE)
 - EXPOSURE FACTOR (Ce) 1.0
 - THERMAL FACTOR (Ct) 1.0
- WIND LOADS
 - ULTIMATE WIND DESIGN SPEED (Vult) 133 mph
 - NOMINAL DESIGN WIND SPEED (Vnd) 103 mph
 - EXPOSURE CATEGORY C
 - ENCLOSED BUILDING (Gcp) ±0.18
 - FOR COMPONENTS & CLADDING WIND PRESSURES, SEE TABLE THIS SHEET
- EARTHQUAKE LOADS
 - IMPORTANCE FACTOR 1.25
 - MAPPED SPECTRAL RESPONSE ACCELERATIONS (Ss, S1) 0.178, 0.062
 - SITE CLASS E
 - DESIGN SPECTRAL RESPONSE COEFFICIENTS (Sds, Sd1) 0.285, 0.174
 - SEISMIC DESIGN CATEGORY C
 - DESIGN PROCEDURE EQUIVALENT LATERAL FORCE PROCEDURE
 - LATERAL FORCE RESISTING SYSTEM STEEL NOT SPECIFICALLY DETAILED FOR SEISMIC (R=3.0)
- OTHER LOADS
 - ADDITIONAL ROOF DEAD LOAD FOR PV SOLAR ALLOWANCE 10 psf
 - ROOF PAVERS AT THIRD FLOOR ROOF GARDEN 25 psf

C. FOUNDATIONS

- FOUNDATIONS HAVE BEEN DESIGNED BASED UPON AN ALLOWABLE BEARING PRESSURE OF 3.0 KSF FOLLOWING GROUND IMPROVEMENTS. REFER TO GEOTECHNICAL REPORT DATED AUGUST 4, 2023, PREPARED BY LAHAF GEOTECHNICAL CONSULTING, INC.
- NO FOOTING OR SLAB SHALL BE PLACED ON FROZEN SOIL OR IN WATER.
- FOOTINGS AND FOUNDATION WALLS SHALL BEAR A MINIMUM OF 4'-0" BELOW FINISH GRADE, UNLESS NOTED OTHERWISE.
- FOOTINGS AND SLABS SHALL BEAR ON GROUND IMPROVED WITH AGGREGATE PIERS AND/OR RIGID INCLUSIONS. THE GROUND IMPROVEMENTS SHALL BE INSTALLED WITHIN THE ENTIRE BUILDING FOOTPRINT AND BENEATH SITE RETAINING WALLS. THE GROUND IMPROVEMENTS SHALL EXTEND LATERALLY TO WITHIN THE ZONE OF INFLUENCE OF FOUNDATIONS. SUBGRADE PREPARATION BELOW ALL FOOTINGS, FOUNDATION WALLS, AND SLABS SHALL BE VERIFIED BY THE GEOTECHNICAL ENGINEER.
- ALL SURFACE WATER SHALL BE DIVERTED AWAY FROM EXCAVATION BY THE CONTRACTOR. CONTRACTOR SHALL MAINTAIN CONTINUOUS CONTROL OF SURFACE AND SUBSURFACE WATER DURING CONSTRUCTION SO THAT WORK IS DONE UNDER DRY CONDITIONS.
- SHORING AND BRACING FOR THE LATERAL SUPPORT OF EXCAVATION SHALL REMAIN IN PLACE UNTIL ALL PERMANENT STRUCTURAL SYSTEMS ARE COMPLETE.
- PERCENT COMPACTION IS DEFINED AS THE RATIO OF THE FIELD MEASURED DRY DENSITY, DETERMINED BY ASTM D-6938, COMPARED TO THE MAXIMUM DRY DENSITY, DETERMINED BY ASTM D-1557 (MODIFIED PROCTOR).
- COMPACT BACKFILL UNDER ALL FOOTINGS, FOUNDATION WALLS, AND SLABS ON GRADE TO A MINIMUM OF 95 PERCENT OF MAXIMUM DRY DENSITY AS DETERMINED BY ASTM D-1557, UNLESS OTHERWISE INDICATED OR SPECIFIED. FREQUENCY OF COMPACTION TESTING SHALL BE INDICATED BY THE GEOTECHNICAL ENGINEER AND PROJECT SPECIFICATIONS.
- DO NOT BACKFILL AGAINST CONCRETE WALLS UNTIL WALLS AND SUPPORTING SLABS HAVE REACHED THE 7-DAY SPECIFIED DESIGN STRENGTH. DO NOT BACKFILL AGAINST CONCRETE RETAINING WALLS (AND FOUNDATION WALLS ACTING AS RETAINING WALLS) UNTIL CONCRETE HAS REACHED 28-DAY SPECIFIED DESIGN STRENGTH.
- BACKFILL SHALL BE PLACED AND COMPACTED SIMULTANEOUSLY ON BOTH SIDES OF FOUNDATION WALLS.
- ANY BOULDER, LEDGE, OR ANY OTHER OBSTRUCTION LOCATED WITHIN THE BUILDING AREA SHALL BE REMOVED TO A DEPTH OF AT LEAST 6" (MIN.) BELOW FOOTINGS AND AT LEAST 24" (MIN.) BELOW SLABS. VOIDS SHALL BE BACKFILLED WITH STRUCTURAL FILL APPROVED BY THE GEOTECHNICAL ENGINEER.
- NO UTILITIES SHALL BE LOCATED WITHIN A REFERENCE LINE DRAWN OUTWARD AND DOWNWARD ON A 1.5H:1V SLOPE FROM THE BOTTOM OF FOOTINGS. NEW UTILITIES SHALL BE LOCATED OUTSIDE OF THIS ZONE. EXISTING UTILITIES SHALL BE REMOVED AND/OR RELOCATED SO THEY ARE OUTSIDE OF THIS ZONE. ALL EXCAVATIONS SHALL BE BACKFILLED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND GEOTECHNICAL ENGINEER'S RECOMMENDATIONS.
- SLAB PROOF-COMPACTION AND SUBGRADE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND VERIFIED BY THE GEOTECHNICAL ENGINEER. SLABS-ON-GRADE SHALL BE SUPPORTED ON A MINIMUM OF 12 INCHES OF STRUCTURAL FILL PLACED ON GROUND IMPROVED WITH AGGREGATE PIERS AND/OR RIGID INCLUSIONS. PROVIDE A 15 MIL VAPOR BARRIER UNDER SLABS-ON-GRADE IN ACCORDANCE WITH THE CONSTRUCTION DOCUMENTS AND PROJECT SPECIFICATIONS.
- FOUNDATION/FOOTING PROOF-COMPACTION AND SUBGRADE PREPARATION SHALL BE PERFORMED IN ACCORDANCE WITH THE PROJECT SPECIFICATIONS AND VERIFIED BY THE GEOTECHNICAL ENGINEER. FOOTINGS/FOUNDATIONS SHALL BE SUPPORTED ON A MINIMUM OF 6 INCHES OF STRUCTURAL FILL PLACED DIRECTLY OVER NATURAL SAND AND GRAVEL OR ON GROUND IMPROVED WITH AGGREGATE PIERS OR RIGID INCLUSIONS PER THE GEOTECHNICAL REPORT.
- COORDINATE PIPING PASSING THROUGH EXTERIOR FOUNDATION WALLS. PIPING SHALL NOT PASS THROUGH OR BELOW WALL FOOTING. FOOTING SHALL STEP AS REQUIRED TO ALLOW PIPING TO PASS THROUGH THE WALL.
- FOOTINGS SHALL BE STEPPED AT A MAXIMUM SLOPE OF 2 HORIZONTAL TO 1 VERTICAL, UNLESS NOTED OTHERWISE. (SEE TYPICAL DETAILS).
- ANY PLAN FOR GROUND IMPROVEMENT SHALL BE PREPARED BY THE CONTRACTOR IN ACCORDANCE WITH THE SPECIFICATIONS AND SUBMITTED FOR REVIEW BY THE GEOTECHNICAL ENGINEER. NO GROUND IMPROVEMENT SHALL BEGIN ON THE SITE UNTIL THE SUBMITTAL HAS BEEN REVIEWED AND APPROVED BY THE GEOTECHNICAL ENGINEER. THE PROPOSED GROUND IMPROVEMENT SYSTEM SHALL IN NO WAY CHANGE OR IMPACT THE DESIGN AND/OR PERFORMANCE OF THE FOUNDATION AND THE NEW CONSTRUCTION.

D. CAST-IN-PLACE CONCRETE

- CONCRETE WORK SHALL CONFORM TO THE LATEST EDITIONS OF THE "BUILDING CODE REQUIREMENTS FOR REINFORCED CONCRETE" (ACI 318) AND STATE CODE.
- CONCRETE SHALL BE PROPORTIONED, MIXED, AND PLACED UNDER THE SUPERVISION OF THE APPROVED TESTING AGENCY.
- UNLESS NOTED OTHERWISE, CONCRETE SHALL BE NORMAL WEIGHT, WITH TYPE II CEMENT, AND SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH AT 28 DAYS AS FOLLOWS:
 - 4,000 PSI 3/4" AGGREGATE - TYPICAL, U.N.O.
 - 4,000 PSI 3/8" AGGREGATE - CONCRETE FILL AT METAL PAN STAIRS
 - 3,000 PSI 3/4" AGGREGATE - EXTERIOR ROOFTOP EQUIPMENT PADS (SEE TYPICAL DETAIL ON [50.23](#))
- CONCRETE SPECIFIED AS LIGHTWEIGHT (SLABS-ON-METAL DECK WHERE INDICATED ON THE DRAWINGS) SHALL USE TYPE II CEMENT AND 3/4" LIGHTWEIGHT AGGREGATE CONFORMING TO ASTM C330. LIGHTWEIGHT CONCRETE SHALL HAVE A MINIMUM COMPRESSIVE STRENGTH OF 4,000 PSI AT 28 DAYS. THE CALCULATED EQUILIBRIUM UNIT WEIGHT SHALL BE 115 PCF +/- 3 PCF.
- PROVIDE CONCRETE MOISTURE VAPOR REDUCTION ADMIXTURE (MVRA) AT ALL INTERIOR SLABS-ON-GRADE. REFER TO SPECIFICATIONS SECTION 03 30 00 FOR REQUIREMENTS. DO NOT PROCEED WITH CONCRETE PLACEMENTS CONTAINING "MVRA" WITHOUT "MVRA" REPRESENTATIVE PRESENT.
- ALL CONCRETE, UNLESS NOTED OTHERWISE, SHALL BE AIR-ENTRAINED WITH AN AIR CONTENT OF 6%± 1%. INTERIOR SLABS ON GRADE AND INTERIOR SLAB-ON-METAL DECK THAT UTILIZES NORMAL WEIGHT CONCRETE (E.G. CONCRETE FILL AT METAL PAN STAIRS) SHALL NOT BE AIR-ENTRAINED. SLAB-ON-METAL DECK WITH LIGHTWEIGHT CONCRETE SHALL BE AIR-ENTRAINED WITH AN AIR CONTENT OF 4% TO 7%.
- CALCIUM CHLORIDE SHALL NOT BE USED.
- ALL CONSTRUCTION JOINT LOCATIONS MUST BE SHOWN ON SHOP DRAWINGS AND APPROVED BY THE ENGINEER. CONSTRUCTION JOINTS NOT SHOWN ON THE DRAWINGS SHALL BE LOCATED SO AS TO LEAST IMPAIR THE STRENGTH OF THE STRUCTURE AND SHOULD GENERALLY BE LOCATED AT MIDSPAN OR AT POINTS OF MINIMUM SHEAR.
- ALL SHORING SHALL REMAIN IN PLACE UNTIL CONCRETE HAS ATTAINED ITS SPECIFIED 28 DAY COMPRESSIVE STRENGTH.
- PROVIDE A SMOOTH RUBBED SURFACE, FREE FROM BURRS, TIE HOLES, HONEYCOMBING, ETC. ON EXPOSED CONCRETE SURFACES.
- PROVIDE A STEEL TROWELED FINISH FOR INTERIOR SLABS AND A BROOM FINISH FOR EXTERIOR SLABS. NOTE THAT LIGHTWEIGHT CONCRETE SLABS CONTAIN ENTRAINED AIR. REFER TO ACI RECOMMENDATIONS FOR FINISHING AIR-ENTRAINED LIGHTWEIGHT CONCRETE.
- ALL EXPOSED EDGES SHALL BE CHAMFERED 1" UNLESS NOTED OTHERWISE.
- WHEN CONCRETE IS PLACED AGAINST PREVIOUSLY HARDENED CONCRETE, THE INTERFACE SHALL BE CLEAN, FREE OF LANTANCE, AND INTENTIONALLY ROUGHENED TO FULL AMPLITUDE OF APPROXIMATELY 1/4 INCH.
- AT ALL CONSTRUCTION JOINTS NOT DESIGNATED TO BE CONTROL JOINTS, NEW CONCRETE SHALL BE EPOXY BONDED TO HARDENED CONCRETE WITH SIKADUR 32 HI-MOD LPL MANUFACTURED BY SIKACORP. OR ENGINEER APPROVED EQUAL. APPLY PER MANUFACTURER'S RECOMMENDATIONS.
- ELASTOMERIC JOINT SEALANT SHALL BE "SIKAFLEX 1CSL" BY SIKACORP. OR ENGINEER APPROVED EQUAL. SEMI-RIGID EPOXY JOINT SEALANT FOR SLAB CONTROL JOINTS (OR SAWN JOINTS) SHALL BE "SIKADUR 51 SL" AS MANUFACTURED BY SIKACORP. OR ENGINEER APPROVED EQUAL.
- ALL CONCRETE SHALL BE PLACED IN THE DRY.
- PROVIDE POUR STOPS AT THE EDGES OF CONCRETE SLAB POURS WHERE NOT OTHERWISE CONTAINED.
- PROVIDE NON-SHRINK, NON-METALLIC GROUT UNDER ALL BASE PLATES. PROVIDE MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 8,000 PSI FOR GROUT.

E. REINFORCING STEEL

- REINFORCING BARS SHALL BE DETAILED IN ACCORDANCE WITH ACI 315 MANUAL OF STANDARD PRACTICE FOR DETAILING REINFORCED CONCRETE STRUCTURES AND THE STATE CODE.
- COMPLETE SHOP DRAWINGS AND SCHEDULES OF ALL REINFORCING STEEL SHALL BE PREPARED BY THE CONTRACTOR AND SUBMITTED TO THE ENGINEER FOR APPROVAL PRIOR TO COMMENCEMENT OF THAT PORTION OF THE WORK. ALL ACCESSORIES MUST BE SHOWN ON THE SHOP DRAWINGS.
- REINFORCING BARS SHALL CONFORM TO ASTM A615 OR A706 (WELDABLE) GRADE 60.
- REINFORCING STEEL SHALL BE UNCOATED, UNLESS NOTED OTHERWISE. HOWEVER, ALL SUPPORTS SUCH AS CHAIRS, BOLSTERS, SPACERS, BLOCKS AND HANGERS SHALL BE OF NON-CORROSIVE MATERIAL. PROVIDE MINIMUM #5 SUPPORT BAR.
- UNLESS NOTED ON THE DRAWINGS, THE MINIMUM CONCRETE PROTECTION (CLEAR COVER) FOR CAST-IN-PLACE CONCRETE COVER SHALL BE AS FOLLOWS:
 - CONCRETE PLACED AGAINST EARTH 3"
 - FORMED CONCRETE EXPOSED TO EARTH OR WATER 2"
- ALL MECHANICAL SLEEVE CONNECTIONS SHALL CONFORM TO ACI 318 REQUIREMENTS AND DEVELOP IN TENSION AND COMPRESSION AT LEAST 125% OF THE YIELD STRENGTH OF THE BAR.
- WELDED WIRE FABRIC (WWF) SHALL CONFORM TO ASTM A1064 AND SHALL BE SUPPLIED IN FLAT SHEETS ONLY. SPLICES OF WWF SHALL BE AT LEAST 12 INCHES.
- ALL REINFORCEMENT SHALL BE CONTINUOUS THROUGH CONSTRUCTION JOINTS.
- UNLESS NOTED OTHERWISE, BARS SHALL BE CONTINUOUS AND SHALL RUN CONTINUOUSLY AROUND CORNERS AND LAPPED AT NECESSARY SPLICES OR HOOKED AT DISCONTINUOUS ENDS. SPLICES SHALL GENERALLY OCCUR AT MID-SPAN FOR TOP AND MIDDLE BARS, AT SUPPORT FOR BOTTOM BARS AND SHALL BE STAGGERED WHEREVER POSSIBLE.
- BARS SHALL NOT BE CUT OR OMITTED FOR SLEEVE OR OPENINGS IN FLOORS. BARS MAY BE MOVED LATERALLY WITHOUT CHANGING THE DISTANCE FROM THE FACE OF CONCRETE. NO BARS SHALL BE BENT IN FIELD WITHOUT APPROVAL OF THE ENGINEER.
- PIPES AND SLEEVES EMBEDDED IN CONCRETE SHALL NOT BE LARGER IN OUTSIDE DIAMETER THAN 1/4 THE THICKNESS OF THE SLAB OR WALL IN WHICH THEY ARE EMBEDDED, UNLESS OTHERWISE SHOWN ON THE DRAWINGS, NOR SHALL THEY BE LOCATED SO AS TO IMPAIR THE STRENGTH OF THE CONCRETE.
- MINIMUM REINFORCEMENT DEVELOPMENT LENGTH AND LAP SPLICE LENGTHS SHALL BE IN ACCORDANCE WITH ACI 318 FOR CLASS B LAPS UNLESS OTHERWISE NOTED ON THE DRAWINGS.
- PROVIDE ADDITIONAL #5 BAR REINFORCEMENT ALONG EACH SIDE OF OPENINGS (AND EACH FACE), UNLESS NOTED OTHERWISE. BARS SHALL EXTEND AT LEAST 1'-0" BEYOND THE OPENING PERIMETER.

F. POST-INSTALLED CONCRETE ANCHORS

- EXPANSION TYPE ANCHORS SHALL CONFORM TO THE REQUIREMENTS OF ASTM E488, "STANDARD TEST METHODS FOR STRENGTH OF ANCHORS IN CONCRETE AND MASONRY ELEMENTS" AND ICBO ES AC-01, ACCEPTANCE CRITERIA FOR ADHESIVE ANCHORS IN CONCRETE AND MASONRY ELEMENTS.
- ADHESIVE TYPE ANCHORS SHALL FURTHER CONFORM TO THE REQUIREMENTS OF ASTM E1512, "STANDARD TEST METHODS FOR TESTING BOND PERFORMANCE OF ADHESIVE-BONDED ANCHORS" AND ICBO ES AC-01, "ACCEPTANCE CRITERIA FOR ADHESIVE ANCHORS IN CONCRETE AND MASONRY ELEMENTS".
- PROVIDE SIZE, TYPE, AND EMBEDMENT OF ANCHOR INDICATED INSTALLED TO DEVELOP THE MAXIMUM CAPACITY FOR THE EMBEDMENT, TYPE AND ANCHOR SIZE WITH A MINIMUM SAFETY FACTOR OF FOUR.
- DRILL AND EPOXY ANCHORAGES FOR CONCRETE SHALL BE HILTI "HIT-HY 200 ADHESIVE SYSTEM" WITH STANDARD "HAS" RODS, OR APPROVED EQUAL. DRILL AND EPOXY ANCHORAGE FOR MASONRY SHALL BE HILTI "HIT-HY 270 ADHESIVE SYSTEM" OR APPROVED EQUAL. ROD EMBEDMENT LENGTH AND DIAMETER SHALL BE AS INDICATED ON DRAWINGS.
- ANCHOR INSTALLATION SHALL CONFORM TO THE MANUFACTURER'S CURRENT PRINTED INSTRUCTIONS. FOR CORRESPONDING HOLE DIAMETER, REFER TO MANUFACTURER'S PRINTED INSTALLATION INSTRUCTIONS (MPI) AS INCLUDED WITH EACH ADHESIVE PACKAGE.
- A QUALIFIED MANUFACTURER'S REPRESENTATIVE SHALL BE PRESENT DURING FIRST INSTALLATION TO ENSURE CORRECT PROCEDURE.
- REMOVE DUST AND DEBRIS FROM DRILLED HOLES USING COMPRESSED AIR OR VACUUM AT BOTTOM OF HOLE. IMMEDIATELY REMOVE STANDING WATER FROM HOLES TO RECEIVE ADHESIVE ANCHORS.
- DO NOT HAMMER IN ANCHOR BOLTS. INSTALL ANCHOR BOLTS USING A WET DIAMOND DRILLING PROCESS WITH EXTENSION BITS ADDED AS REQUIRED. DO NOT HAMMER DRILL.
- USE ONLY DRILL TYPE AND BIT TYPE AND DIAMETER RECOMMENDED BY ANCHOR MANUFACTURER.
- WHEN EMBEDDED STEEL OR REBAR IS ENCOUNTERED IN THE DRILL PATH, SLANT DRILL TO CLEAR OBSTRUCTION. IF DRILL MUST BE SLANTED MORE THAN 10 DEGREES TO CLEAR OBSTRUCTION, NOTIFY ENGINEER FOR DIRECTION ON HOW TO PROCEED.

G. MASONRY

- CONCRETE MASONRY UNITS SHALL BE ASTM C90, TYPE I, NORMAL WEIGHT HOLLOW LOAD BEARING UNITS, UNLESS NOTED OTHERWISE. THE AVERAGE ASTM C1314 PRISM STRENGTH SHALL BE A MINIMUM OF 1,900 PSI.
- JOINT REINFORCEMENT SHALL BE PREFABRICATED FROM 9-GAUGE DEFORMED WIRE CONFORMING TO ASTM A1064. JOINT REINFORCEMENT SHALL BE HOT-DIPPED GALVANIZED IN CONFORMANCE WITH ASTM A153. USE EXTRA HEAVY DUTY LADDER TYPE AT 16" O.C. VERTICAL - 3/16" SIDE RODS AND 9 GA. CROSS RODS.
- REINFORCING BARS SHALL CONFORM TO ASTM A615, GRADE 60.
- MORTAR SHALL BE ASTM C270, TYPE M OR S PORTLAND CEMENT MORTAR (LOAD BEARING WALLS AND ELEVATOR WALLS) AND TYPE N PORTLAND CEMENT MORTAR (NON-LOAD BEARING WALLS). DO NOT USE CALCIUM CHLORIDE IN MORTAR OR GROUT.
- MASONRY SHALL BE SET ON FULL MORTAR BED.
- CONCRETE FILL FOR LINTELS AND BOND BEAMS SHALL HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
- GROUT FILL FOR MASONRY CELLS SHALL CONFORM TO ASTM C476 AND HAVE A MINIMUM 28-DAY COMPRESSIVE STRENGTH OF 2,500 PSI.
- ALL REINFORCING SHALL BE GROUTED SOLID CONTINUOUSLY IN 4" OR WIDER CELLS OR BOND BEAMS.
- ALL MASONRY WALLS SHALL BE LATERALLY BRACED AGAINST FAILURE OR COLLAPSE UNTIL ANCHORED BY THE STRUCTURE.
- INDEPENDENT THIRD PARTY SPECIAL INSPECTOR SHALL INSPECT ALL GROUTING OPERATIONS AND THE INSTALLATION OF REINFORCING IN LOAD BEARING CONCRETE MASONRY WALLS.
- REINFORCE WALLS AS SHOWN ON PLANS.

H. STEEL DECKS

- ALL STEEL DECKING SHALL CONFORM TO THE STEEL DECK INSTITUTE (SDI) APPLICABLE SPECIFICATIONS AND REQUIREMENTS. INSTALLATION SHALL BE PER THE MANUFACTURER'S RECOMMENDATIONS IN ACCORDANCE WITH SDI SPECIFICATIONS.
- STEEL DECK SHALL TYPICALLY BE STORED OFF THE GROUND AT THE JOB SITE, AND BE PROTECTED FROM THE ELEMENTS WITH A WATERPROOF COVERING WHERE REQUIRED.
- DECK SHEETS SHALL BE PLACED IN ACCORDANCE WITH APPROVED ERECTION LAYOUT DRAWING (INCLUDING FASTENING SCHEDULE) SUPPLIED BY THE DECK MANUFACTURER AND IN CONFORMANCE WITH THE MANUFACTURER'S STANDARDS. UNLESS NOTED OTHERWISE, END LAPS SHALL OCCUR OVER SUPPORTS, AND SHALL NOT BE LESS THAN 2" MINIMUM.
- ALL STEEL TO BE USED FOR DECKING SHALL BE GALVANIZED. TOUCH-UP ALL DAMAGED COATINGS WITH GALVANIZED REPAIR PAINT PER PROJECT SPECIFICATIONS.
- DECK GAUGE, DEPTH, AND TYPE SHALL BE AS INDICATED ON THE DRAWINGS. PROVIDE MINIMUM YIELD STRESS (Fy) OF 50 KSI FOR COMPOSITE FLOOR DECK AND 40 KSI FOR ROOF DECK.
- SUBMIT SHOP DRAWINGS FOR REVIEW AND APPROVAL PRIOR TO CONSTRUCTION.
- DECK SPANS ARE DESIGNED TO MINIMIZE SHORING REQUIREMENTS. THE CONTRACTOR IS RESPONSIBLE FOR ACCOMPLISHING ANY SHORING REQUIRED TO RESIST CONSTRUCTION LOADS ON THE STEEL DECKS.
- ROOF DECK WAS SELECTED ASSUMING TRIPLE SPAN CONDITION.
- PROVIDE 20 GA., GALV. STEEL PLATES AT ALL RIDGES, VALLEYS AND LOCATIONS WHERE DECK CHANGES DIRECTION FOR CONTINUOUS EVEN SURFACE.
- USE WELD WASHERS WHERE RECOMMENDED BY THE DECK MANUFACTURER.
- USE FM-APPROVED STEEL ROOF DECK. FASTENING PATTERN OF NEW DECKING TO CONFORM TO FM-APPROVAL GUIDE FOR CLASS 1-90 AND AS INDICATED BELOW (TYPICAL UNLESS NOTED OTHERWISE).
 - ROOF DECK:
 - SUPPORTS: HILTI X-EMP-19 PAFs, 24/4 PATTERN
 - SIDE LAPS: HILTI #10 DRILL SCREWS @ 4' O.C.
 - COMPOSITE FLOOR DECK:
 - SUPPORTS: 3/4" PUDDLE WELDS, 36/4 PATTERN
 - SIDE LAPS: 1 1/2" LONG WELD @ 24" O.C. (MAX)

I. STRUCTURAL STEEL

- DESIGN FABRICATION AND ERECTION SHALL BE IN ACCORDANCE WITH AISC SPECIFICATION FOR BUILDINGS.
- NEW STRUCTURAL STEEL SHALL CONFORM TO THE FOLLOWING:
 - STRUCTURAL STEEL A572 OR A992 GR. 50 Fy=50 KSI
 - TYPICAL PLATES AND ANGLES ASTM A36 Fy=36 KSI
 - STRUCTURAL TUBING ASTM A1085 Fy=50 KSI
 - HIGH STRENGTH BOLTS ASTM F3125 (GR. A325 TYPE 1) Fy=92 KSI
 - CAST-IN-PLACE ANCHOR RODS F1554 (GRADE 36) Fy=36 KSI
 - HEADED STUDS A108 GR. 50 Fy=50 KSI
 - DRILL & EPOXY ANCHOR RODS A449 Fy=92 KSI
- SHAPES NOTED "GALV." ON DRAWINGS SHALL BE HOT-DIP GALVANIZED IN ACCORDANCE WITH ASTM A123.
- ALL STRUCTURAL STEEL CONNECTIONS NOT SPECIFICALLY DETAILED ON THE PLANS SHALL BE DESIGNED BY THE CONTRACTOR IN ACCORDANCE WITH THE CURRENT EDITION OF THE AISC "MANUAL OF STEEL CONSTRUCTION - ALLOWABLE STRENGTH DESIGN (ASD)". DESIGN FOR ALL CONNECTIONS SHALL BE STAMPED BY A RHODE ISLAND PROFESSIONAL ENGINEER ENGAGED BY THE CONTRACTOR AND SUBMITTED TO THE ARCHITECT/ENGINEER FOR REVIEW PRIOR TO FABRICATION. CONNECTIONS SHALL BE DESIGNED TO DEVELOP 1/2 OF MEMBER'S TOTAL UNIFORM LOAD CAPACITY, TYPICAL UNLESS NOTED OTHERWISE.
- ALL BOLTED CONNECTIONS SHALL USE 3/4" DIA., A-325-N TYPE 1 BOLTS, UNLESS NOTED OTHERWISE.
- ALL NEW STRUCTURAL STEEL SHALL BE GIVEN ONE COAT OF AN APPROVED SHOP PRIMER AND PAINT APPLIED IN ACCORDANCE WITH MANUFACTURER'S RECOMMENDATIONS, UNLESS NOTED OTHERWISE (SEE NOTE B BELOW). DO NOT PAINT TOP FLANGES OF BEAMS THAT RECEIVE SHEAR STUDS. SHOP PAINTING OF STRUCTURAL STEEL SHALL CONFORM TO SSPC-SP2 (INTERIOR SURFACES) OR SSPC-SP6 (EXTERIOR SURFACES).
- AFTER ERECTION IS COMPLETE, TOUCH-UP ALL SHOP PRIMED COATS DAMAGED DURING TRANSPORT AND ERECTION, AND PRIME ALL FIELD WELDS USING THE SAME PAINT USED FOR SHOP PRIMING.
- ANY STRUCTURAL STEEL TO RECEIVE SPRAY-ON FIREPROOFING SHALL BE FABRICATED WITHOUT ANY PRIMER OR PAINT COATINGS. COORDINATE WITH THE ARCHITECT REGARDING ADDITIONAL INFORMATION RELATED TO FIREPROOFING.
- ALL EXPOSED STRUCTURAL STEEL AND CONNECTORS SHALL BE PRIMED AND PAINTED WITH AN APPROVED PAINT SYSTEM. COORDINATE REQUIREMENTS WITH THE ARCHITECT.
- HIGH STRENGTH BOLTS SHALL BE TORQUED TO 70% OF THE MINIMUM TENSILE STRENGTH OF THE BOLT IN CONFORMANCE WITH AISC SPECIFICATION FOR STRUCTURAL JOINTS USING A-325-N TYPE 1 BOLTS. PROVIDE ONE HARDENED WASHER UNDER THE ELEMENT TURNED IN TIGHTENING.
- WELDS SHALL BE MADE ONLY BY OPERATORS CERTIFIED BY THE STANDARD QUALIFICATION PROCEDURE OF THE AMERICAN WELDING SOCIETY. TOUCH UP ALL WELDS WITH THE APPROVED PAINT SYSTEM.
- WELDING: IN ACCORDANCE WITH LATEST EDITION OF AWS D1.1 CODE FOR WELDING IN BUILDING CONSTRUCTION. USE E70 SERIES ELECTRODES UNLESS NOTED OTHERWISE.
- FIELD WELDING OF STRUCTURAL MEMBERS IS NOT PERMITTED UNLESS SPECIFICALLY INDICATED.
- FURNISH AND INSTALL ONE WASHER AND ONE HEAVY HEX NUT WITH ASTM F1554 ANCHOR BOLTS UNLESS OTHERWISE INDICATED.
- PROVIDE FITTED WELDED 3/8" WEB STIFFENER PLATES ON EACH SIDE OF ALL BEAMS SEATED ON WALLS OR COLUMNS UNLESS NOTED OTHERWISE.
- FIELD CUTTING OR MODIFICATION OF STRUCTURAL STEEL IS PROHIBITED UNLESS PRIOR WRITTEN APPROVAL IS RECEIVED FROM THE ENGINEER.
- SURFACES OF GALVANIZED MEMBERS TO BE WELDED SHALL BE GROUND TO BARE METAL PRIOR TO WELDING, AND TOUCHED UP AFTER WELDING IN ACCORDANCE WITH PROJECT SPECIFICATIONS.
- MINIMUM FILLET WELD (LEG) SIZE SHALL BE 3/16", UNLESS NOTED OTHERWISE. FOR ALL OTHER WELDS, EFFECTIVE THROAT SHALL BE 3/16" UNLESS NOTED OTHERWISE ON THE DRAWINGS.
- SHARED ENDS OF GALVANIZED PRETENSIONED TWIST-OFF SPLINE BOLTS SHALL BE TOUCHED UP WITH A ZINC RICH PRIMER IN ACCORDANCE WITH ASTM A780 AFTER INSTALLATION.
- PROVIDE ALL NECESSARY TEMPORARY GUYING, STAYS, AND BRACING REQUIRED TO ERECT AND HOLD NEW STRUCTURE TO RESIST VERTICAL AND LATERAL LOADS. ALL LATERAL LOAD RESISTANCE AND STABILITY OF THE BUILDING IN THE COMPLETED STRUCTURE IS PROVIDED BY A COMBINATION OF MOMENT FRAMES AND BRACED FRAMES, IN EACH ORTHOGONAL DIRECTION (SEE PLAN SHEETS FOR LOCATIONS). THE COMPOSITE METAL DECK/CONCRETE FLOORS AND ROOF DECKS SERVE AS HORIZONTAL DIAPHRAGMS THAT DISTRIBUTE THE LATERAL LOADS HORIZONTALLY TO THE VERTICAL BRACE/MOMENT FRAMES AND SHEAR WALLS. THE VERTICAL BRACE/MOMENT FRAMES AND SHEAR WALLS CARRY THE APPLIED LATERAL LOADS TO THE BUILDING FOUNDATION. PROVIDE TEMPORARY SUPPORTS UNTIL ALL ELEMENTS REQUIRED FOR THE STABILITY OF THE STRUCTURE ARE COMPLETED.

J. FLOOR SYSTEMS

- STRUCTURAL FLOORS ON COMPOSITE METAL DECK: STRUCTURAL FLOORS, EXCEPT AS NOTED, SHALL BE METAL DECK ACTING COMPOSITELY WITH LIGHT-WEIGHT CONCRETE (SEE PLANS). THE COMPOSITE STEEL BEAMS SHEAR CONNECTORS SHALL BE 3/4"x4", 4" LONG HEADED SHEAR STUDS.
- COMPOSITE METAL DECK WAS SELECTED TO SPAN (TRIPLE SPAN CONDITION) UNSHORED TO STEEL BEAMS UNDER WET WEIGHT OF THE SLAB PLUS 20 PSF CONSTRUCTION LIVE LOAD. THE CONTRACTOR SHALL BE COGNIZANT OF ALLOWABLE CONSTRUCTION LIVE LOADS AND PLAN HIS CONCRETE PLACING OPERATIONS ACCORDINGLY SO AS NOT TO OVERSTRESS OR DAMAGE THE METAL FLOOR DECK. THE CONTRACTOR SHALL VERIFY WITH METAL FLOOR DECK MANUFACTURER THAT HIS PARTICULAR CONCRETE PLACING OPERATION IS COMPATIBLE WITH THE TYPE, GAUGE, SPAN, AND LENGTH OF THE METAL FLOOR DECK FURNISHED.
- THE CONTRACTOR SHALL INCLUDE SUFFICIENT CONCRETE AND SHALL ARRANGE PLACING AND FINISHING OPERATIONS TO ACHIEVE LEVEL FLOORS CONSIDERING THE DEFLECTION OF THE NON-COMPOSITE BEAMS, GIRDERS, AND METAL DECK UNDER THE LOAD OF ANY NEWLY PLACED CONCRETE. THE SLAB THICKNESS GIVEN ON THE DRAWING IS THE MINIMUM THICKNESS.
- CURTAIN WALLS & STOREFRONT SYSTEMS ARE ASSUMED AS BEING VERTICALLY & LATERALLY SUPPORTED AT EACH FLOOR SYSTEM.
- ELECTRICAL CONDUITS MAY BE INSTALLED WITHIN THE SLABS OR DECK, SUBJECT TO THE FOLLOWING CRITERIA:
 - CONDUITS ARE OF PVC AND NOT ALUMINUM MATERIAL.
 - SUBMIT A LAYOUT PLAN TO ENSURE THE CONDUITS ARE NOT CONGESTED AND NO MORE THAN 2 CONDUITS CROSS AT THE SAME LOCATION.
 - A MINIMUM 1 1/2" COVER IS MAINTAINED ALL AROUND THE CONDUIT.
 - THE OUTSIDE DIAMETER OF THE CONDUIT IS NO LARGER THAN 1/3 THE CONCRETE SLAB THICKNESS.
 - CONDUITS ARE SPACED A MINIMUM OF 18 INCHES ON CENTER APART.
 - CONDUITS SHALL NOT BE LOCATED OVER A LINE OF STUDS.

ABBREVIATIONS

ADD'L	ADDITIONAL	L.G.	LONG
ALT	ALTERNATE	LONG.	LONGITUDINAL
A.B.	ANCHOR BOLT	LV	LONG LEG VERTICAL
ARCH	ARCHITECT	L.W.	LONG WAY
BTM.	BOTTOM	L.W.C.	LIGHT WEIGHT CONCRETE
BEW	BOTTOM EACH WAY	MAX.	MAXIMUM
BM	BEAM	MECH.	MECHANICAL
BOF	BOTTOM OF FOOTING	M.M.	MISCELLANEOUS METAL
BRG	BEARING	MIN.	MINIMUM
BS	BOTH SIDES	MTL	METAL
C	CAMBER	NF	NEAR FACE
C CMF	C COLD FORMED METAL FRAMING	N-S	NON SHRINK
CIP	CAST-IN-PLACE	NFS	NOT TO SCALE
CLR	CLEAR	O.C.	ON CENTER
COL	COLUMN	PL	PLATE
COMP. DK.	COMPOSITE DECK	OPNG	OPENING
CONC.	CONCRETE	R & D	REMOVE AND DISPOSE
CONU	CONCRETE MASONRY UNIT	REINF.	REINFORCING
CJ	CONTROL JOINT	SC	SHEAR CONNECTOR
CONST. JT.	CONSTRUCTION JOINT	SLV	SHORT LEG VERTICAL
CONT.	CONTINUOUS	SOG	SLAB ON GRADE
DIA or Ø	DIAMETER	S.S.	STAINLESS STEEL
DWL'S	DOWELS	STIFF	STIFFENER
DWG	DRAWING	STL	STEEL
EA.	EACH	SJ	SAWN JOINT
E.F.	EACH FACE	T	TOP
E.W.	EACH WAY	TCX	TOP CHORD EXTENSION
EL.	ELEVATION	THK	THICK
E.J.	EXPANSION JOINT	TOC	TOP OF CONCRETE
EQ.	EQUAL	TOP	TOP OF FOOTING
EX. or EXIST.	EXISTING	TOW	TOP OF WALL
F.F.	FAR FACE	TRANS.	TRANSVERSE
F.FE	FINISH FLOOR ELEVATION	TSL	TOP OF SLAB
FND	FOUNDATION	TST	TOP OF STEEL
FTG	FOOTING	TYL	TYPICAL
GA.	GAUGE	U.N.O.	UNLESS NOTED OTHERWISE
GALV.	GALVANIZED	VERT.	VERTICAL
G.C.	GENERAL CONTRACTOR	V.I.F.	VERIFY IN FIELD
HORIZ.	HORIZONTAL	WWF	WELDED WIRE FABRIC
HSS	HOLLOW STRUCTURAL SHAPE	W/	WITH
I.F.	INSIDE FACE	W.P.	WORKING POINT
JBE	JOIST BEARING ELEVATION		

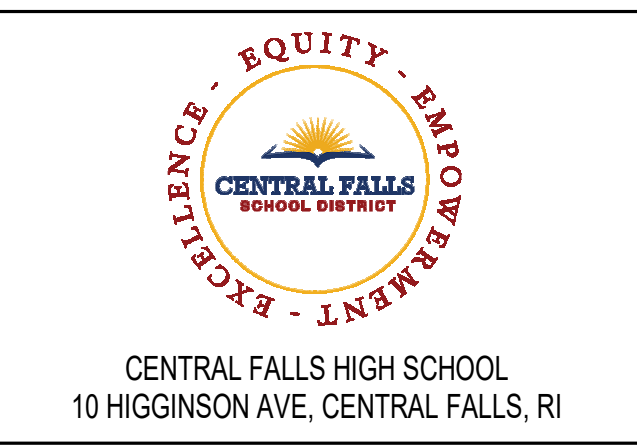
COMPONENTS & CLADDING WIND LOADS AT ROOF/WALLS

COMPONENTS & CLADDING WIND PRESSURE				
WIND SPEED V _w = 133 mph				
MEAN ROOF HEIGHT h = 56'				
EXPOSURE C				
ASCE 7-16 Ch. 30 PART 2				
ZONE	EFFECTIVE WIND AREA (ft ²)	PRESSURE (psf)		
WALLS	4 10	50.9	-55.2	
	4 20	48.5	-52.9	
	4 50	45.5	-49.9	
	4 100	43.3	-47.5	
	5 10	50.9	-68.1	
	5 20	48.5	-63.5	
	5 50	45.5	-57.4	
	5 100	43.3	-52.9	
	ROOF	1 10	20.7	-81.0
		1 20	19.4	-75.6
1 50		17.7	-68.6	
1 100		16.4	-63.2	
1' 10		10.7	-46.5	
1' 20		19.4	-46.5	
1' 50		17.7	-46.5	
1' 100		16.4	-46.5	
2 10		20.7	-106.9	
2 20		19.4	-100.0	
2 50	17.7	-90.9		
2 100	16.4	-84.0		
3 10	20.7	-145.7		
3 20	19.4	-132.0		
3 50	17.7	-113.7		
3 100	16.4	-100.0		



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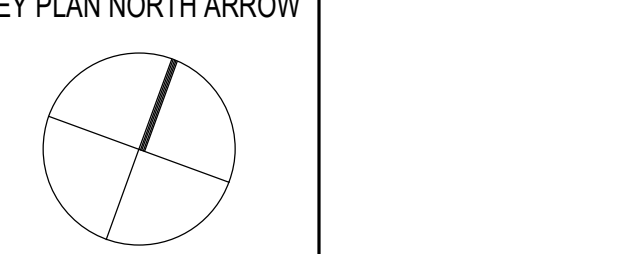


CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

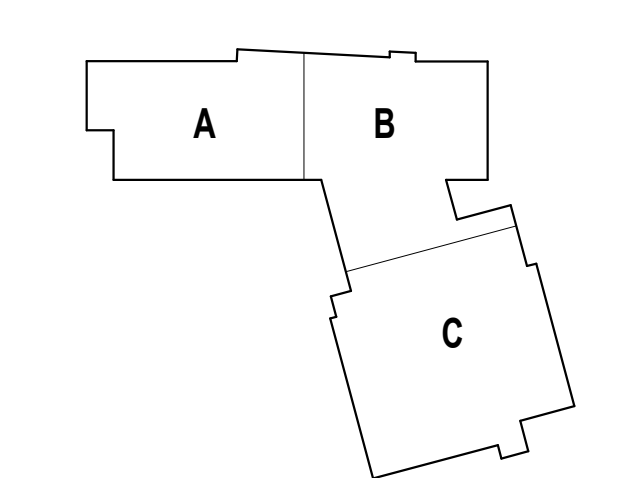
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100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW

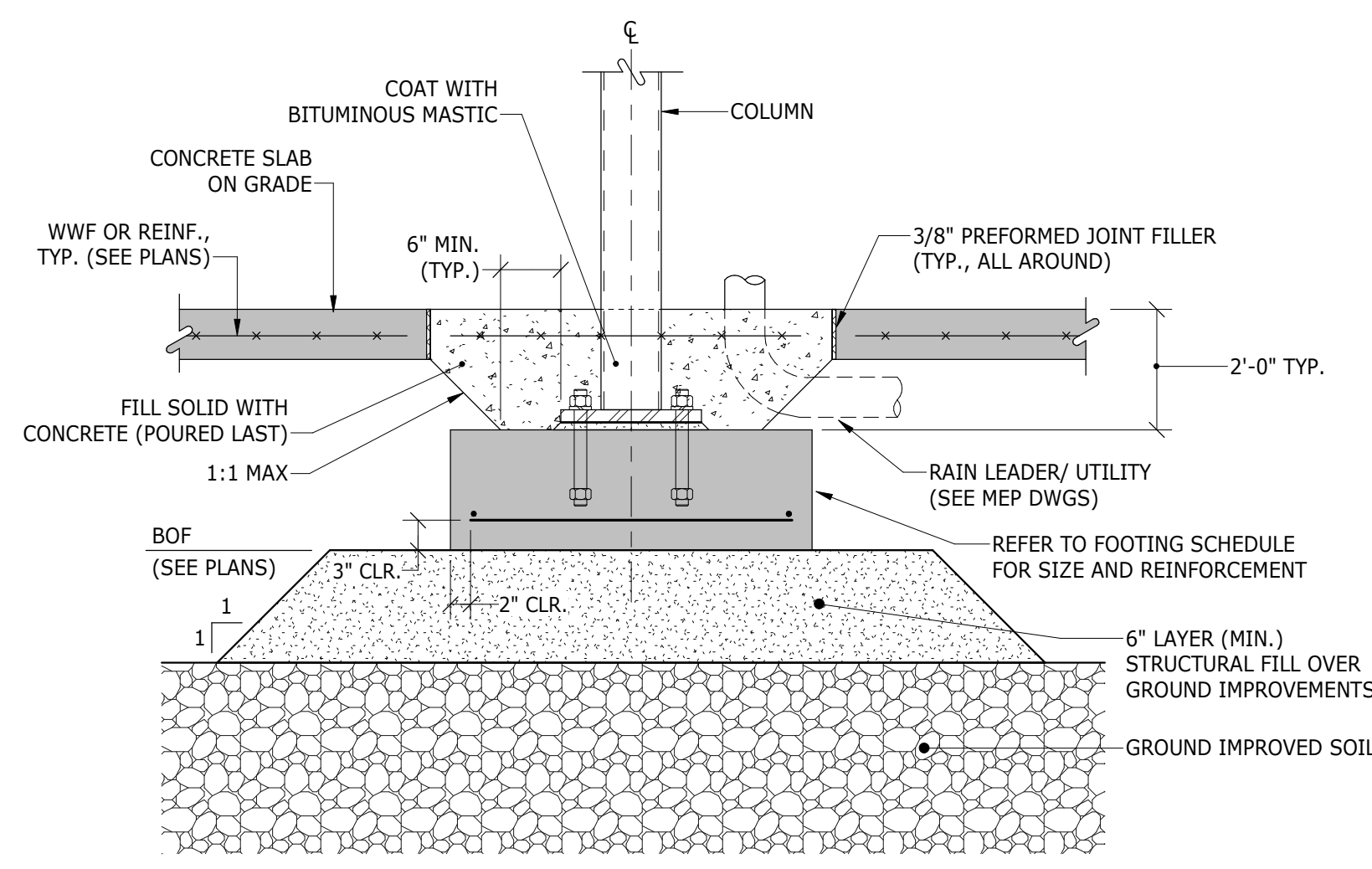


KEYPLAN



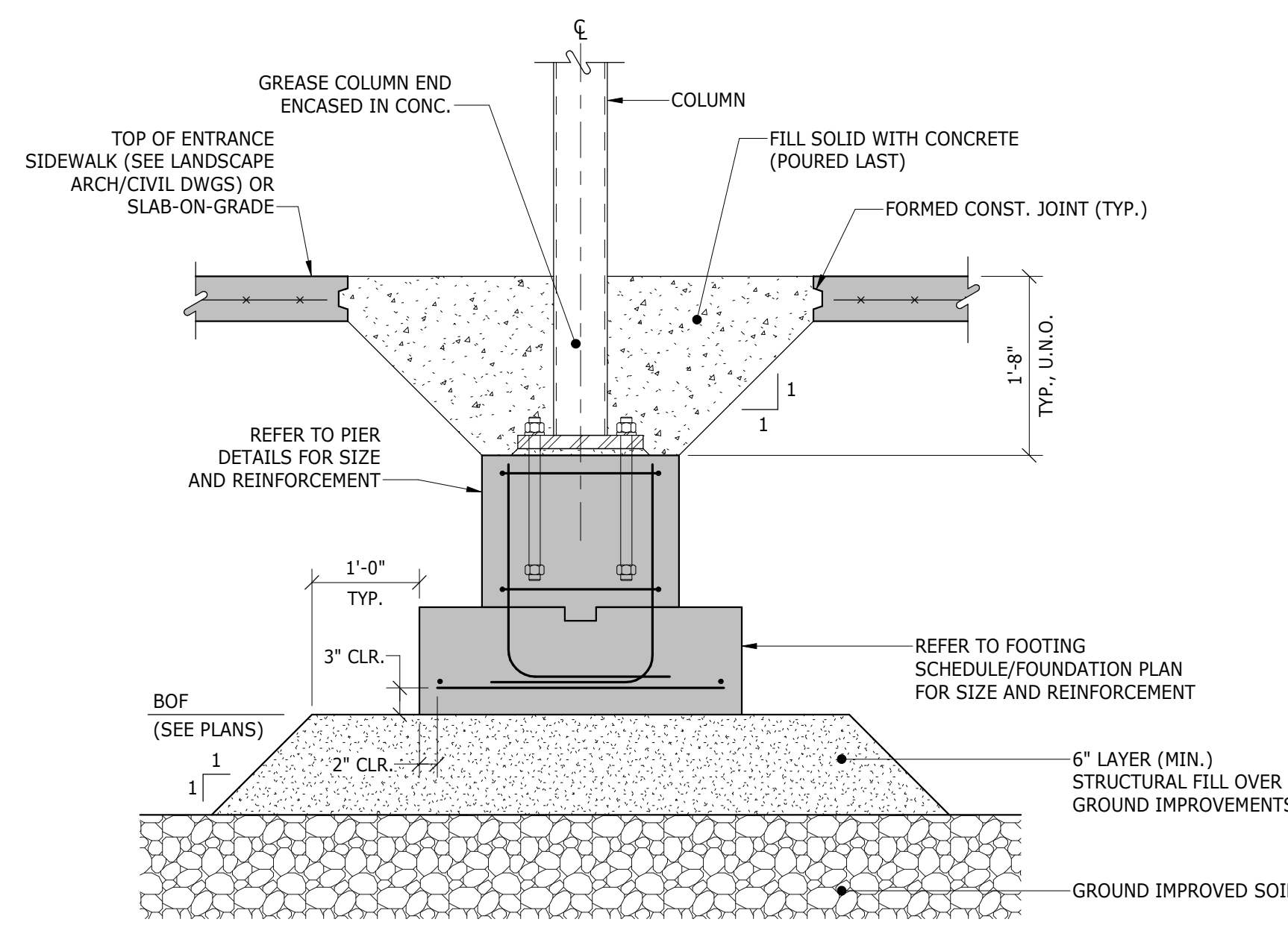
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STRUCTURAL NOTES	
DRAWN BY:	JDB / MSS
REVIEWED BY:	MGM / BP
SCALE:	AS INDICATED
JOB NO:	2202.02
DATE:	OCTOBER 13, 2023
DRAWING NUMBER:	S0.01



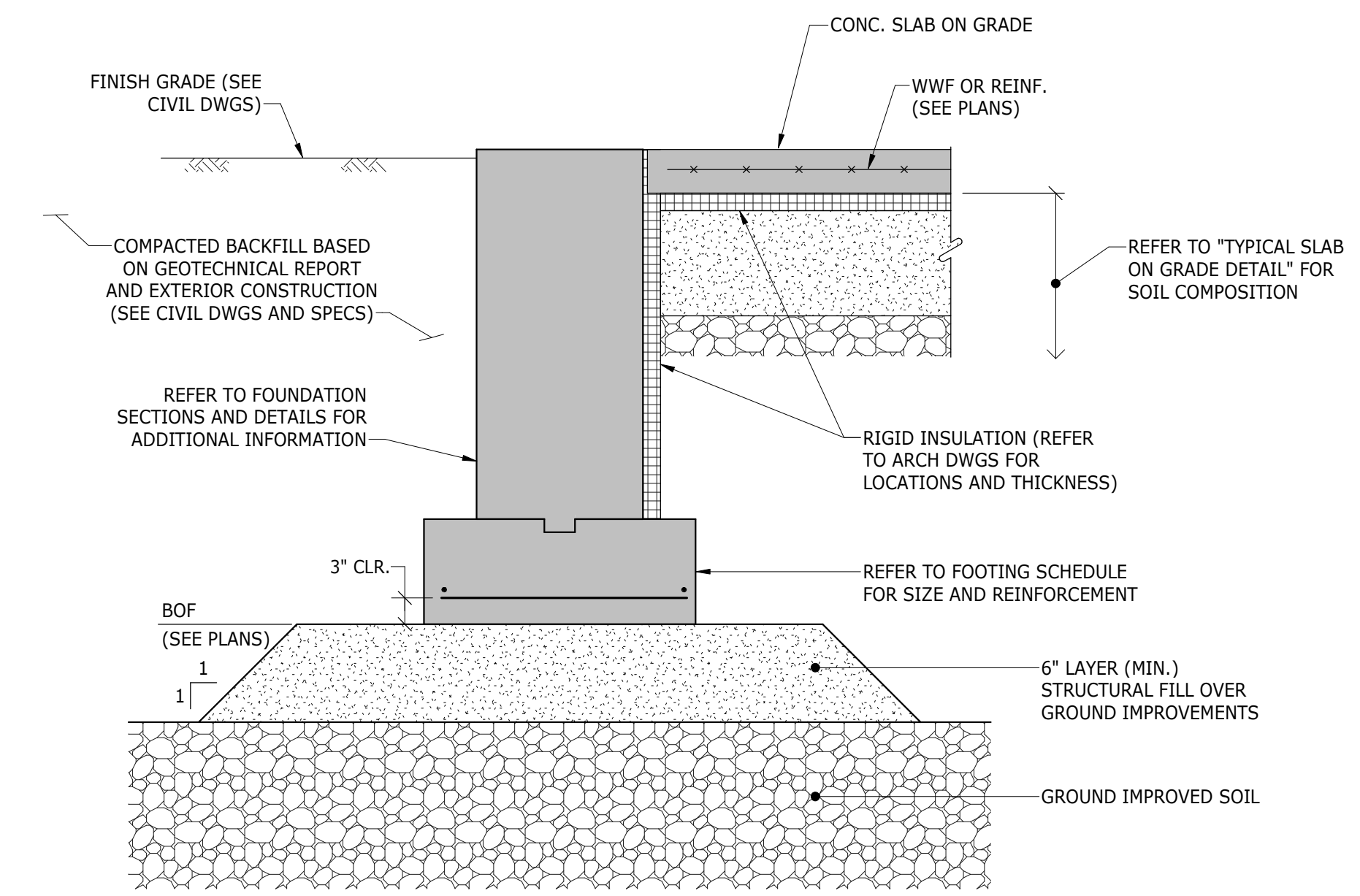
NOTE:
REFER TO DRAWING **S0.01** GEOTECHNICAL REPORT, AND PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION REGARDING SUBGRADE PREPARATION AND OVER EXCAVATION REQUIREMENTS.

TYPICAL COLUMN FOOTING DETAIL
NOT TO SCALE



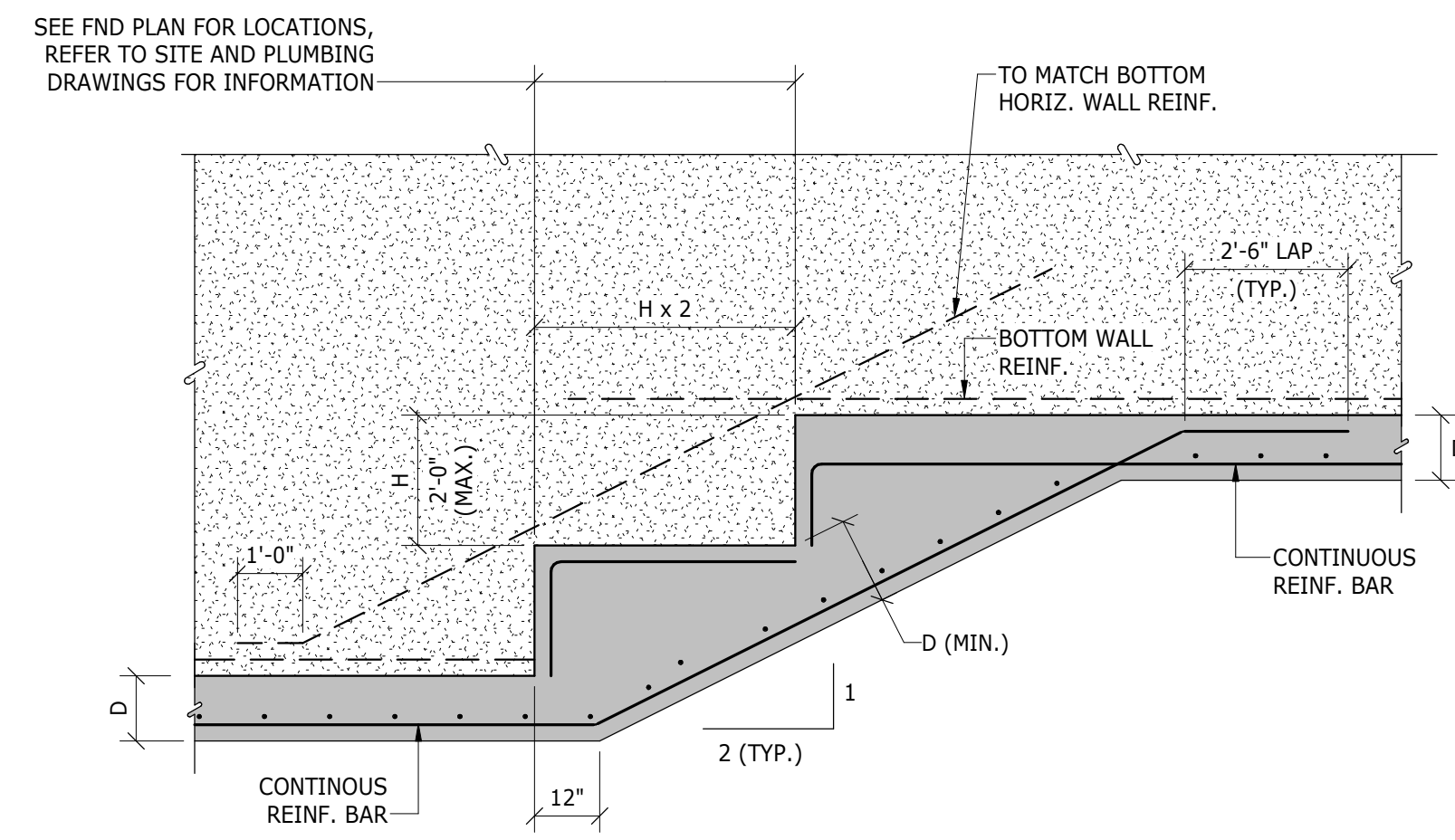
NOTE:
REFER TO DRAWING **S0.01** GEOTECHNICAL REPORT, AND PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION REGARDING SUBGRADE PREPARATION AND OVER EXCAVATION REQUIREMENTS.

TYPICAL COLUMN PIER FOOTING DETAIL
NOT TO SCALE

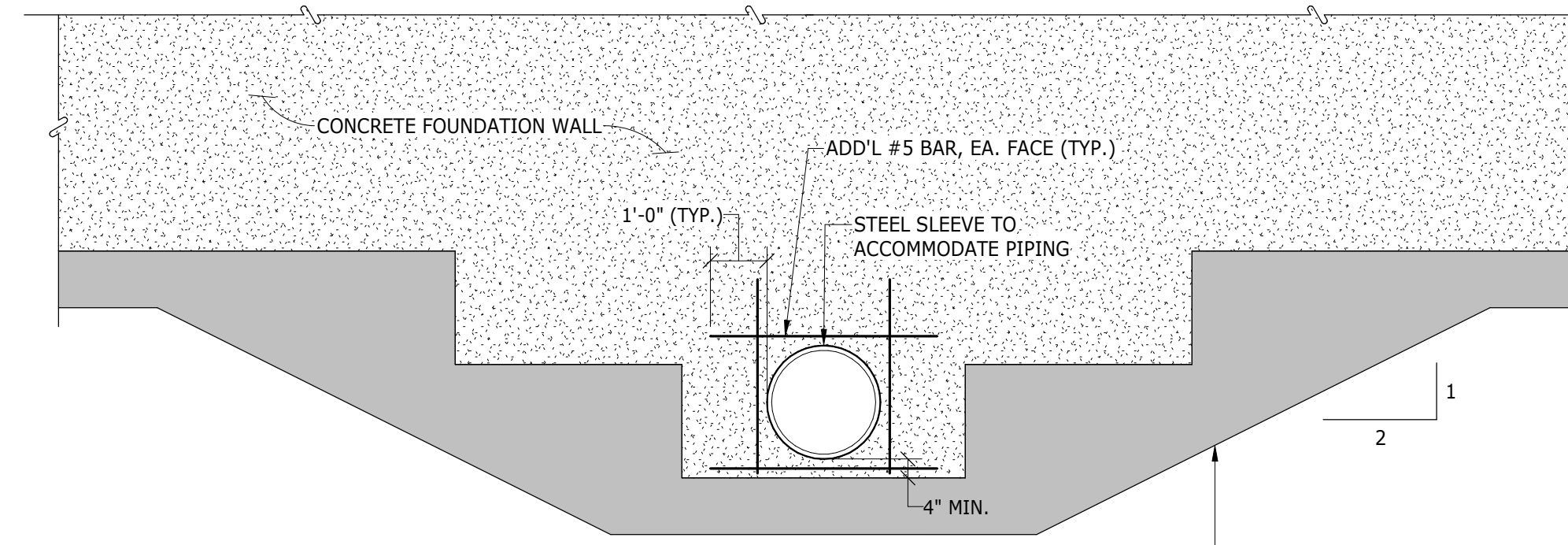


NOTE:
REFER TO DRAWING **S0.01** GEOTECHNICAL REPORT, AND PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION REGARDING SUBGRADE PREPARATION AND OVER EXCAVATION REQUIREMENTS.

TYPICAL PERIMETER FOUNDATION WALL DETAIL
NOT TO SCALE

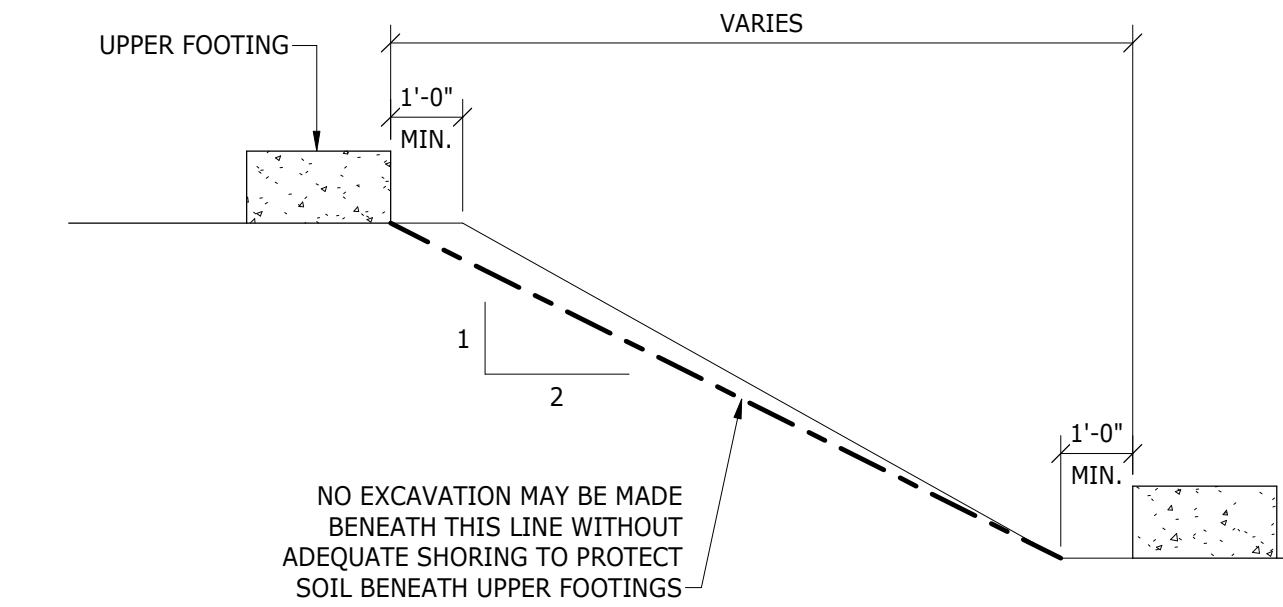


TYPICAL ELEVATION OF CONTINUOUS STEPPED WALL FOOTING
NOT TO SCALE

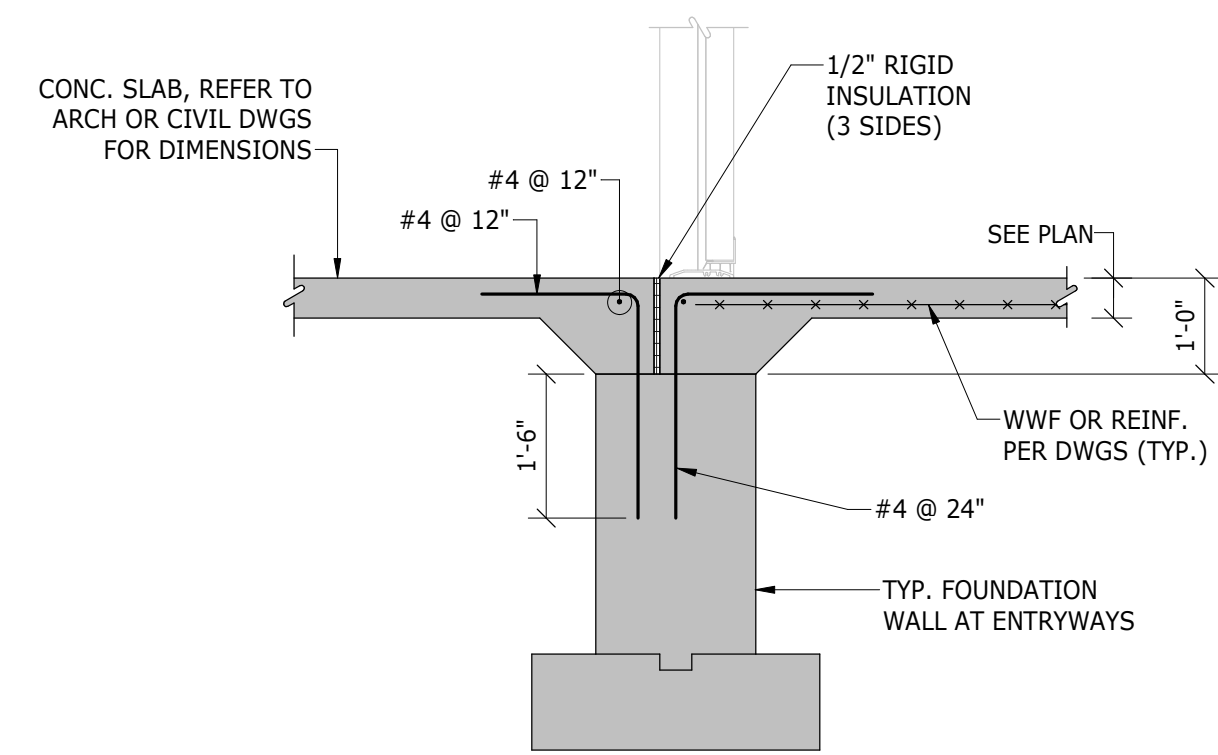


NOTE:
STEP FOOTING AS REQUIRED TO BE BELOW UTILITY OPENING. SEE SITE AND PLUMBING DRAWINGS FOR SIZE AND INVERT LOCATIONS. PROVIDE CONCRETE THRUST BLOCK FOR UTILITY WHERE REQUIRED PER MEP DWGS.

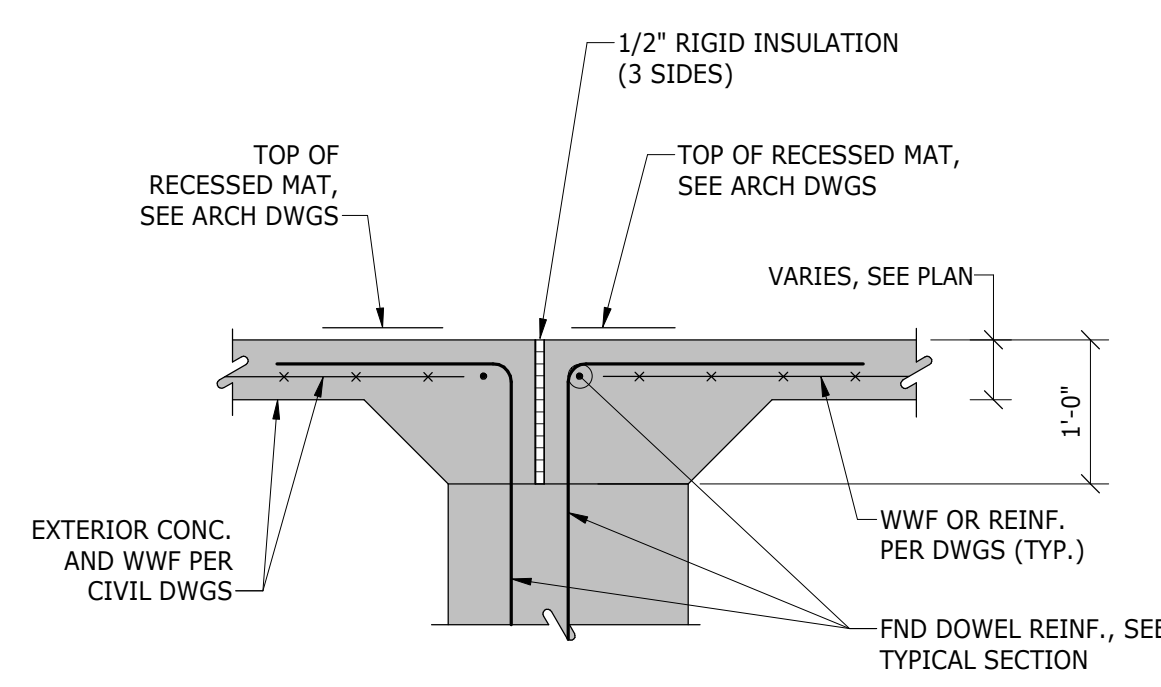
TYPICAL ELEVATION OF STEPPED WALL FOOTING AT UTILITY OPENING DETAIL
NOT TO SCALE



TYPICAL SLOPE BETWEEN FOOTINGS DETAIL
NOT TO SCALE

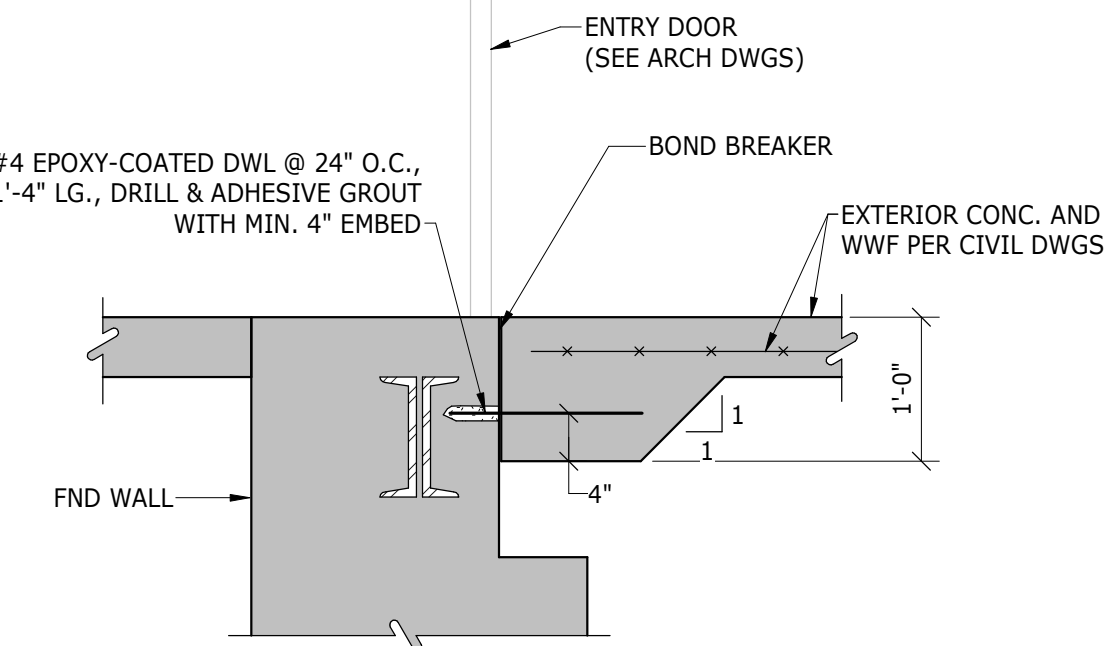


TYPICAL

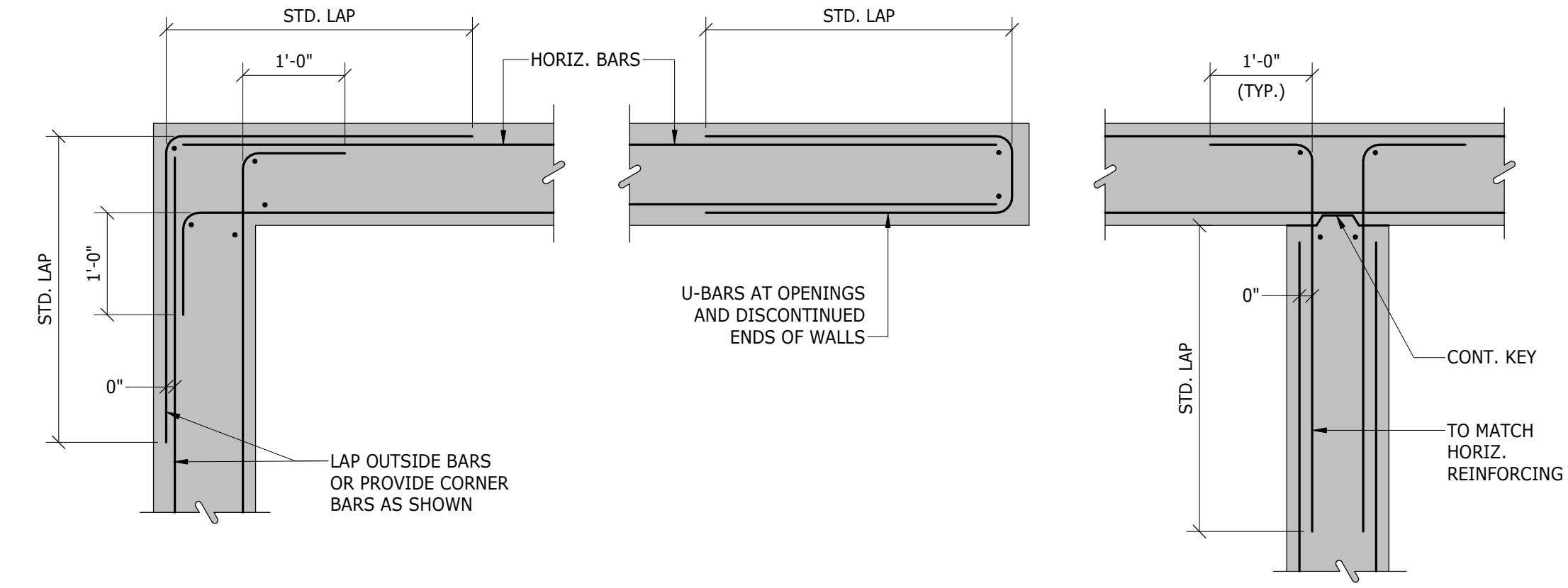


AT RECESS MAT

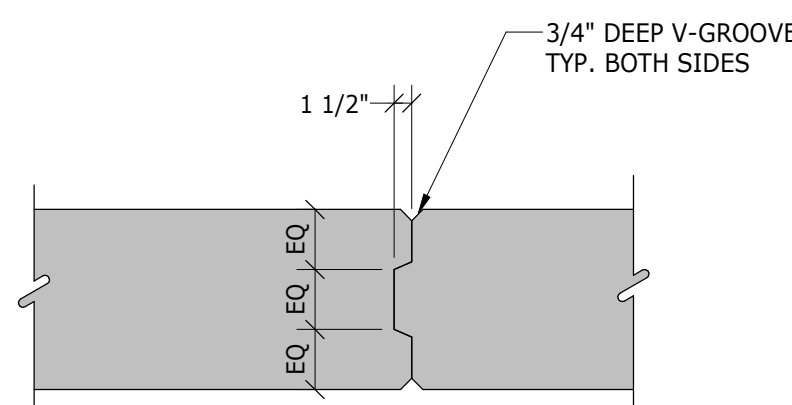
TYPICAL ENTRYWAY DETAILS
NOT TO SCALE



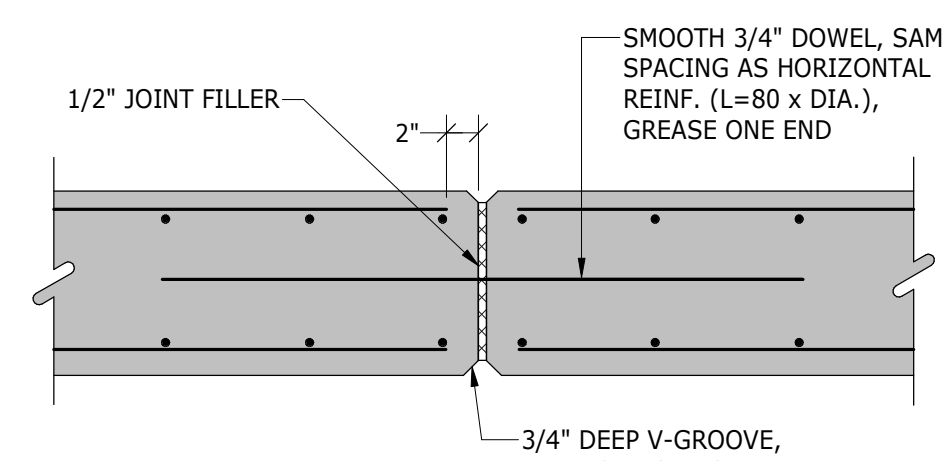
AT EXTERIOR SHEAR WALL



TYPICAL PLAN OF HORIZONTAL REINFORCING OF CONCRETE AND FOUNDATION WALLS
NOT TO SCALE

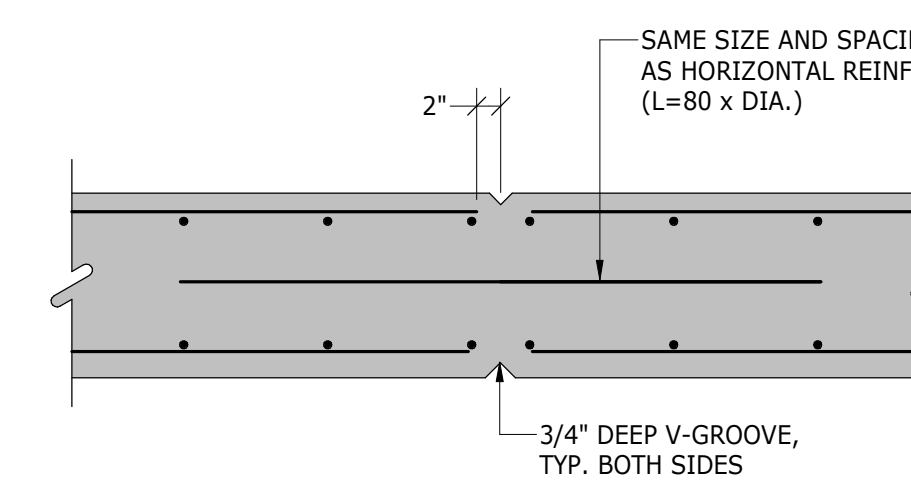


TYPICAL KEY DETAIL
NOT TO SCALE



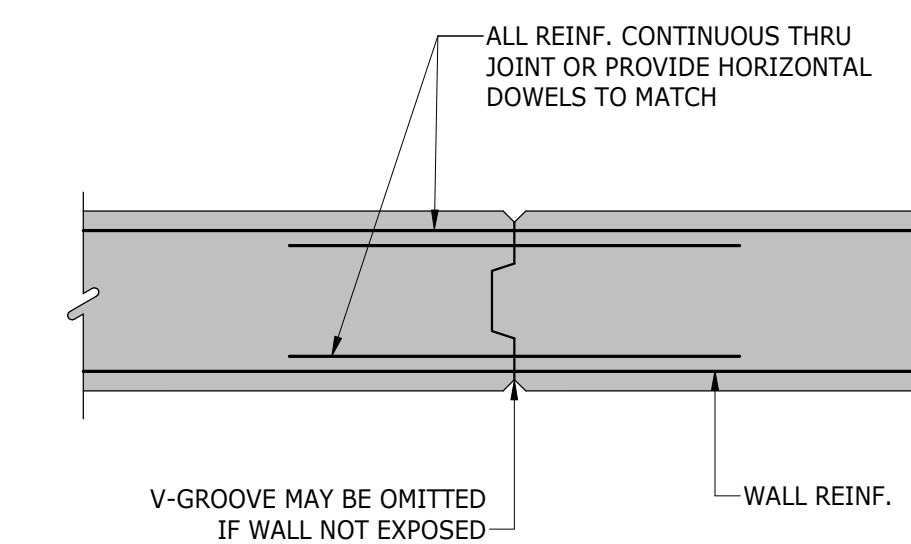
NOTES:
1. SPACE AT 40'-0" CENTER TO CENTER MAX.
2. PROVIDE AT ALL FOUNDATION STEPS.

TYPICAL CONCRETE WALL EXPANSION JOINT DETAIL (FOR SITE WALLS ONLY)
NOT TO SCALE



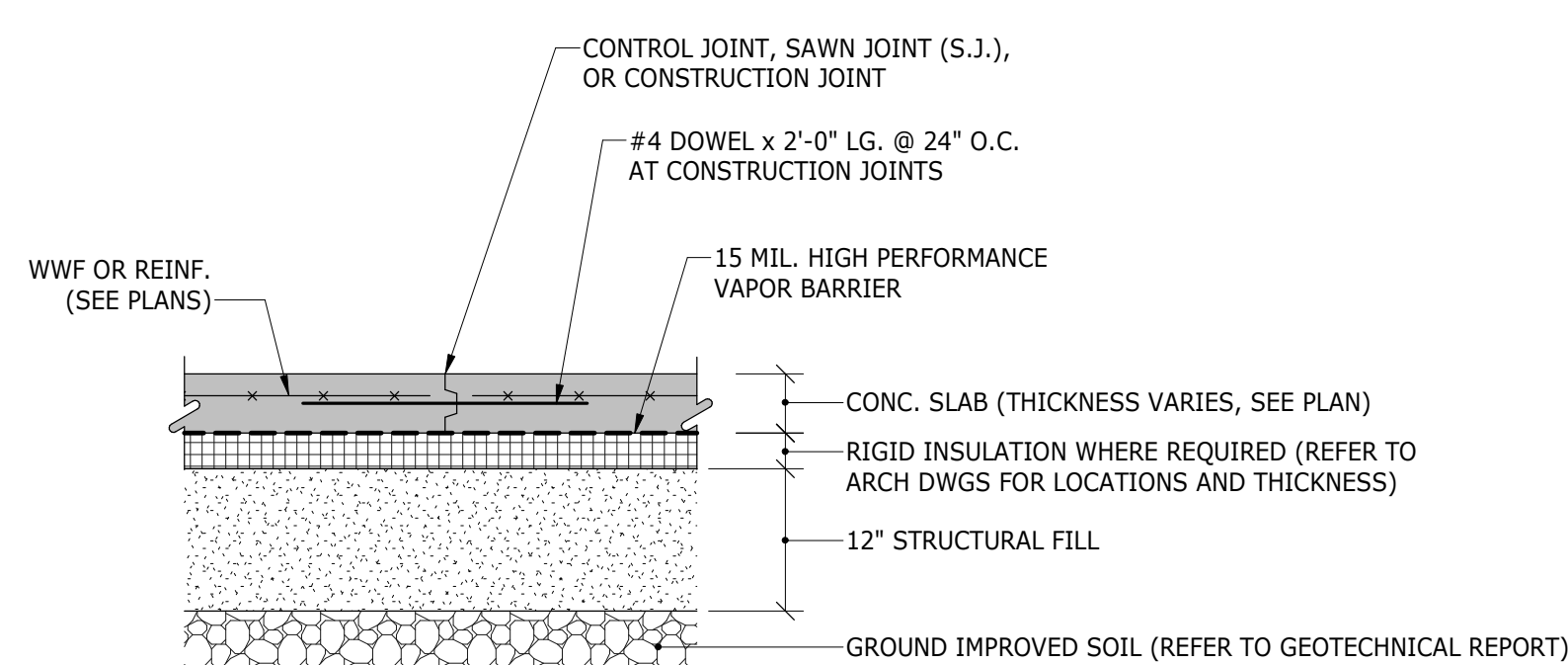
NOTES:
1. SPACE AT 20'-0" CENTER TO CENTER MAX.
2. PROVIDE AT ALL FOUNDATION STEPS (SITE WALLS ONLY).
3. CONSTRUCTION JOINT MAY BE SUBSTITUTED FOR A CONTROL JOINT.

TYPICAL CONCRETE WALL CONTROL JOINT DETAIL
NOT TO SCALE



NOTE:
PROVIDE CONSTRUCTION JOINTS @ 60' O.C. (MAX.) ALTERNATE PLACING OF PANELS, ALLOWING 36 HOURS FROM THE END OF ONE POUR TO THE BEGINNING OF ADJACENT POURS.

TYPICAL CONCRETE WALL CONSTRUCTION JOINT DETAIL
NOT TO SCALE



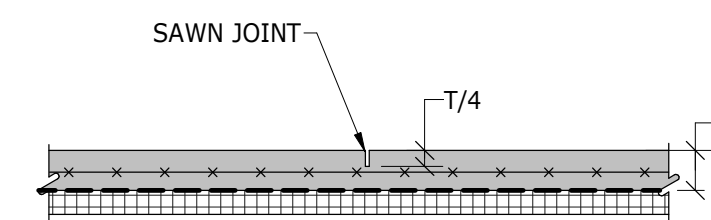
NOTES:

1. PROVIDE "MVRA" PER NOTES ON DRAWING [S0.01](#)
2. REFER TO DRAWING [S0.01](#) GEOTECHNICAL REPORT, AND PROJECT SPECIFICATIONS FOR ADDITIONAL INFORMATION REGARDING SUBGRADE PREPARATION AND OVER EXCAVATION REQUIREMENTS.

TYPICAL SLAB ON GRADE DETAIL

NOT TO SCALE

SLAB THICKNESS "T" (IN.)	MAX. JOINT SPACING (FT.)
5	15

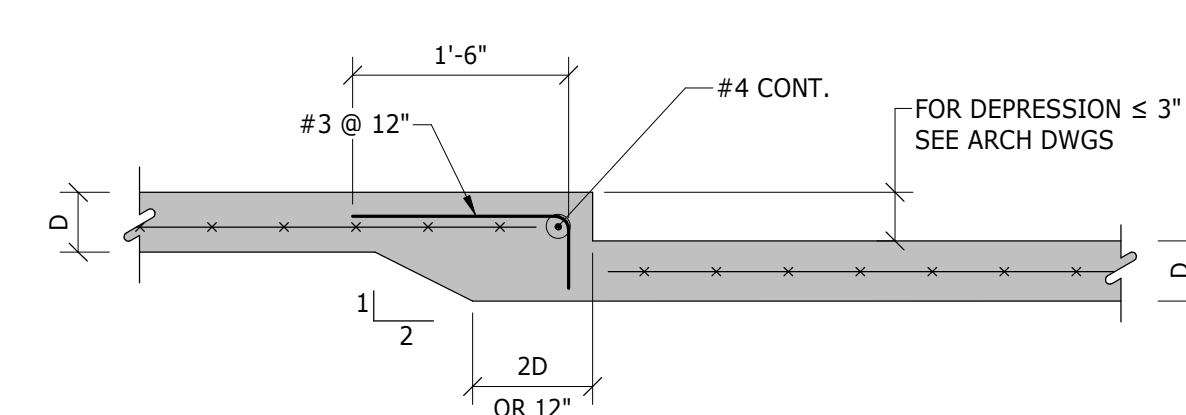


NOTES:

1. EXTEND JOINTS BY HAND TOOL WHERE VERTICAL SURFACES OBSTRUCT SAW CUTTING.
2. SAW CUT MINIMUM 1/4" DEEP JOINT WITH EARLY ENTRY SAW AS SOON AS POSSIBLE AFTER FINISHING SLAB (4 TO 8 HOURS MAX. - PROPER TIMING IS DICTATED BY THE RATE OF CONCRETE HARDENING).
3. MAXIMUM SPACING OF JOINTS IN EACH DIRECTION IN FEET IS 3-TIMES THE SLAB THICKNESS IN INCHES (SEE CHART).
4. LOCATE CONTROL JOINTS ON A RECTANGULAR GRID AND SUBMIT LAYOUT OF CONTROL JOINTS FOR APPROVAL UNLESS JOINT LOCATIONS ARE OTHERWISE SPECIFIED. NO RE-ENTRANT CORNERS ARE PERMITTED.
5. ALTERNATE CONCRETE PLACEMENTS ALLOWING 36 HOURS BETWEEN ADJACENT POURS.
6. PREPARE ALL CONTROL JOINTS TO RECEIVE SEALANT BY HIGH PRESSURE WASHING AFTER SAW CUTTING, SAND BLASTING AFTER THE JOINTS ARE DRY AND THEN BLOWING OUT THE JOINTS WITH CLEAN, DRY COMPRESSED AIR.
7. FILL JOINTS IN EXPOSED SLABS WITH A FLEXIBLE EPOXY CONTROL JOINT RESIN (SIKADUR 51 SL OR EQUIVALENT. INSTALL 60-90 DAYS AFTER CONCRETE PLACEMENT.

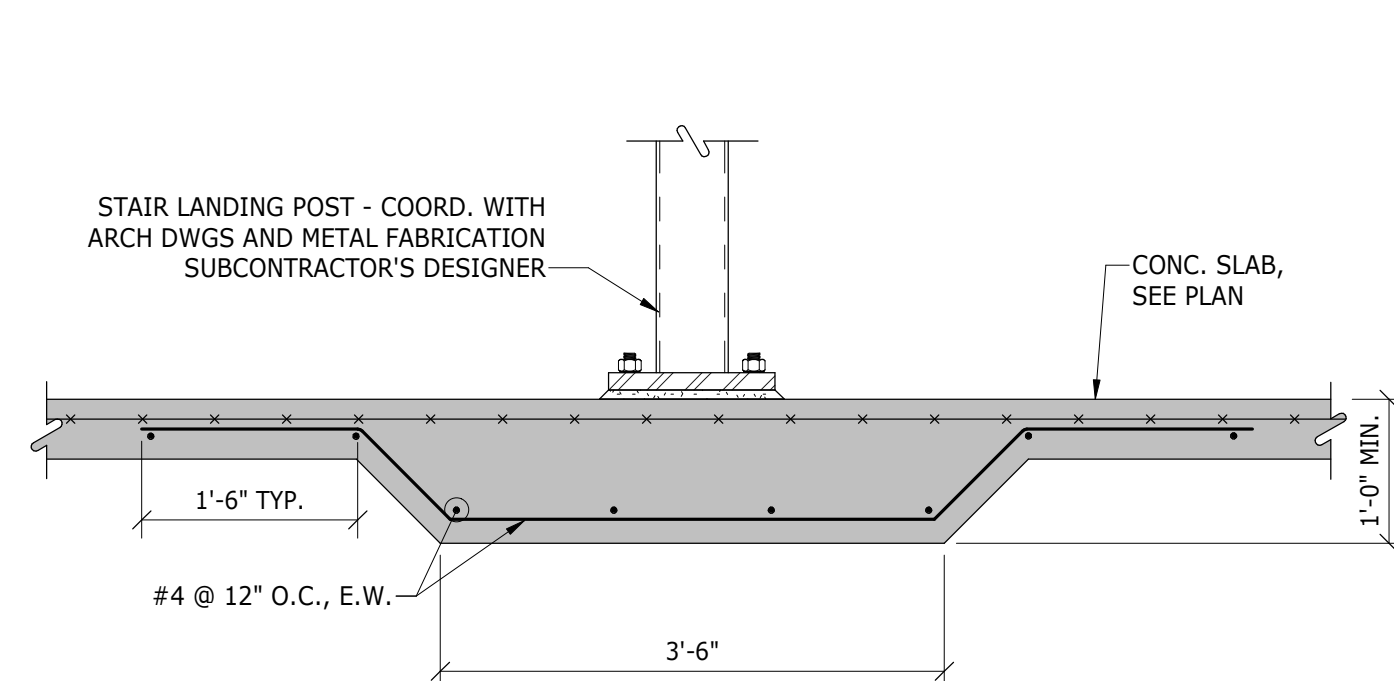
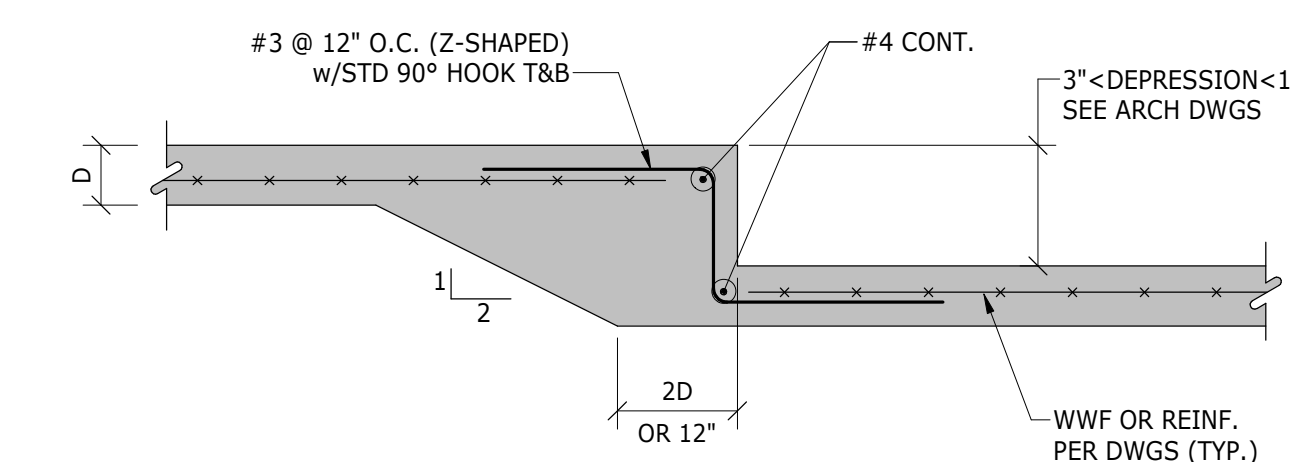
TYPICAL SLAB ON GRADE CONTROL JOINT DETAIL

NOT TO SCALE



TYPICAL DEPRESSED SLAB ON GRADE DETAIL

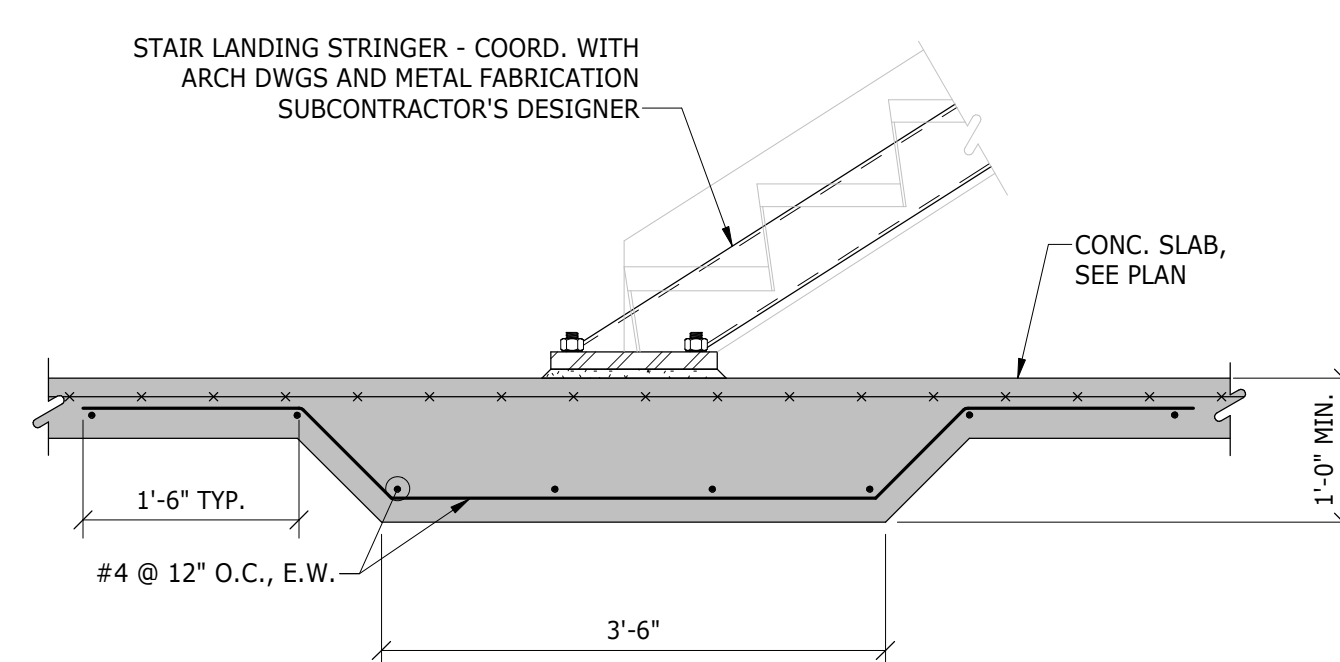
NOT TO SCALE



NOTE:

THICKENED SLAB TO BE 3'-6" x 3'-6". CENTER POST ON THICKENED SLAB.

STAIR LANDING POST



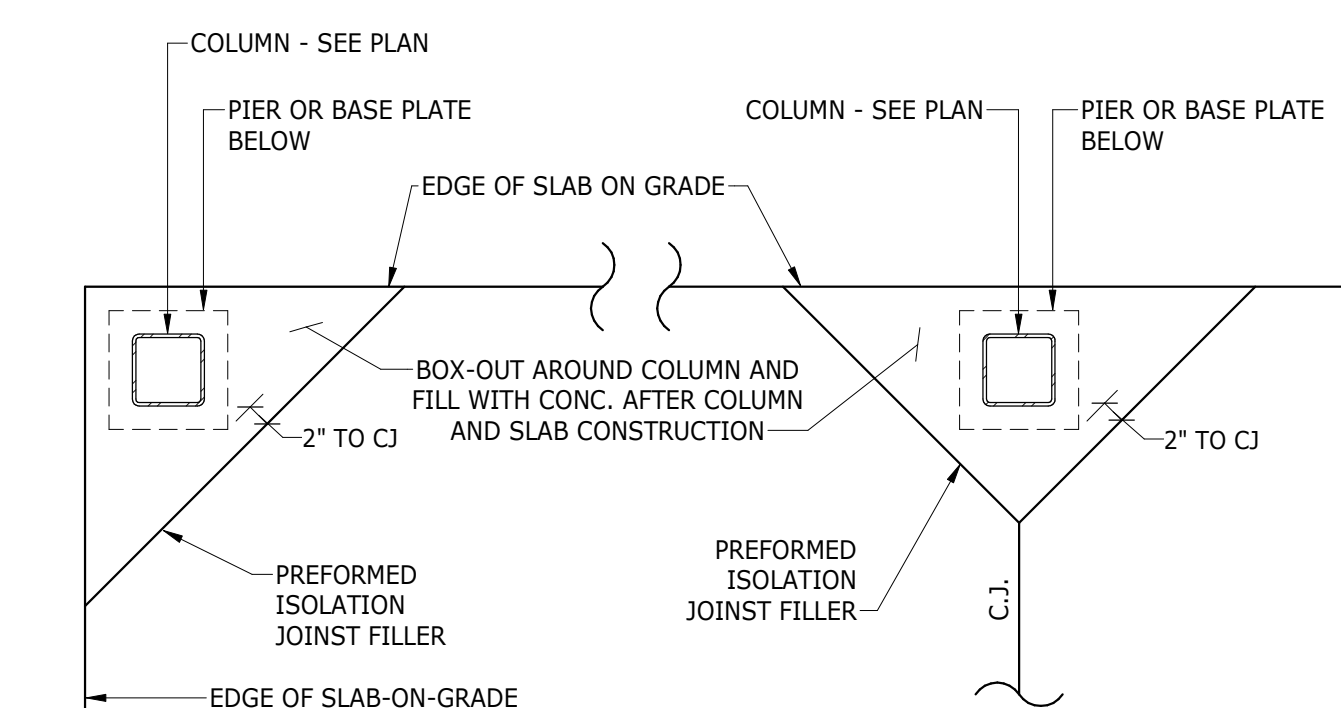
NOTE:

THICKENED SLAB TO BE 3'-6" x 3'-6". CENTER STRINGER ON THICKENED SLAB.

STAIR STRINGER

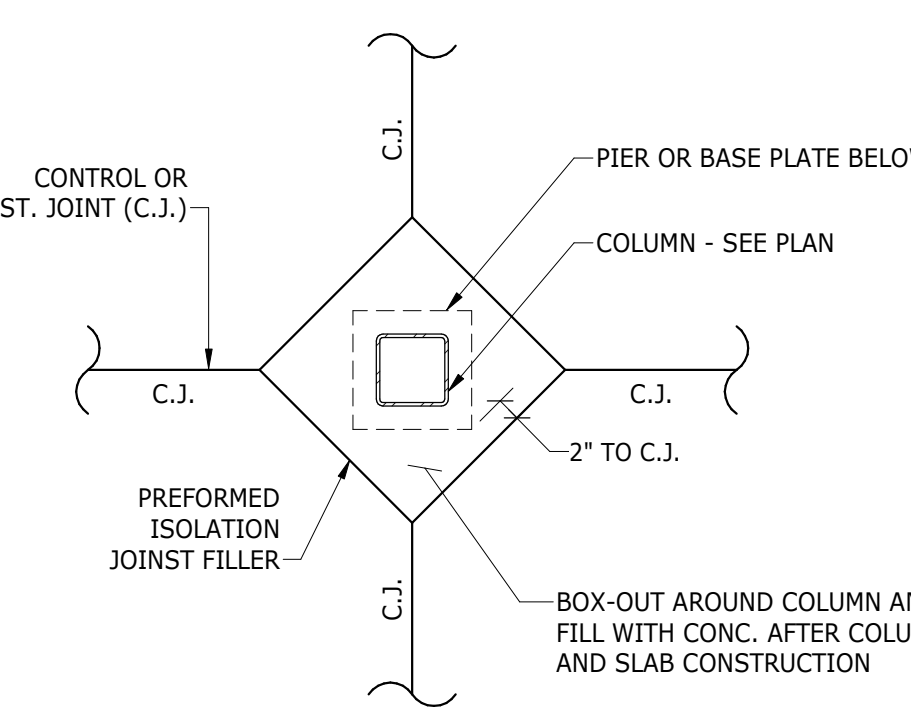
TYPICAL THICKENED SLAB DETAILS

NOT TO SCALE

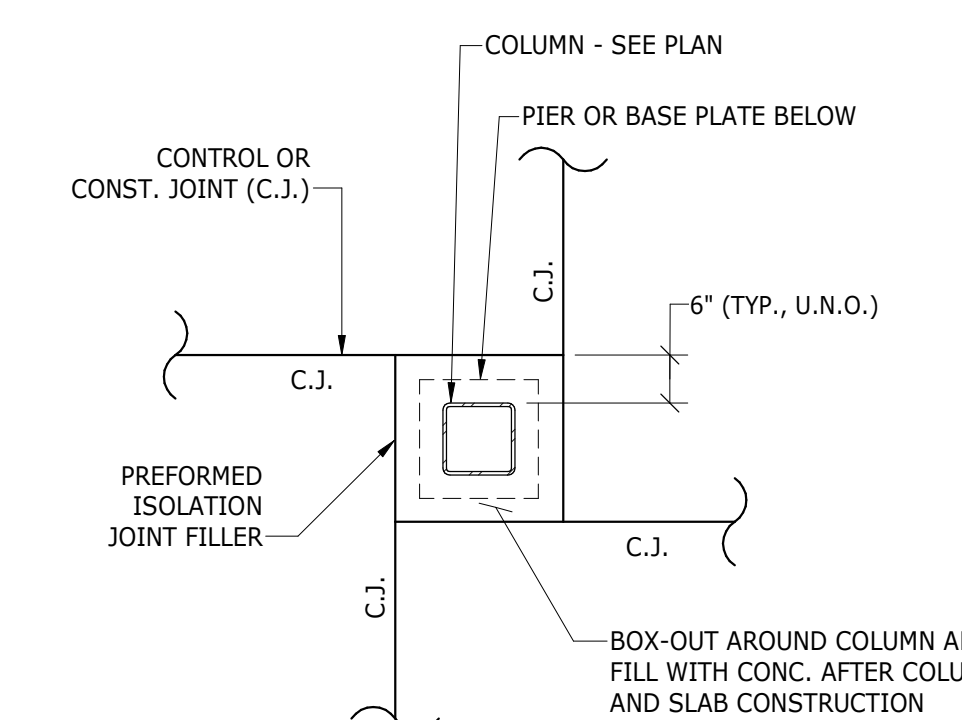


CORNER COLUMN

EDGE COLUMN



INTERIOR COLUMN - DIAMOND PATTERN



INTERIOR COLUMN - PINWHEEL PATTERN

NOTES:

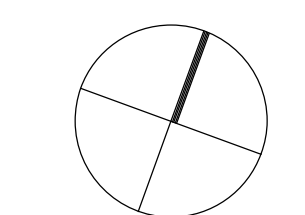
1. REMOVE TOP INCH OF PREFORMED ISOLATION JOINT FILLER AND SEAL JOINT AS REQUIRED FOR CONTROL JOINTS.
2. AT PINWHEEL PATTERN, CONTRACTOR HAS THE OPTION OF PLACING ISOLATION JOINT TIGHT TO COLUMN AND SAW CUT PINWHEEL PATTERN

TYPICAL COLUMN AT SLAB ON GRADE ISOLATION JOINT DETAILS

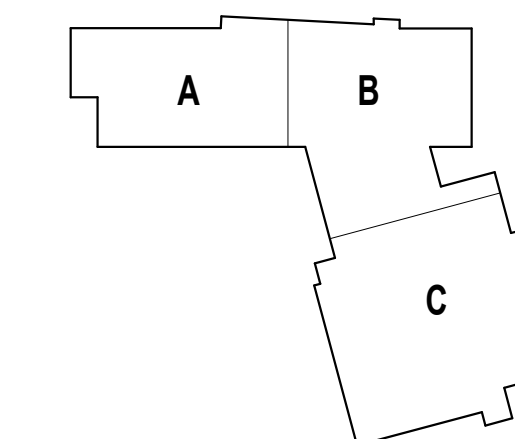
NOT TO SCALE

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

TYPICAL FOUNDATION DETAILS - 2

DRAWN BY: JDB / MSS

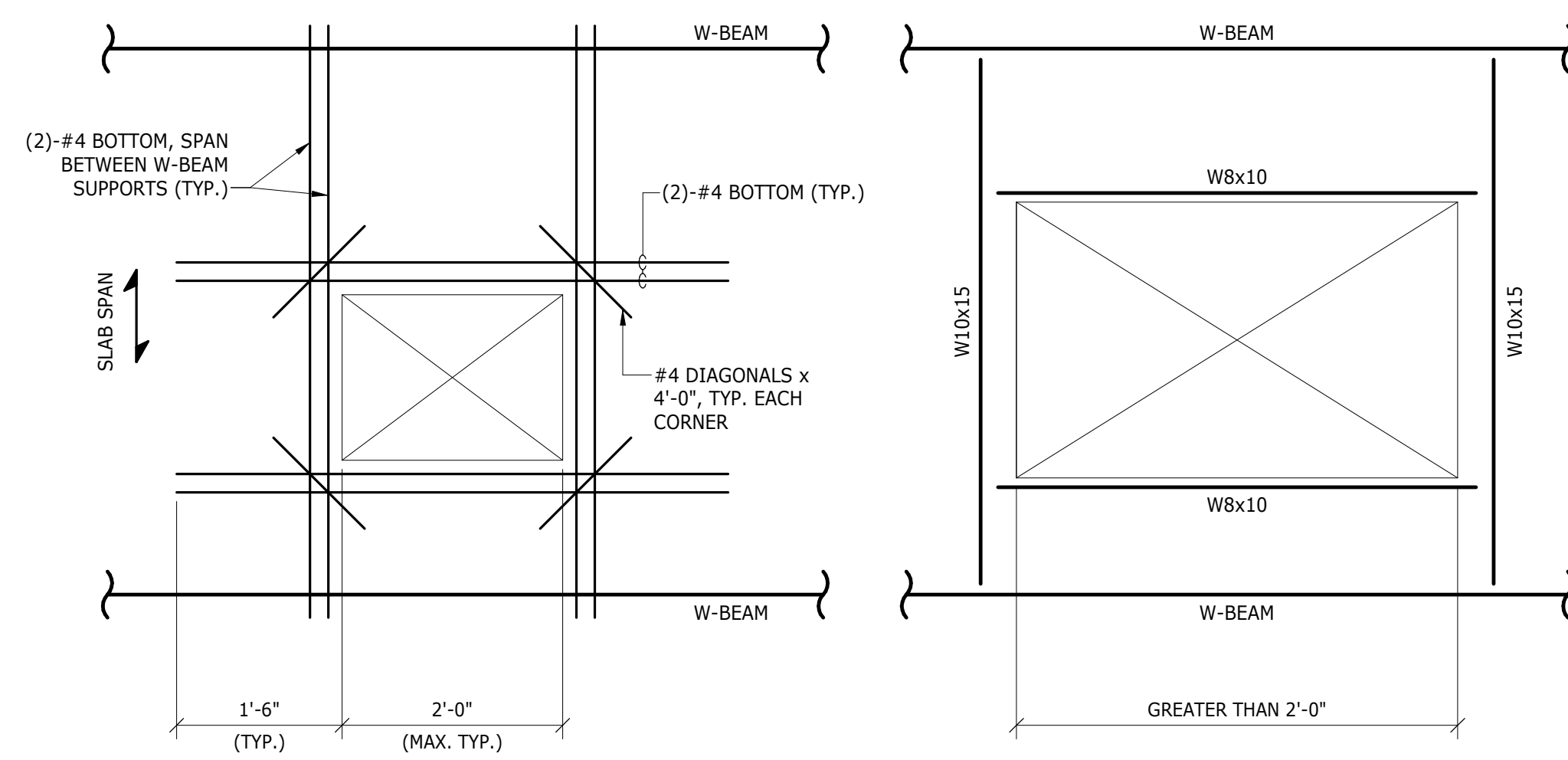
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

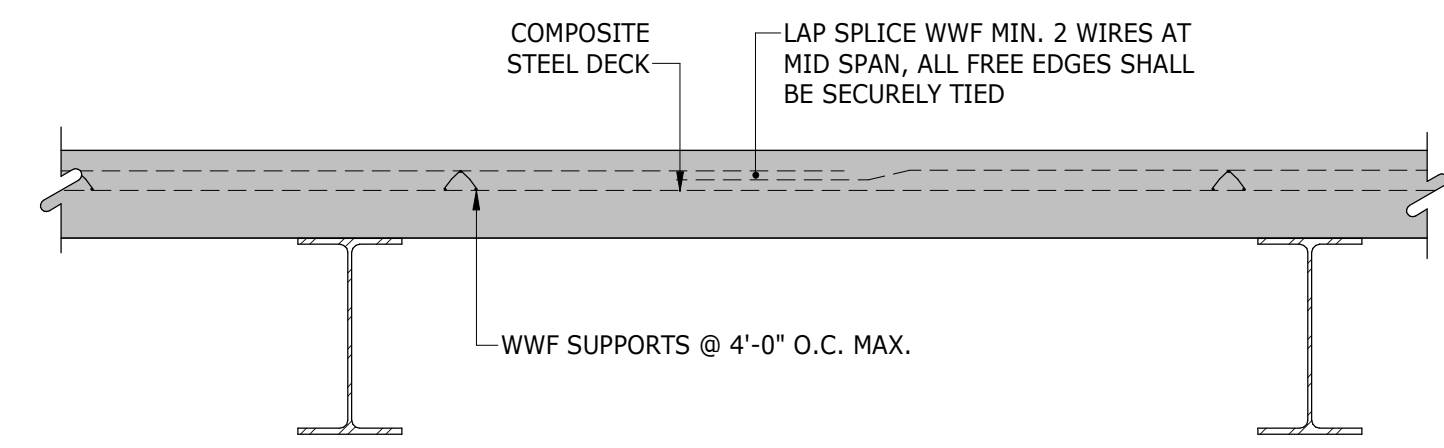
JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

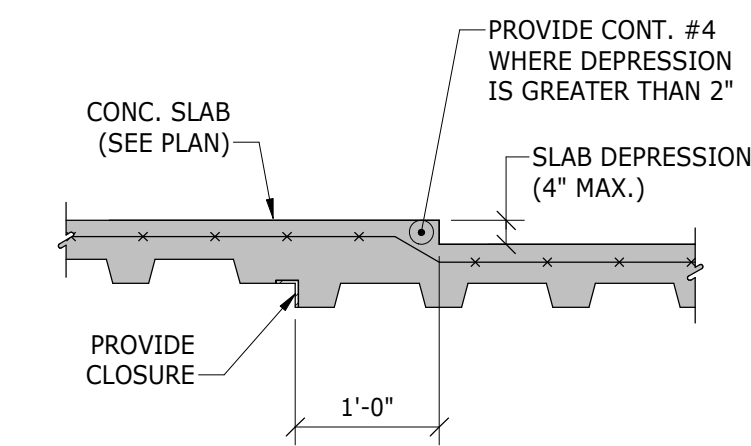
S0.12



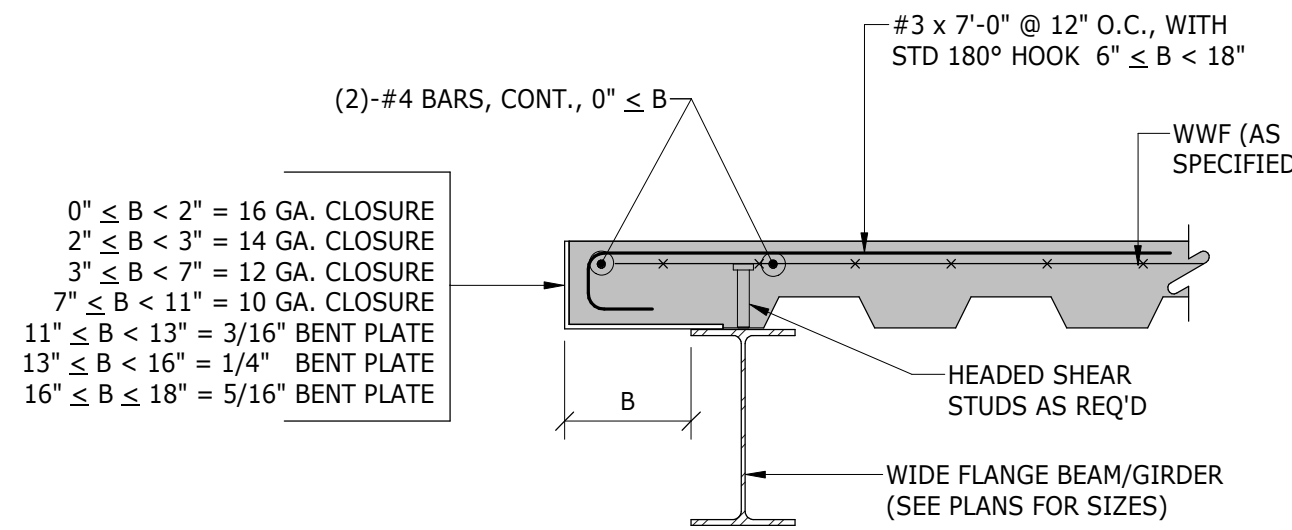
TYPICAL OPENINGS IN SLAB
NOT TO SCALE



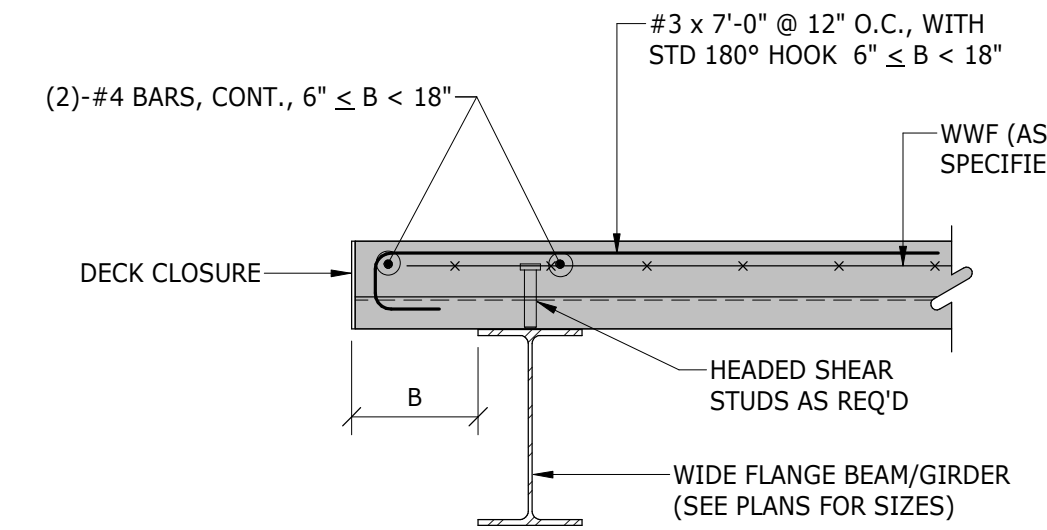
INTERIOR - TYPICAL



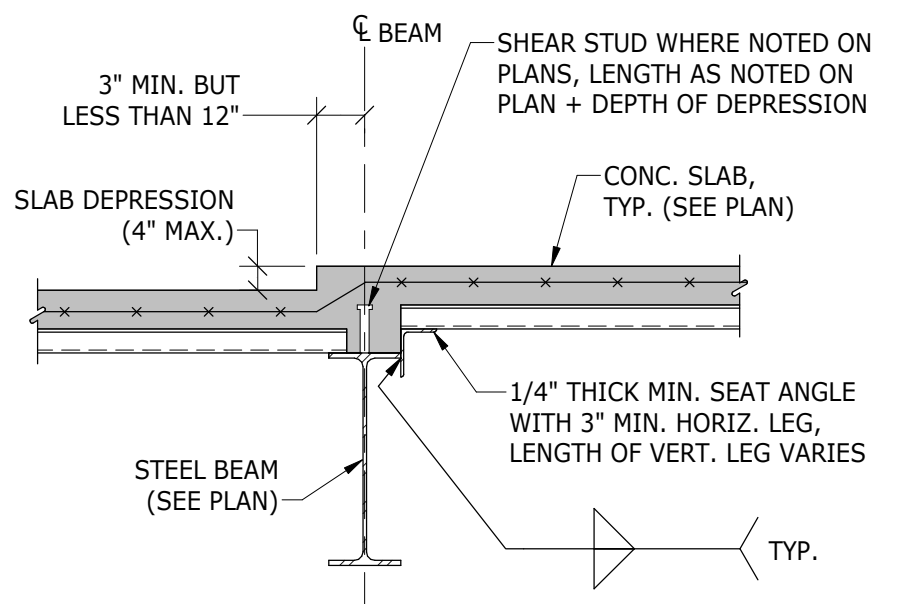
DECK PARALLEL



SLAB EDGE DETAIL
(DECK PARALLEL TO BEAM SPAN)

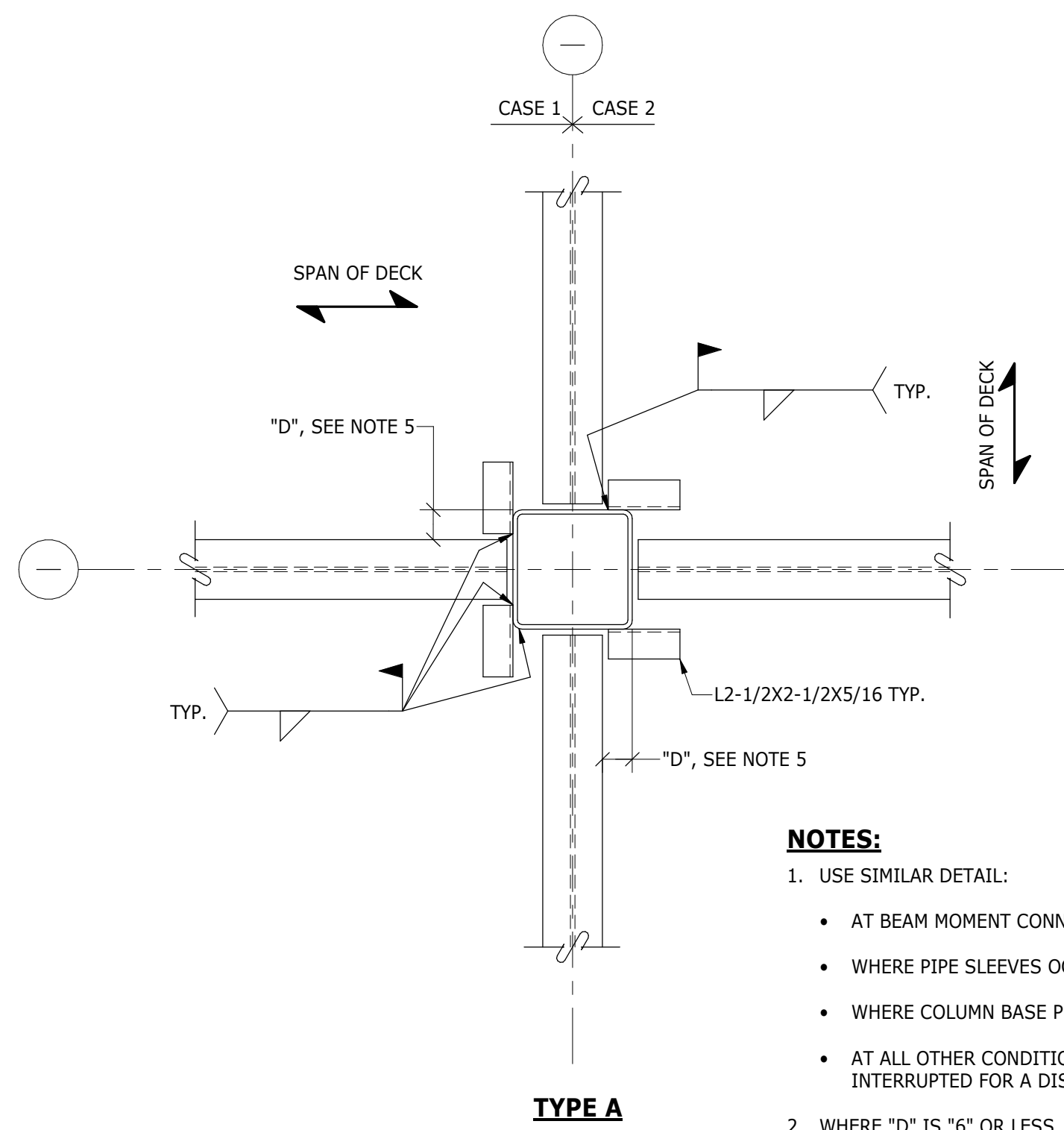


SLAB EDGE DETAIL
(DECK PERPENDICULAR TO BEAM SPAN)

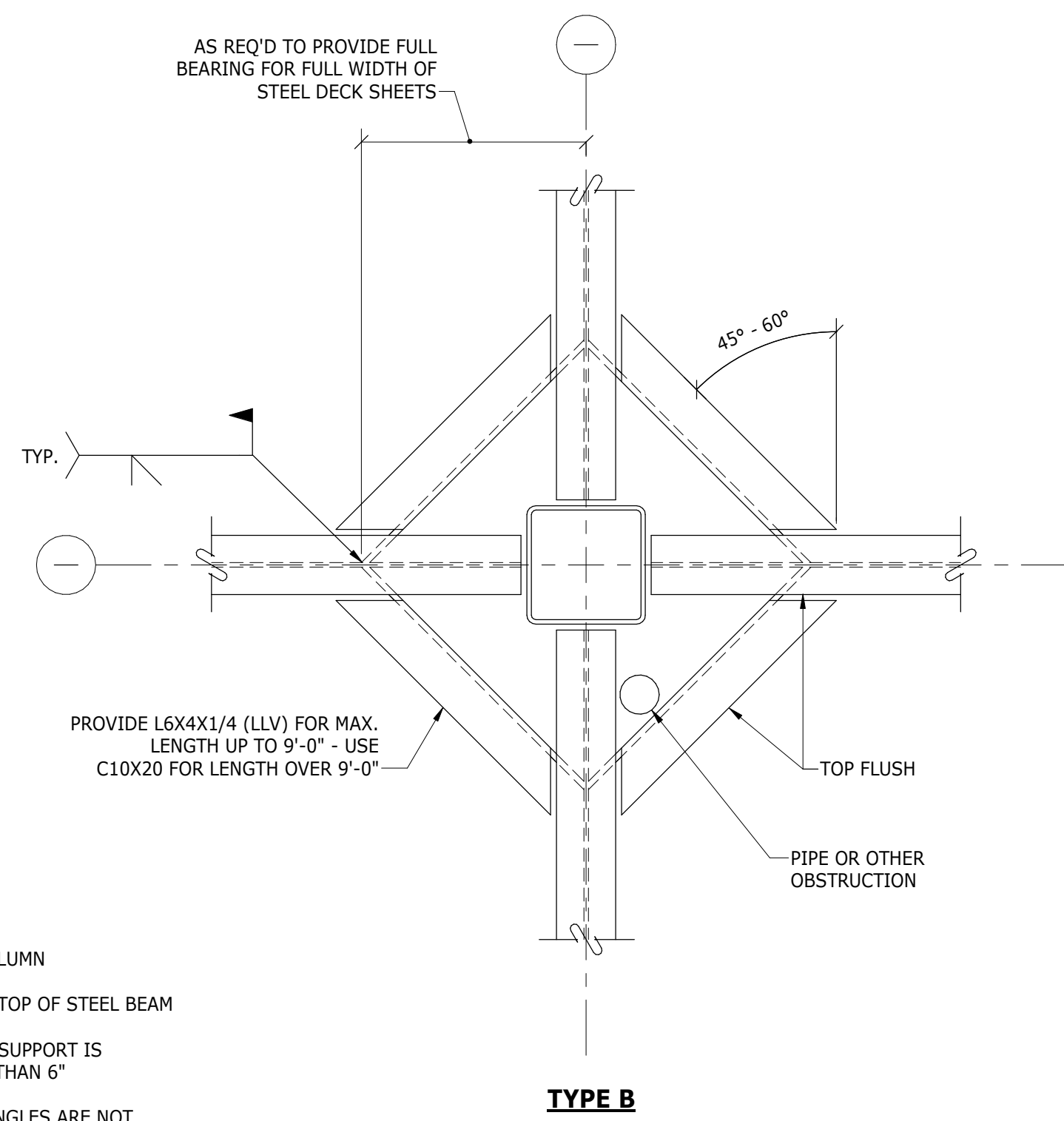


DECK PERPENDICULAR

TYPICAL STEEL DECK AT CHANGES IN SLAB ELEVATIONS OR SLAB THICKNESS DETAIL
NOT TO SCALE



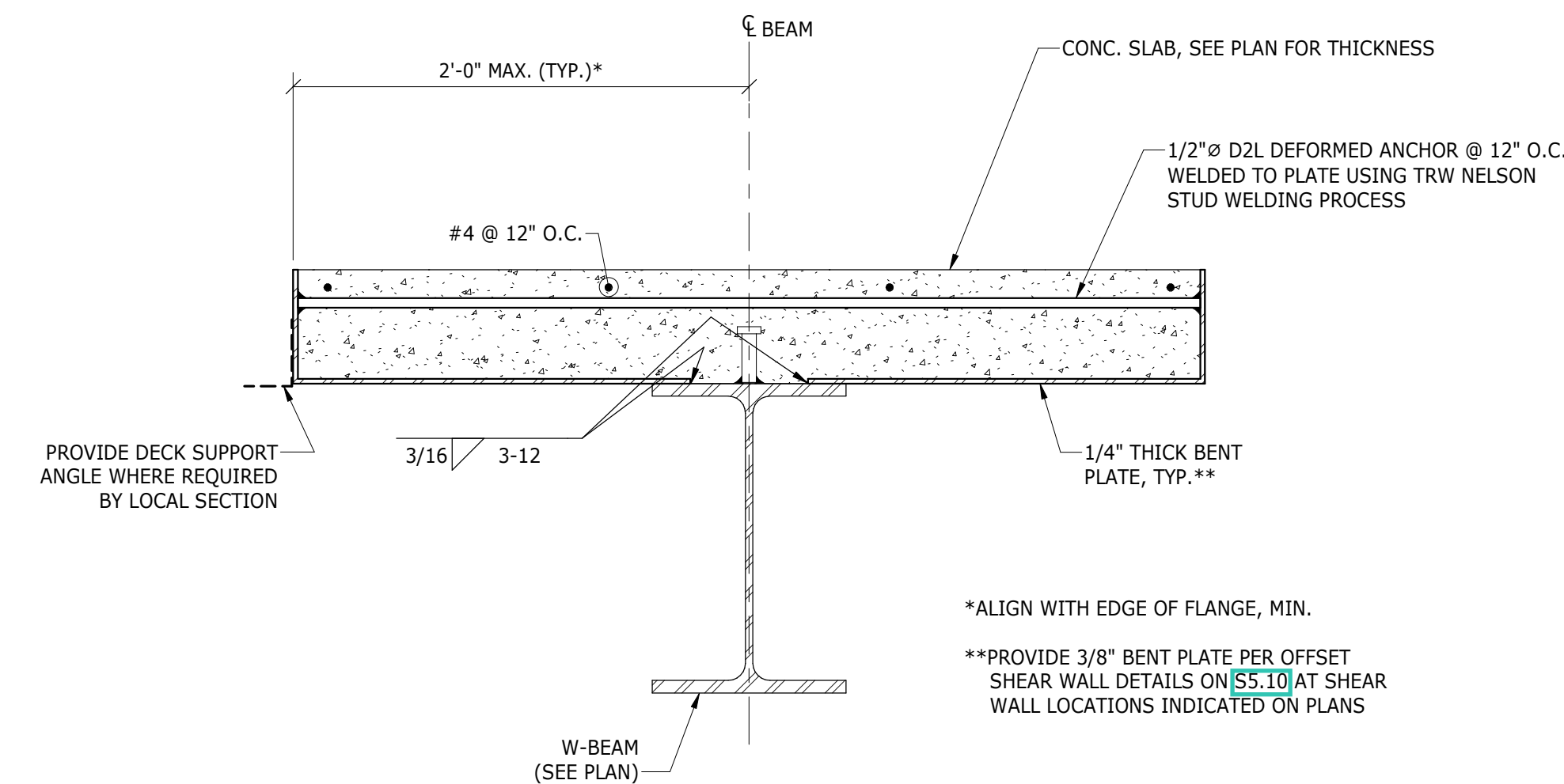
TYPE A



TYPE B

- NOTES:**
- USE SIMILAR DETAIL:
 - AT BEAM MOMENT CONNECTION
 - WHERE PIPE SLEEVES OCCUR NEXT TO COLUMN
 - WHERE COLUMN BASE PLATE OCCURS ON TOP OF STEEL BEAM
 - AT ALL OTHER CONDITIONS WHERE DECK SUPPORT IS INTERRUPTED FOR A DISTANCE GREATER THAN 6"
 - WHERE "D" IS "6" OR LESS, DECK SUPPORT ANGLES ARE NOT REQUIRED EXCEPT AS NEEDED AT DECK PERIMETER LOCATIONS WHERE REQ'D TO SUPPORT POUR STOPS, ETC.
 - HSS COLUMNS SHOWN, WIDE FLANGE COLUMNS SIMILAR.

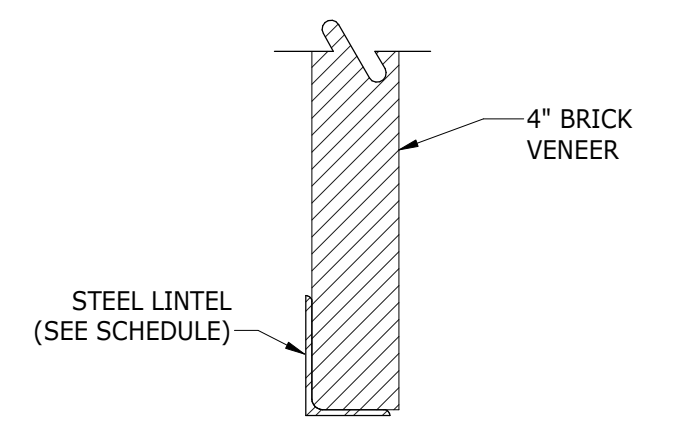
TYPICAL PLAN OF STEEL DECK SUPPORT DETAIL
NOT TO SCALE



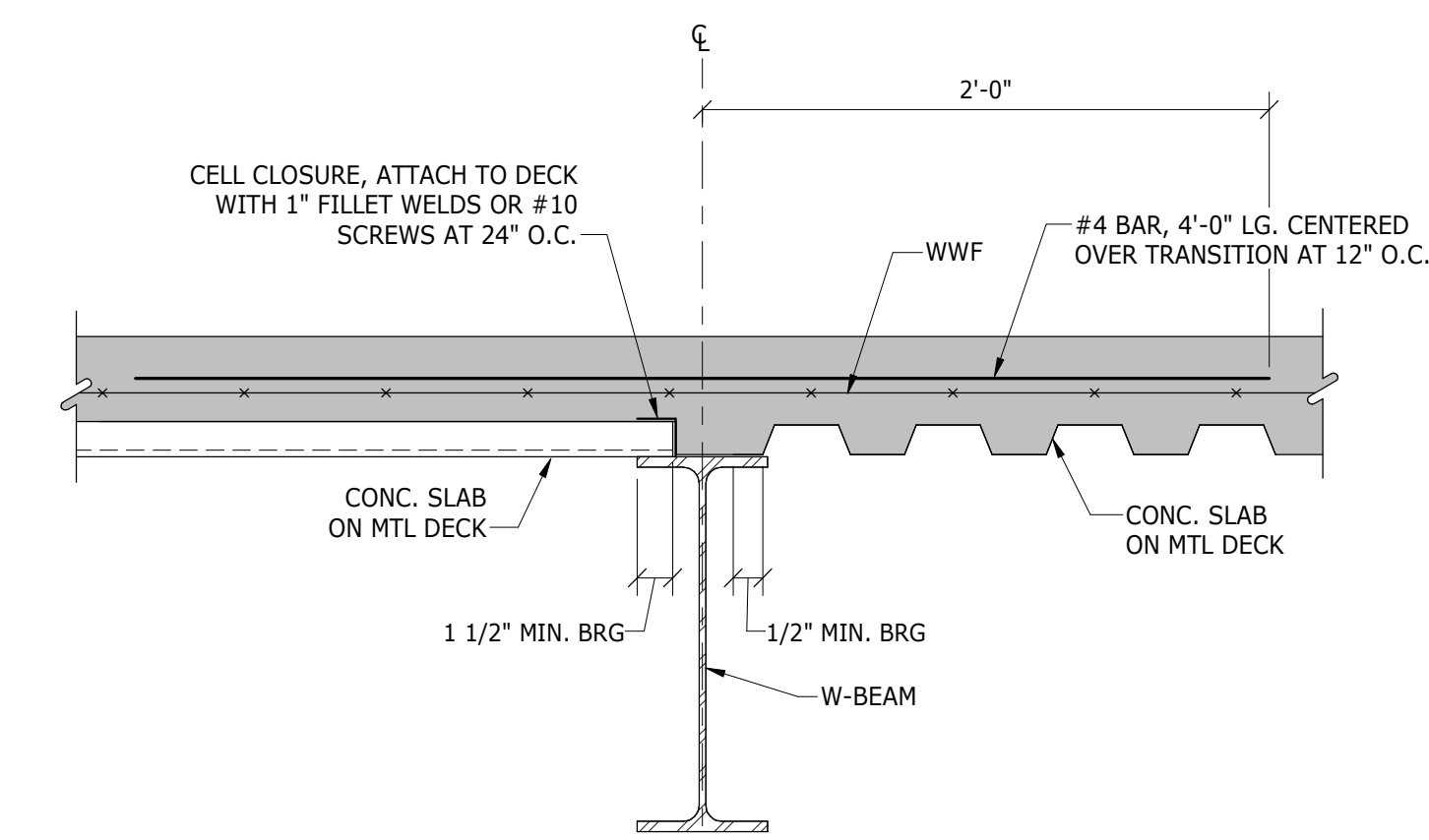
TYPICAL SECTION AT ISOLATED SLAB-ON-DECK
NOT TO SCALE

STEEL LINTEL SCHEDULE	
SPAN	LINTEL
To 4'-0"	L4X3-1/2X1/4
4'-1" To 6'-0"	L6X3-1/2X5/16
6'-1" To 8'-0"	L6X3-1/2X3/8

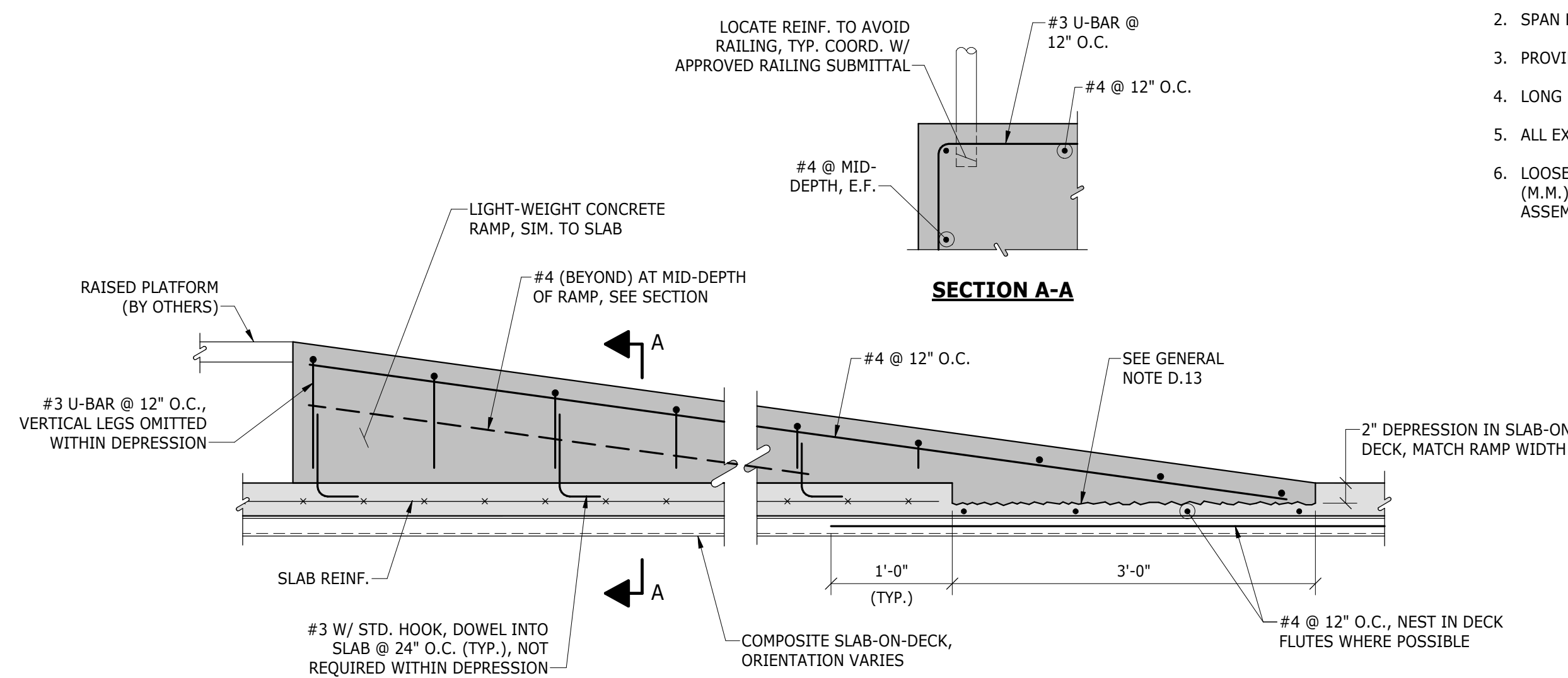
- NOTES:**
- SINGLE ANGLE SIZE PER EACH 4" WYTHE OF MASONRY UP TO 8". PROVIDE SIZES SHOWN IN TABLE ABOVE FOR OPENINGS UNLESS INDICATED OTHERWISE ON DRAWINGS.
 - SPAN LENGTH IS CLEAR OPENING.
 - PROVIDE MIN. 8" BEARING EACH END.
 - LONG LEG SET VERTICAL.
 - ALL EXTERIOR ANGLES SHALL BE HOT-DIP GALVANIZED.
 - LOOSE LINTELS SHALL BE FURNISHED BY METAL FABRICATIONS (M.M.) (SPEC. 05 50 00) AND INSTALLED BY UNIT MASONRY ASSEMBLIES (SPEC. 04 20 00).



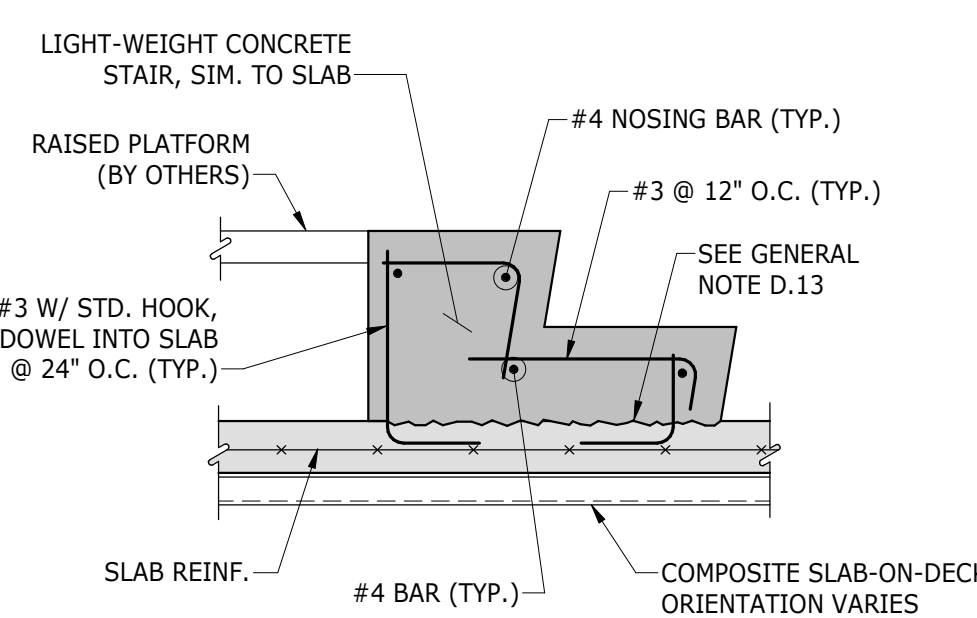
TYPICAL STEEL LINTEL SECTION
NOT TO SCALE



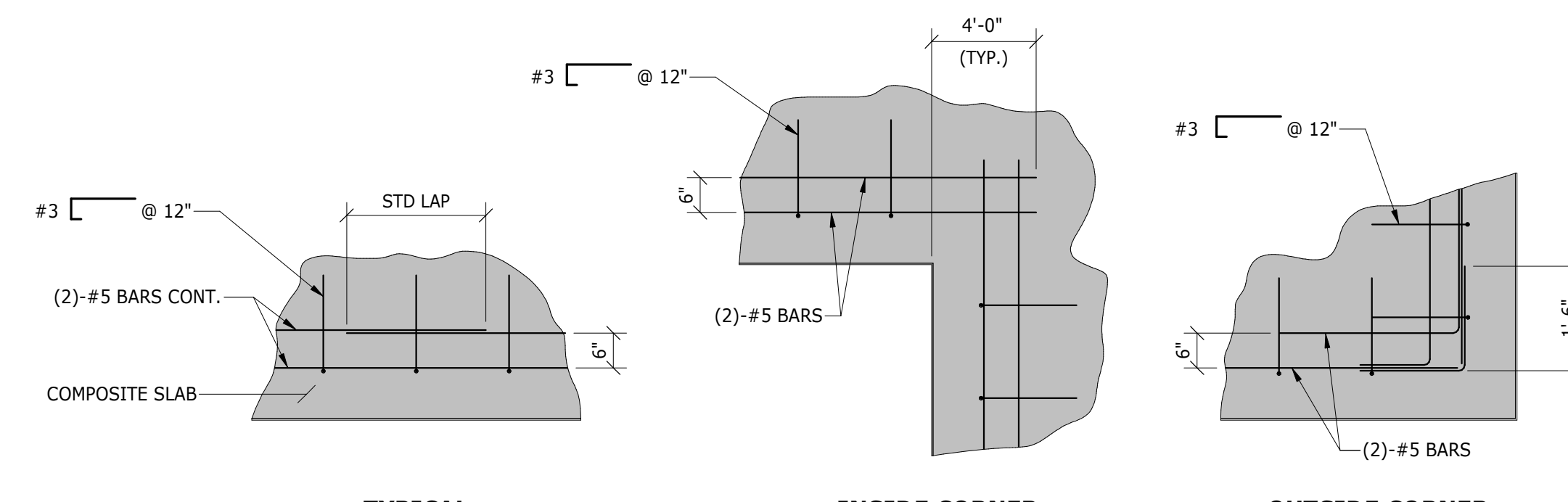
TYPICAL SLAB ON METAL DECK DIRECTION CHANGE DETAIL
NOT TO SCALE



TYPICAL CONCRETE RAMP ON SLAB-ON-DECK
NOT TO SCALE



TYPICAL CONCRETE STAIR ON SLAB-ON-DECK
NOT TO SCALE



TYPICAL COMPOSITE SLAB EDGE REINFORCEMENT
NOT TO SCALE

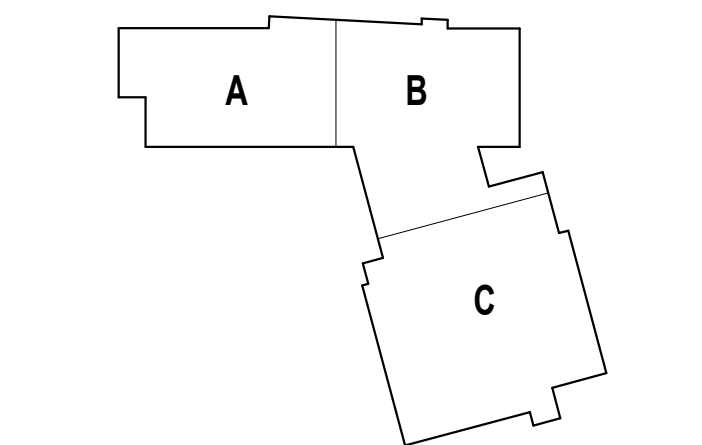


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100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW

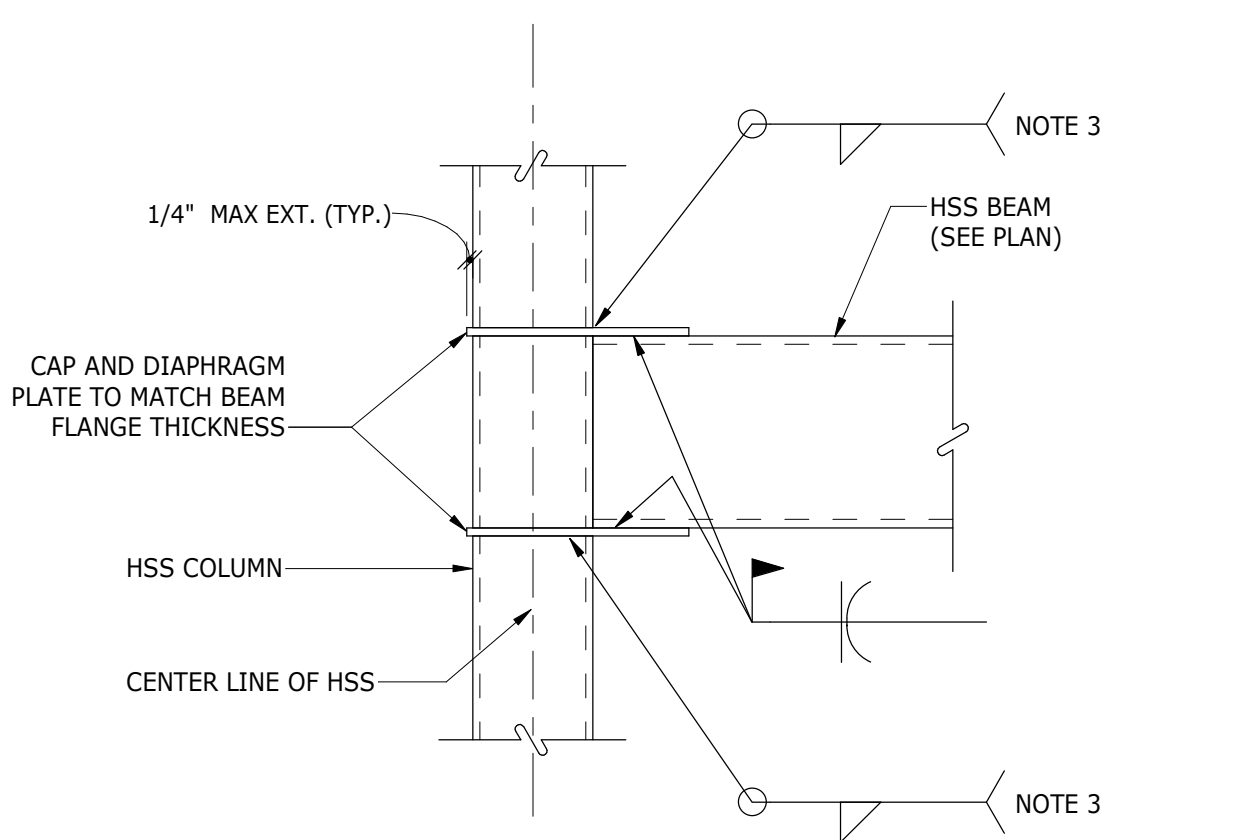
KEYPLAN



DRAWING NAME:

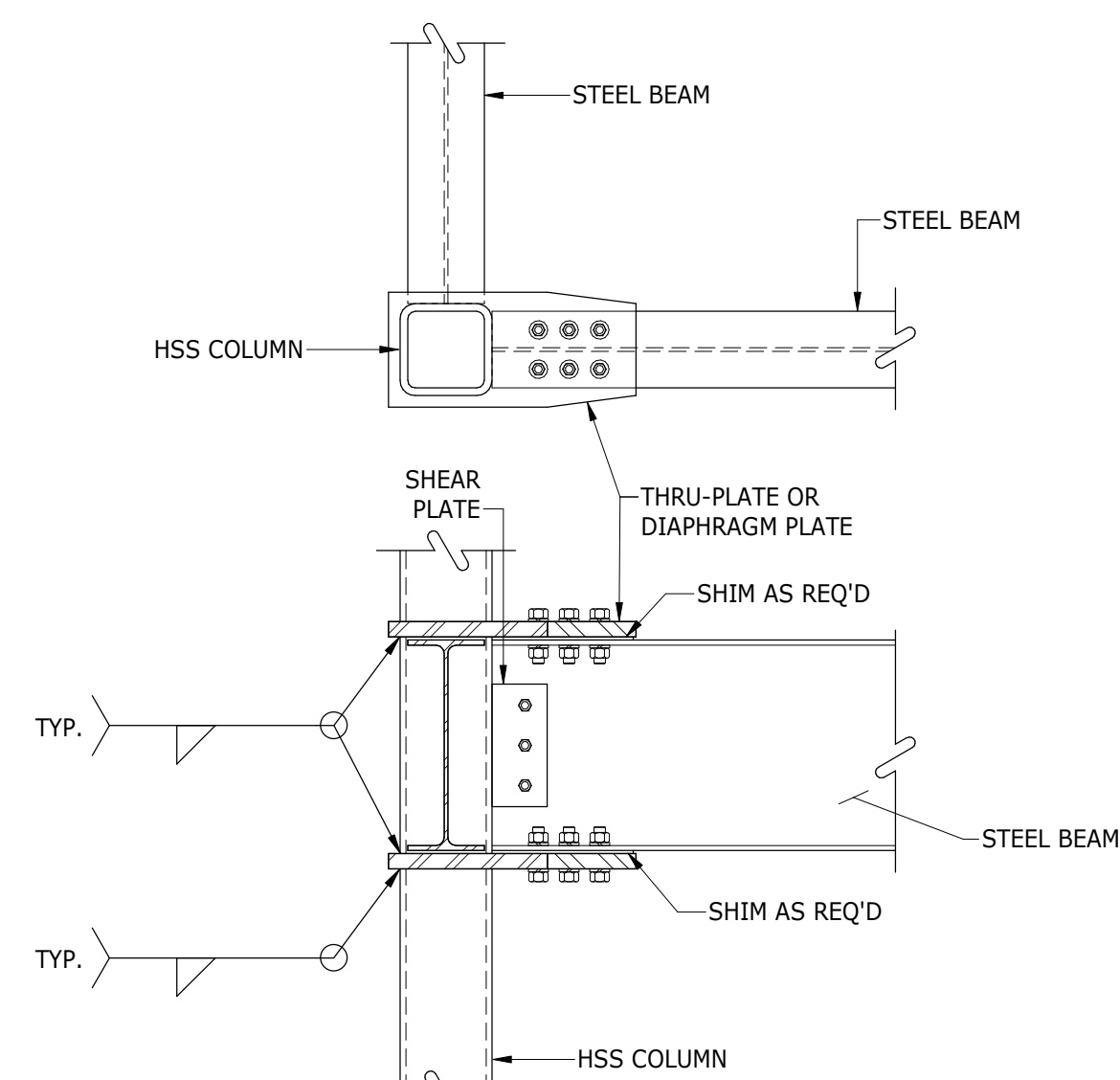
TYPICAL DETAILS - 1

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED / DRAWING NUMBER:
JOB NO.: 2202.02 / **S0.21**
DATE: OCTOBER 13, 2023



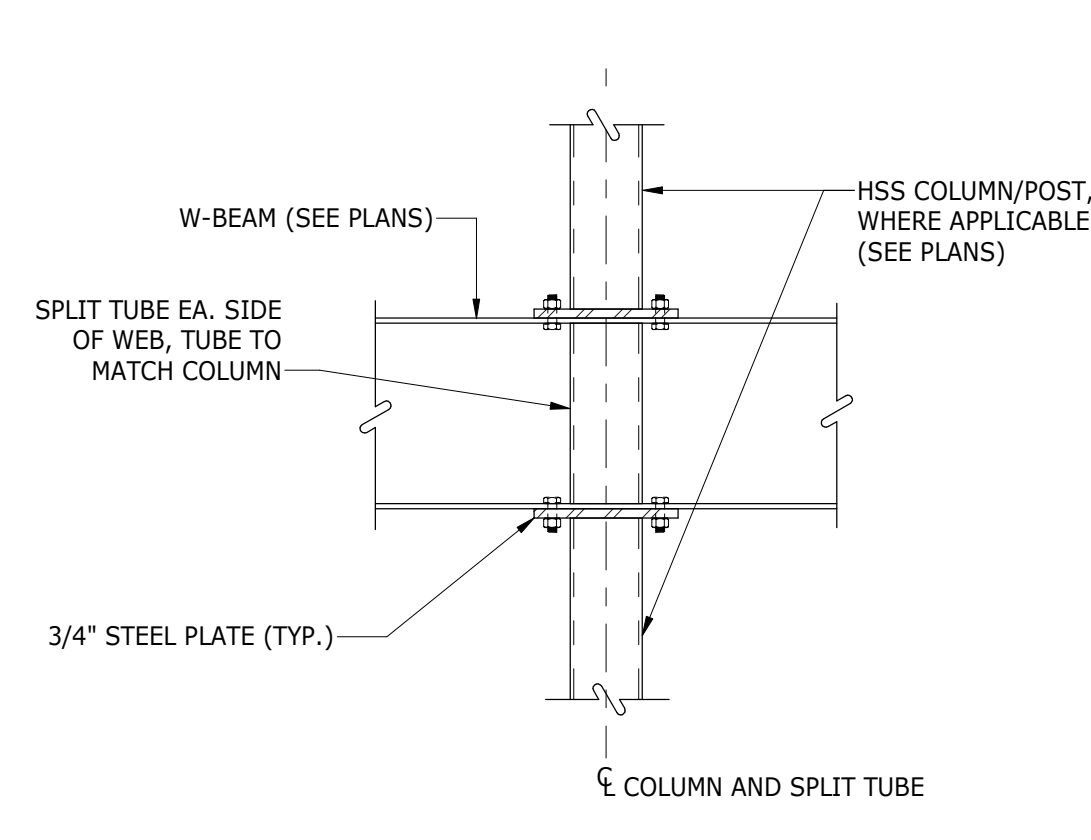
- NOTES:**
- WHERE A DESIGN FORCE IS NOT SPECIFIED ON THE PLANS, THE MOMENT CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL MOMENT CAPACITY OF THE HSS COLUMN.
 - CONNECTIONS SHALL BE DESIGNED BY A P.E. REGISTERED IN RHODE ISLAND. SUBMIT STAMPED DRAWINGS AND CALCULATIONS FOR REVIEW.
 - SEE CONNECTION DESIGN FORCES TABLE FOR SHEAR (DEAD + LIVE) FORCE ACTING ON BEAM MEMBER.
 - FORCES ARE PROVIDED AT SERVICE LEVEL (ASD).

HSS BEAM TO HSS COLUMN MOMENT CONNECTION DETAIL
NOT TO SCALE

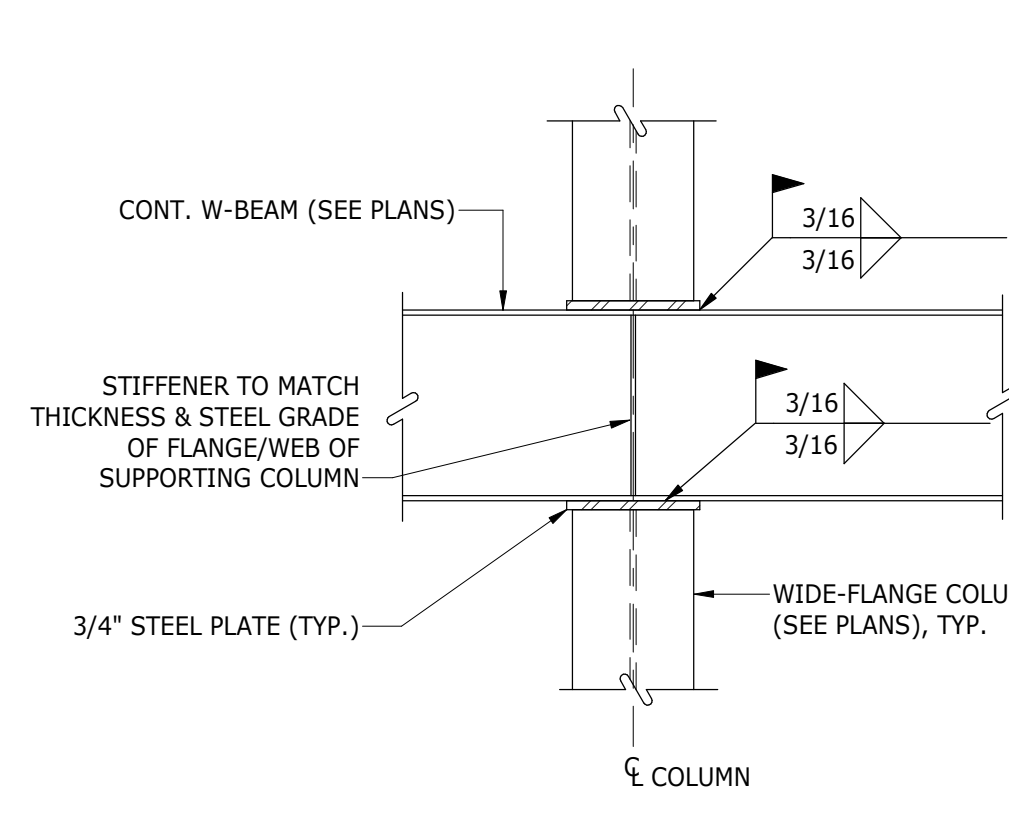


- NOTES:**
- IF A DESIGN FORCE IS NOT SPECIFIED ON THE PLANS, THE MOMENT CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL MOMENT CAPACITY OF THE HSS COLUMN.
 - CONNECTIONS SHALL BE DESIGNED BY A P.E. REGISTERED IN RHODE ISLAND. SUBMIT STAMPED DRAWINGS AND CALCULATIONS FOR REVIEW.
 - SEE CONNECTION FORCE TABLE FOR SHEAR (DEAD AND LIVE) FORCE ACTING ON BEAM MEMBER.
 - FORCES ARE PROVIDED AT SERVICE-LEVEL (ASD).

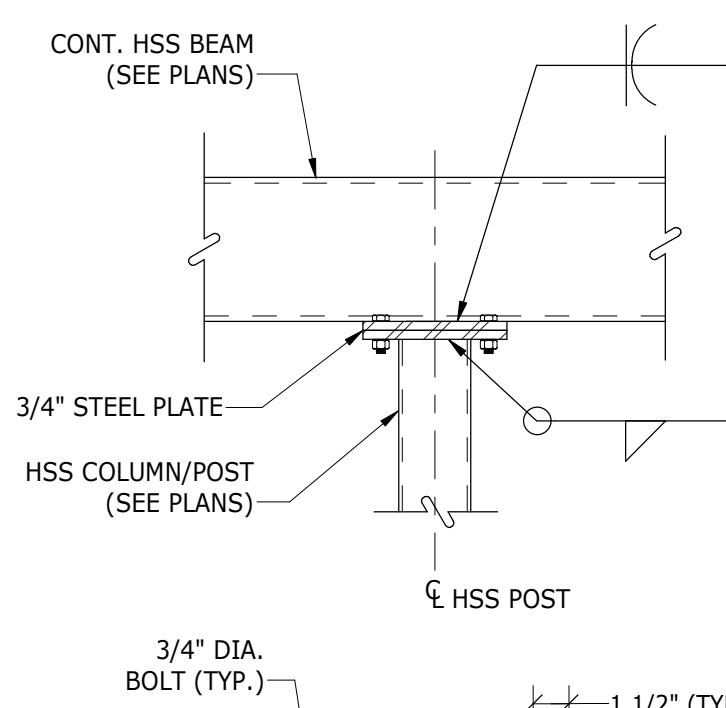
WIDE FLANGE BEAM TO HSS COLUMN MOMENT CONNECTION DETAIL
NOT TO SCALE



HSS COLUMN



WIDE-FLANGE COLUMN



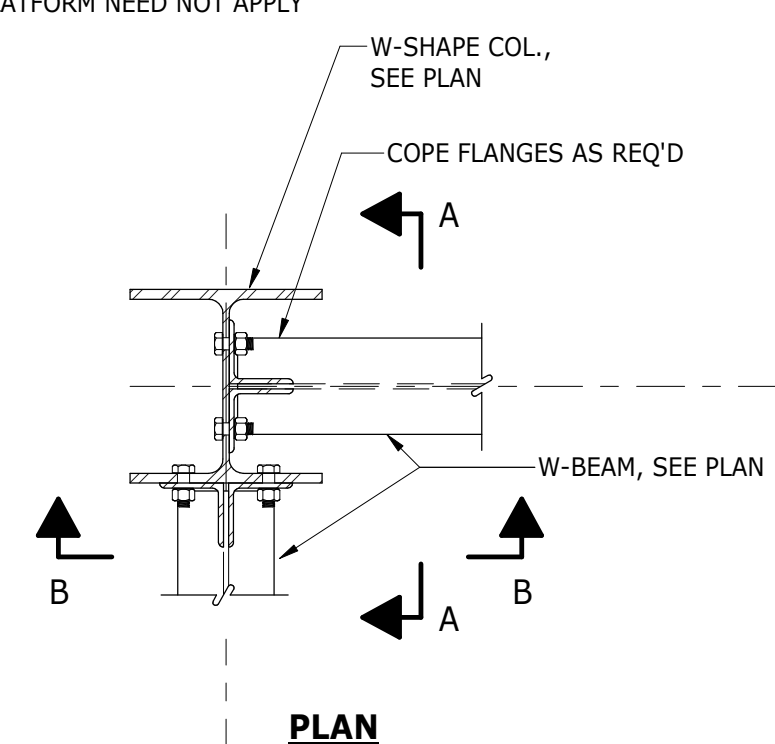
TYPICAL BOLTED HSS CONTINUOUS BEAM CONNECTION DETAIL
NOT TO SCALE

CONNECTION DESIGN FORCES (UNLESS OTHERWISE NOTED ON FRAMING PLANS)		
SHAPE RANGE	VERTICAL (KIPS)	MOMENT (KIP-FT) WHERE INDICATED (SEE PLAN)
W10 (TYP.)	10	-
W10X33	25	-
W12	25	-
W14X22-43	30	45
W14X53-68	40	95
W16	40	55
W18	50	65
W18X143	70	300
W21	55	85
W24X55-76	70	265
W24X84-94	110	300
W27 (TYP.)	80	450
W27X129	95	615
W30	100	-
W33	110	-
W36X135	125	-
W36X150-210	145	245
W40	165	380
HSS12 AND UNDER	20	-
HSS14 AND ABOVE	30	100

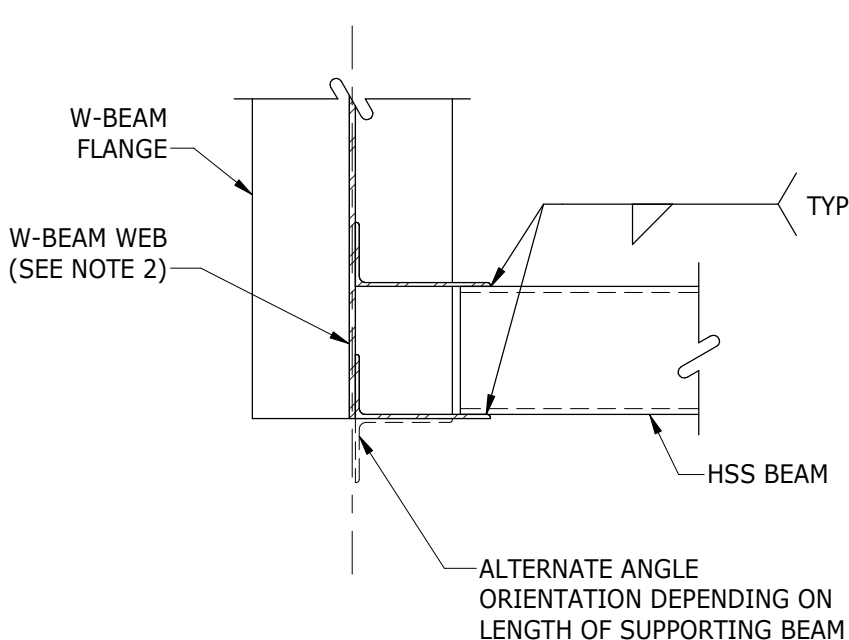
NOTE:
ALL HSS BEAM AND GIRDER CONNECTIONS SHALL BE DESIGNED FOR A HORIZONTAL FORCE (H) AND TORSIONAL LOAD AS FOLLOWS. HORIZONTAL FORCE IS IN ADDITION TO FORCES SHOWN IN TABLE ON THIS SHEET AND ON PLANS AND SHALL BE ATTACHED AT THE TOP AND BOTTOM FOR HORIZONTAL FORCE RESISTANCE. DIVIDE FORCE EQUALLY BETWEEN TOP AND BOTTOM ATTACHMENT (SEE DETAIL, THIS SHEET):

- HSS 12 & UNDER = 15 KIPS (H), 5 K-FT TORSION*
- HSS14 & ABOVE = 20 KIPS (H), 8 K-FT TORSION

*HSS FRAMING AT THIRD FLOOR ROOF PLATFORM NEED NOT APPLY

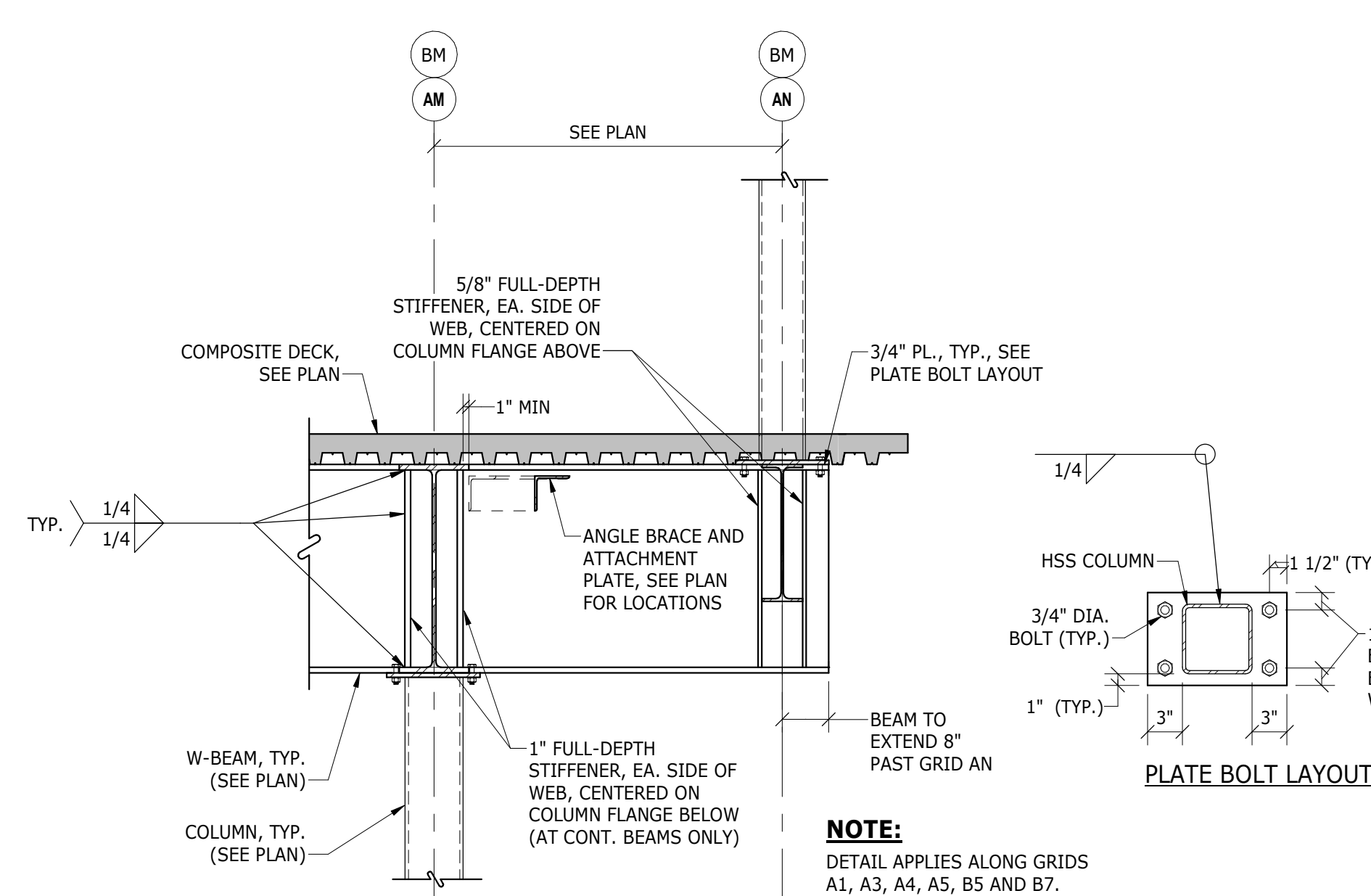


TYPICAL CAP PLATE HSS COLUMNS
NOT TO SCALE

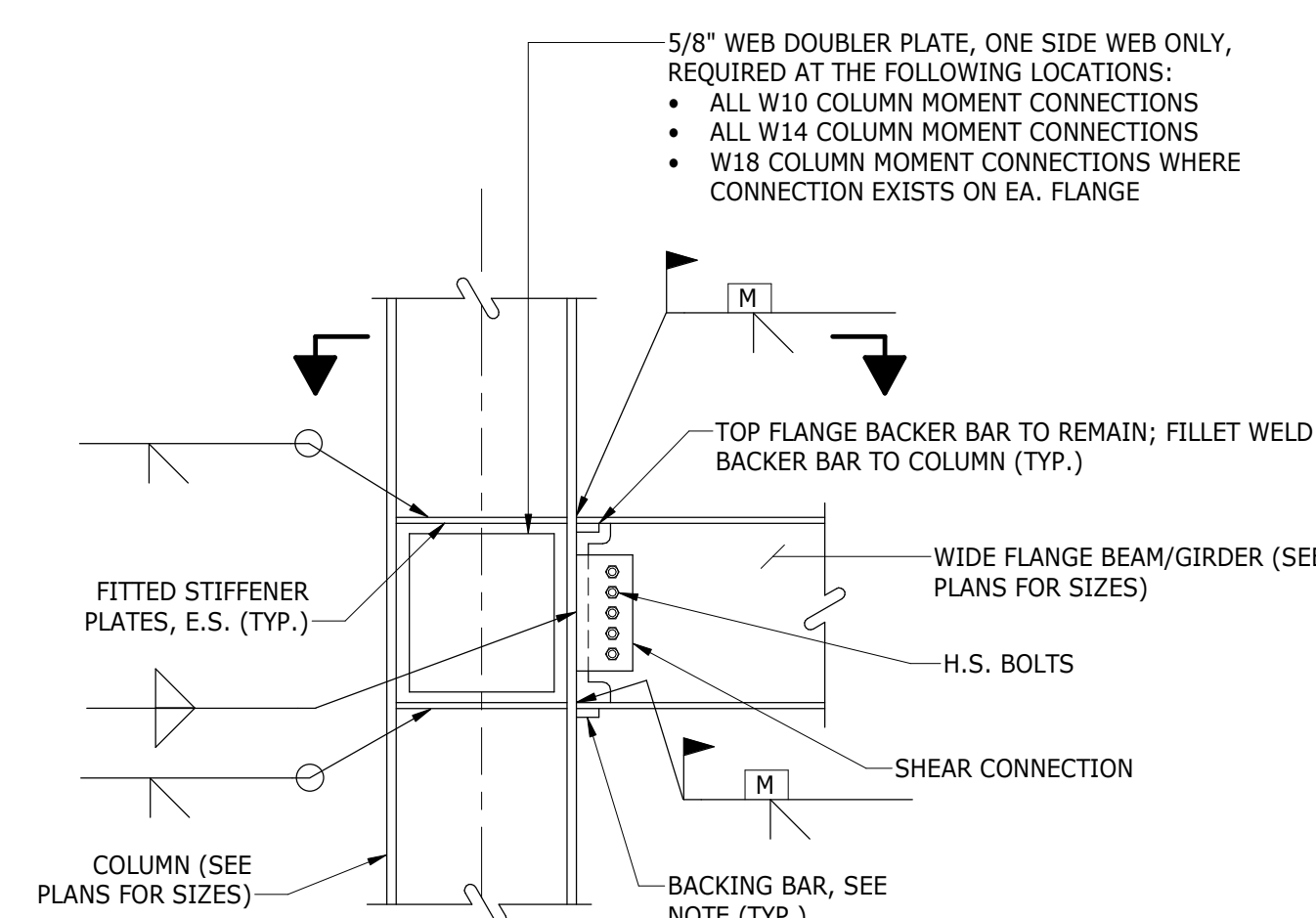


- NOTES:**
- CONTRACTOR IS RESPONSIBLE FOR THE FINAL DESIGN OF CONNECTIONS FOR FORCES SHOWN ON THE DRAWINGS.
 - PROVIDE SIMILAR CONNECTION AT HSS BEAMS TO HSS BEAMS

TYPICAL HSS BEAM TO WIDE-FLANGE OR HSS BEAM CONNECTION (SHEAR)
NOT TO SCALE

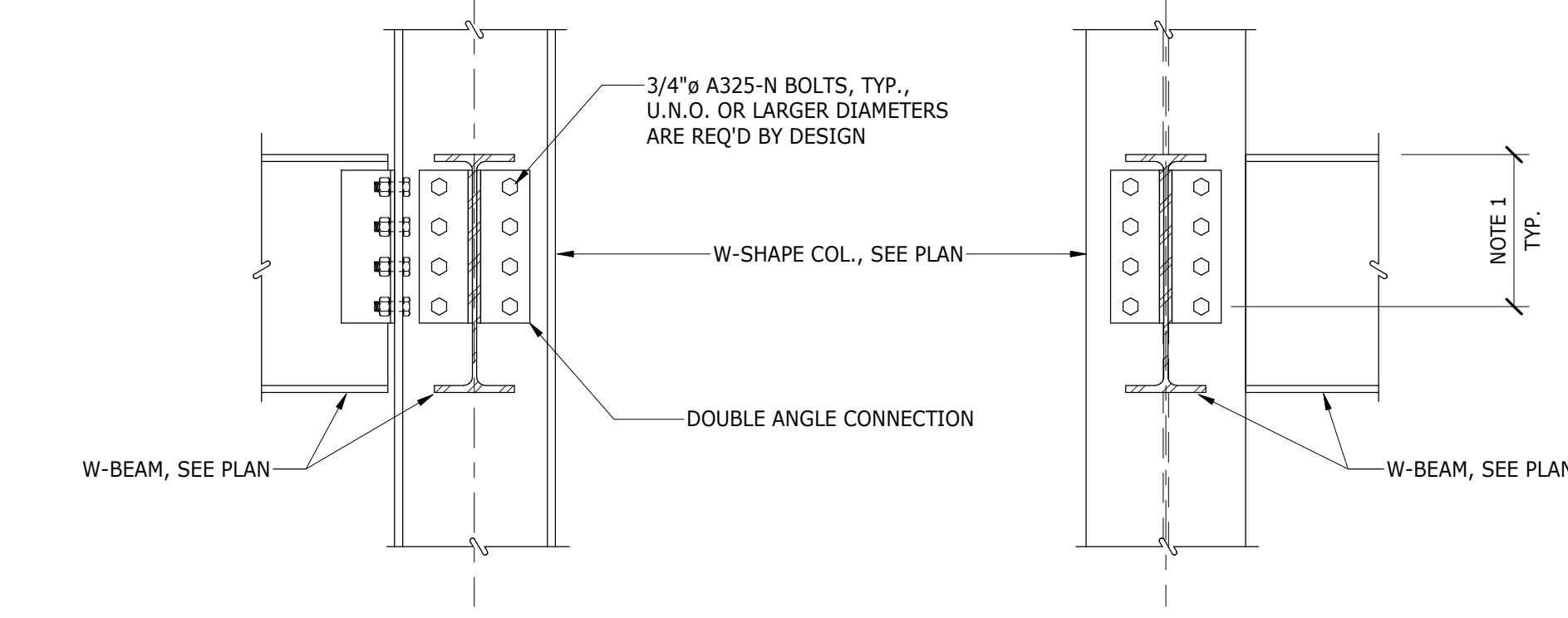


TYPICAL @ CONT. COLUMN
TYPICAL COL. ON BEAM/OUTRIGGER
TYPICAL STIFFENER DETAIL AT SECOND-FLOOR CONTINUOUS BEAMS AND OUTRIGGERS SUPPORTING COLUMNS (SIMILAR)
NOT TO SCALE

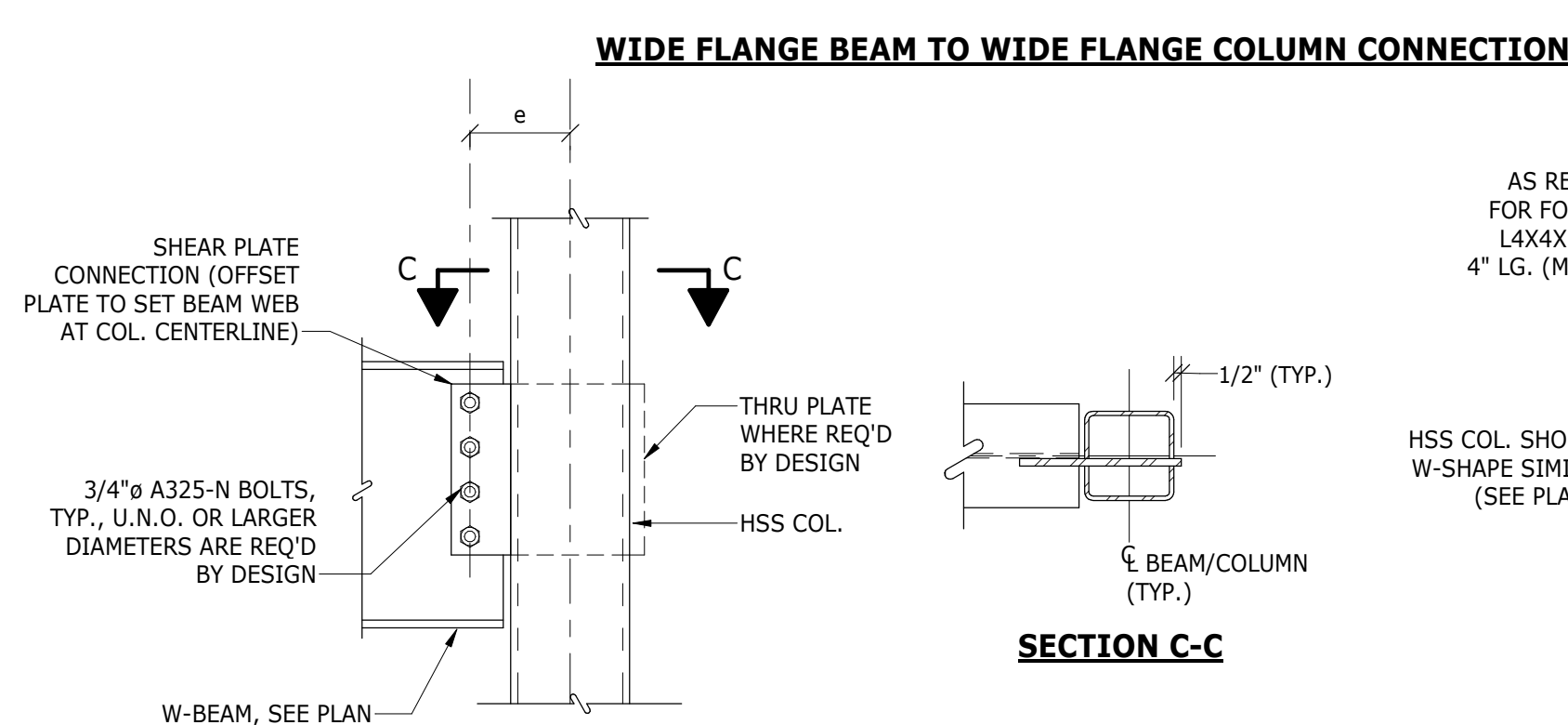


- NOTE:**
- WHERE A DESIGN FORCE IS NOT SPECIFIED ON THE PLANS, THE MOMENT CONNECTIONS SHALL BE DESIGNED TO DEVELOP THE FULL MOMENT CAPACITY OF THE COLUMN.
 - CONNECTIONS SHALL BE DESIGNED BY A P.E. REGISTERED IN RHODE ISLAND. SUBMIT STAMPED DRAWINGS AND CALCULATIONS FOR REVIEW.
 - SEE CONNECTION DESIGN FORCES TABLE FOR SHEAR (DEAD + LIVE) FORCE ACTING ON BEAM MEMBER.
 - FORCES ARE PROVIDED AT SERVICE-LEVEL (ASD).
 - REMOVE BOTTOM FLANGE BACKER BAR AND WELD TABS AFTER WELDING BACK GOUGE AND RE-WELD.

TYPICAL WIDE-FLANGE COLUMN WELDED MOMENT CONNECTION
NOT TO SCALE

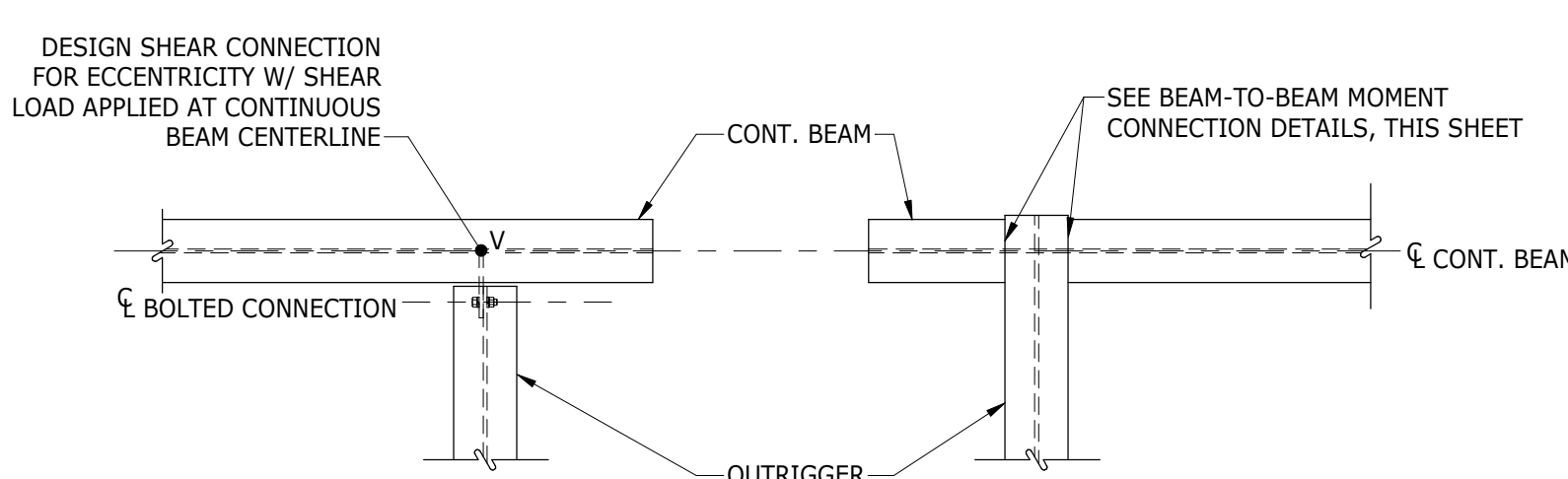


SECTION A-A
SECTION B-B



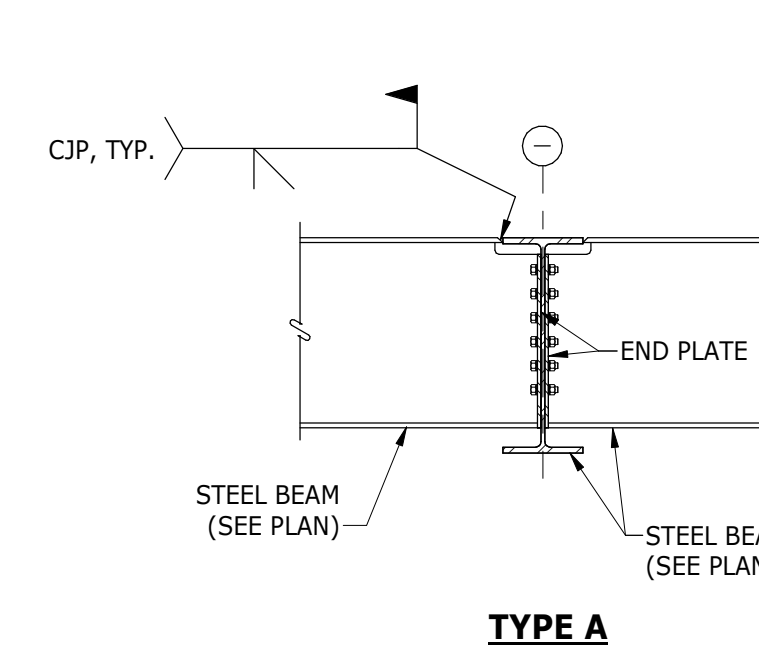
- NOTES:**
- SPACING OF BOLTS SHALL EXTEND AT LEAST TO THE MID-DEPTH OF THE BEAM.
 - CONNECTIONS SHALL CONFORM TO THE REQUIREMENTS OF PART 9, "CONNECTIONS" OF THE AISC MANUAL OF STEEL CONSTRUCTION AND SHALL BE DESIGNED BY A P.E. REGISTERED IN THE STATE OF RHODE ISLAND. SUBMIT STAMPED DRAWINGS AND CALCULATIONS FOR REVIEW. FORCES ARE PROVIDED AT SERVICE LEVEL (ASD).
 - ALTERNATE BEAM CONNECTIONS MAY BE SUBMITTED BY THE STRUCTURAL STEEL FABRICATOR FOR CONSIDERATION BY THE STRUCTURAL ENGINEER.
 - BOLTS FOR THRU PLATE CONNECTIONS SHALL BE DESIGNED FOR THE ECCENTRICITY e .

MINIMUM REQUIREMENTS FOR BEAM CONNECTIONS
NOT TO SCALE

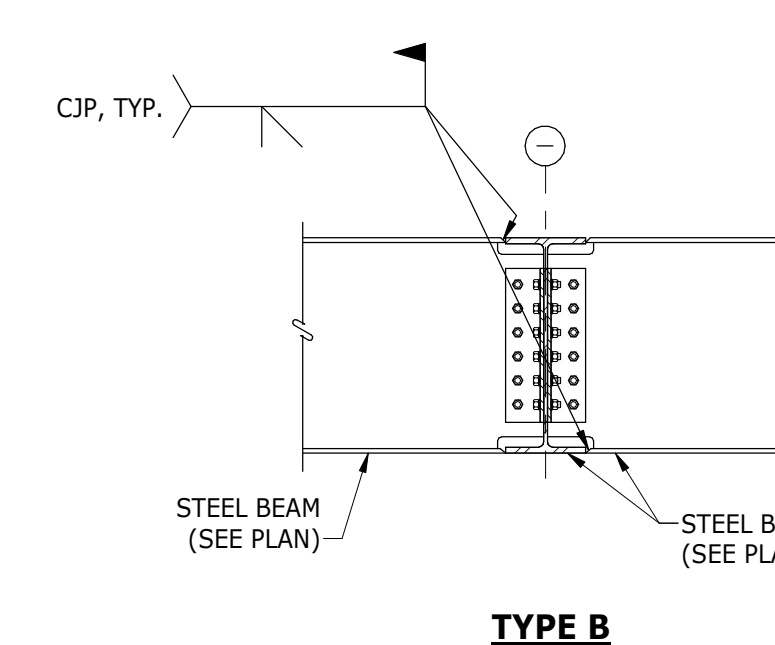


- OPTION A @ SHEAR LOADS 10K OR LESS**
OPTION B ALL OTHER CASES

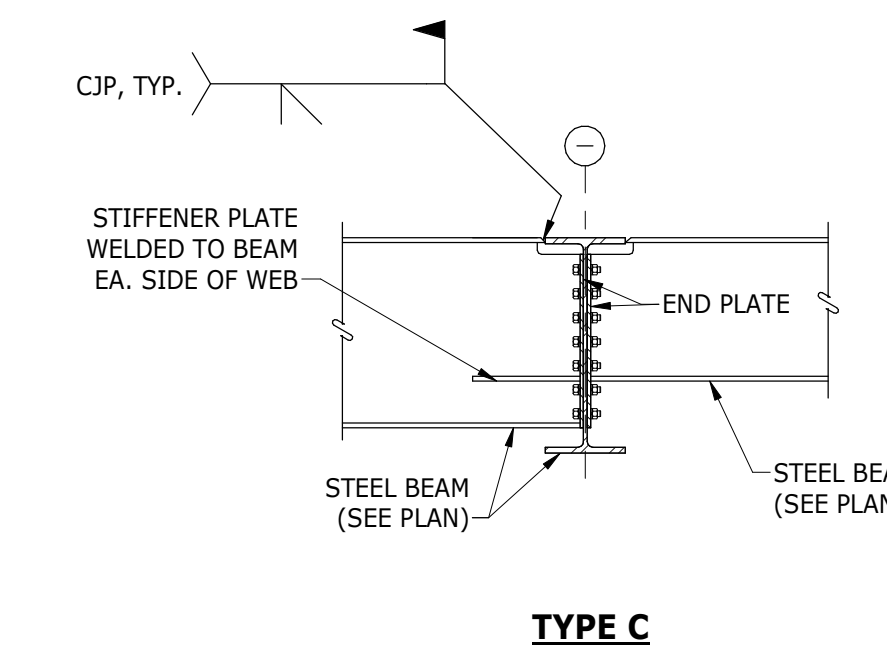
CONTINUOUS BEAM SUPPORTED AT OUTRIGGER END CONNECTION DETAIL
NOT TO SCALE



TYPE A



TYPE B



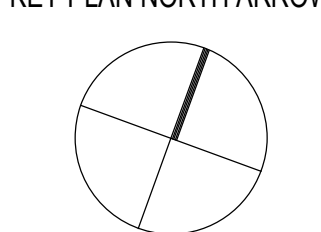
TYPE C

- NOTES:**
- SHEAR CONNECTION SHALL BE DESIGNED FOR THE BEAM END REACTIONS GIVEN ON THE DRAWINGS.
 - IF A DESIGN FORCE IS NOT SPECIFIED ON THE PLANS, DESIGN MOMENT PLATES FOR THE FULL MOMENT CAPACITY OF THE SMALLER BEAM.
 - AT HSS MOMENT CONNECTIONS, PROVIDE SPLICE PLATES IN LIEU OF FLANGE WELDS.

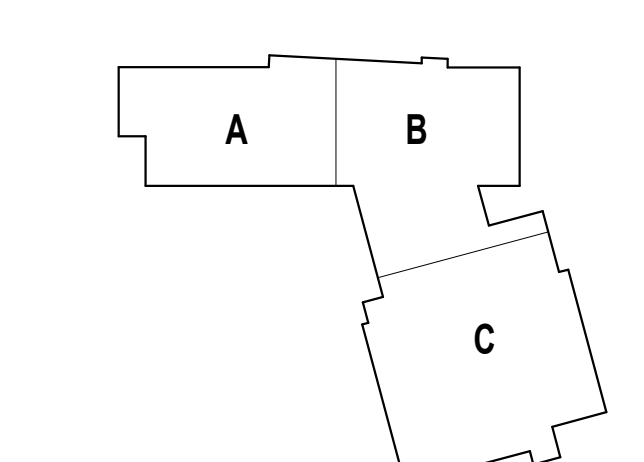
BEAM TO BEAM MOMENT CONNECTION DETAILS
NOT TO SCALE

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

TYPICAL DETAILS - 2

DRAWN BY: JDB / MSS

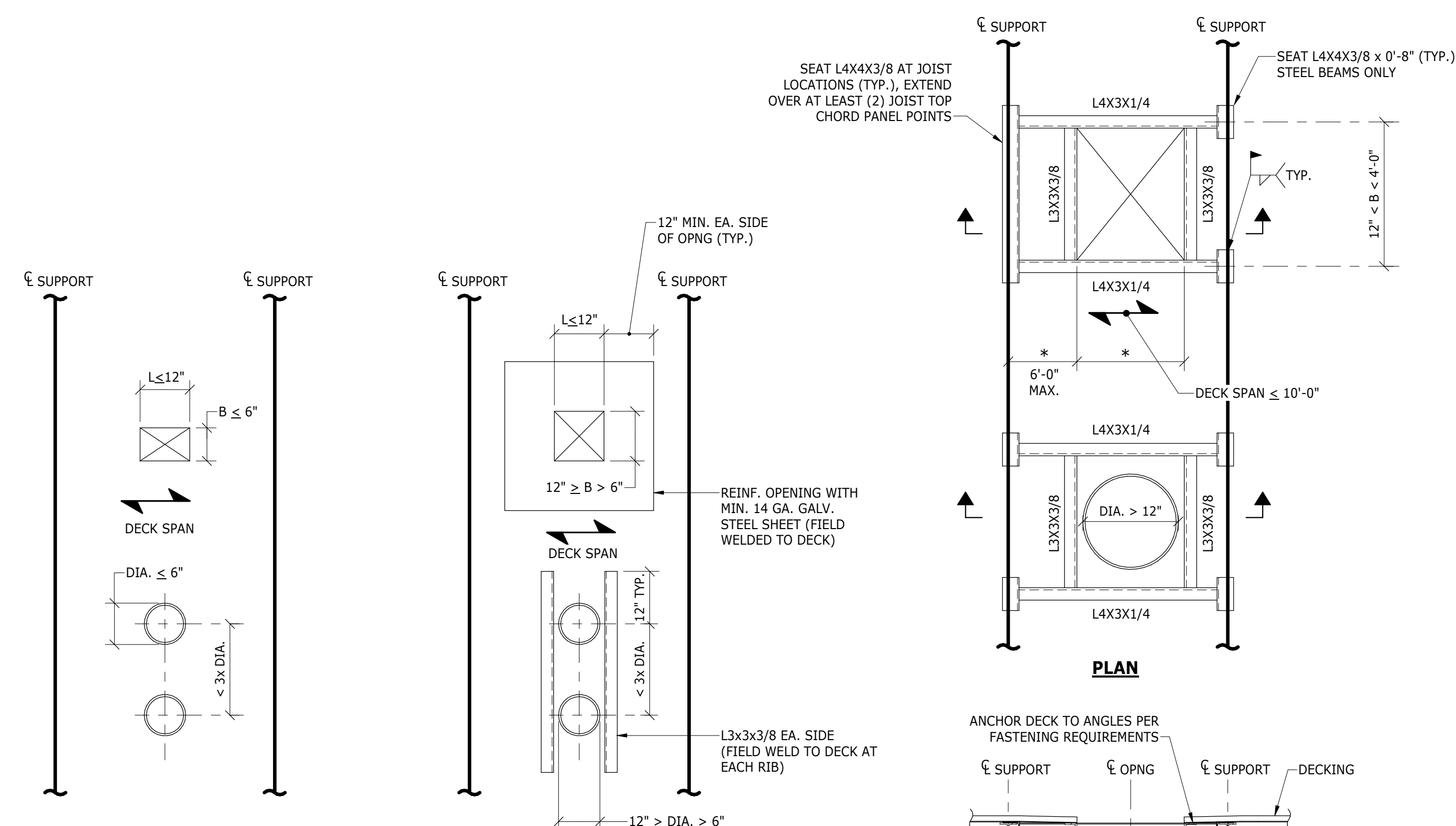
REVIEWED BY: MGM / BP

SCALE: AS INDICATED DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

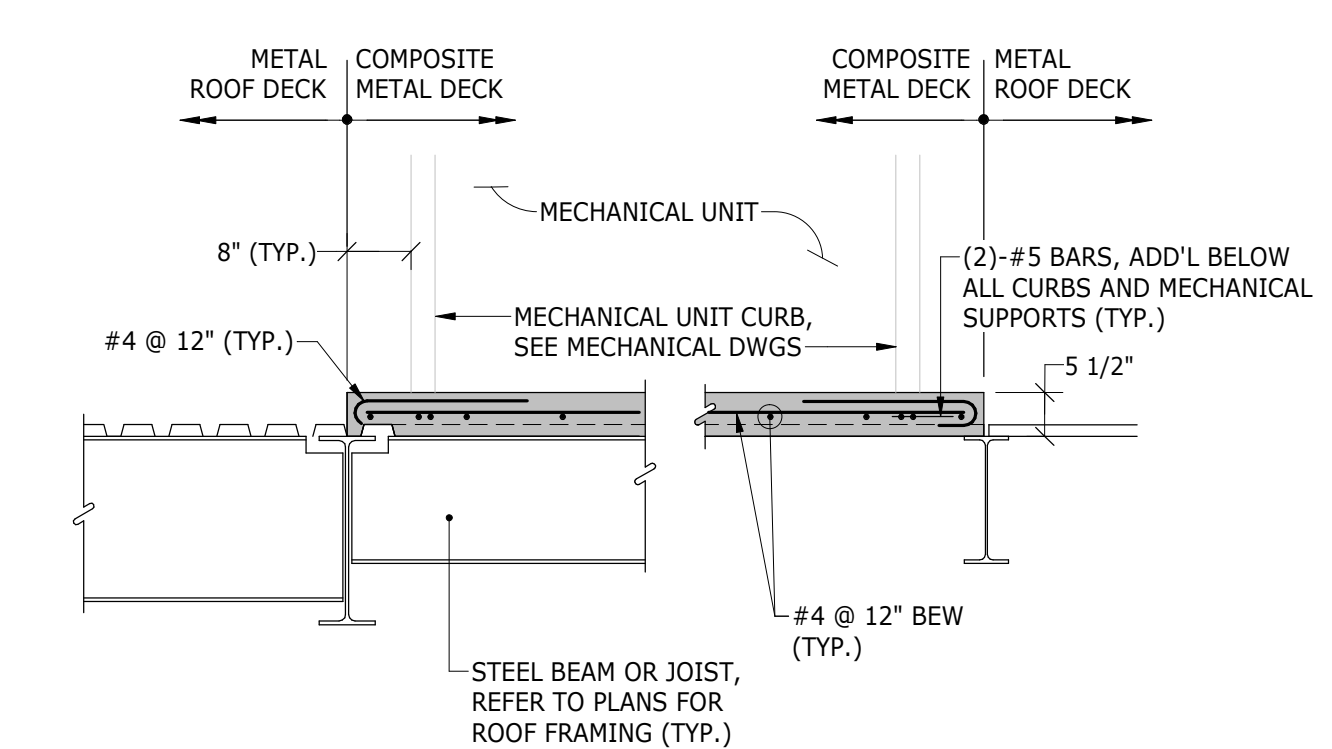
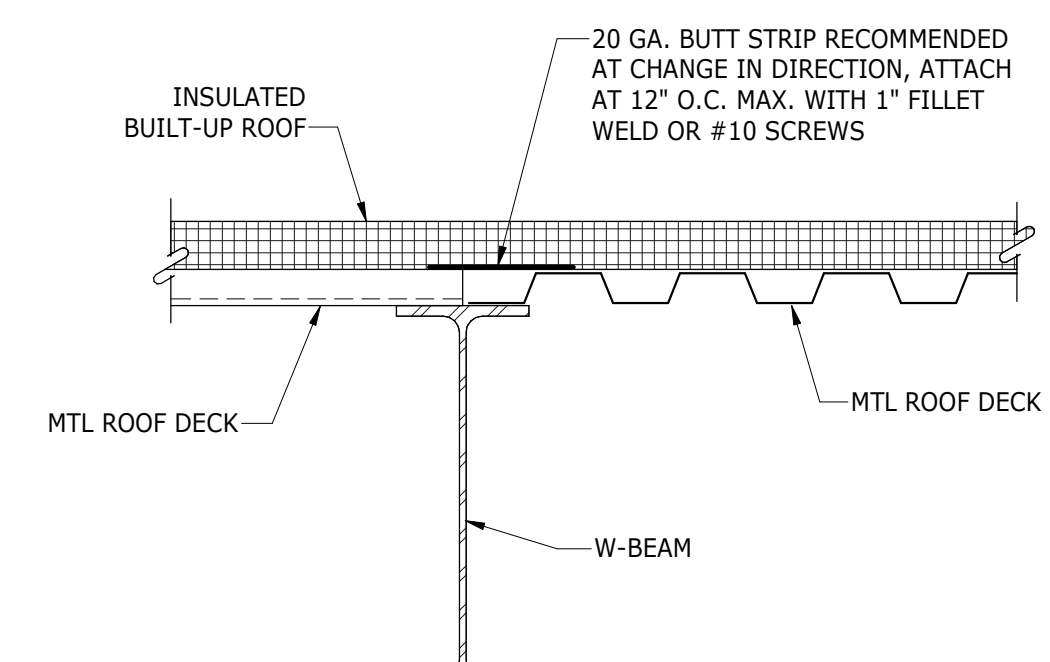
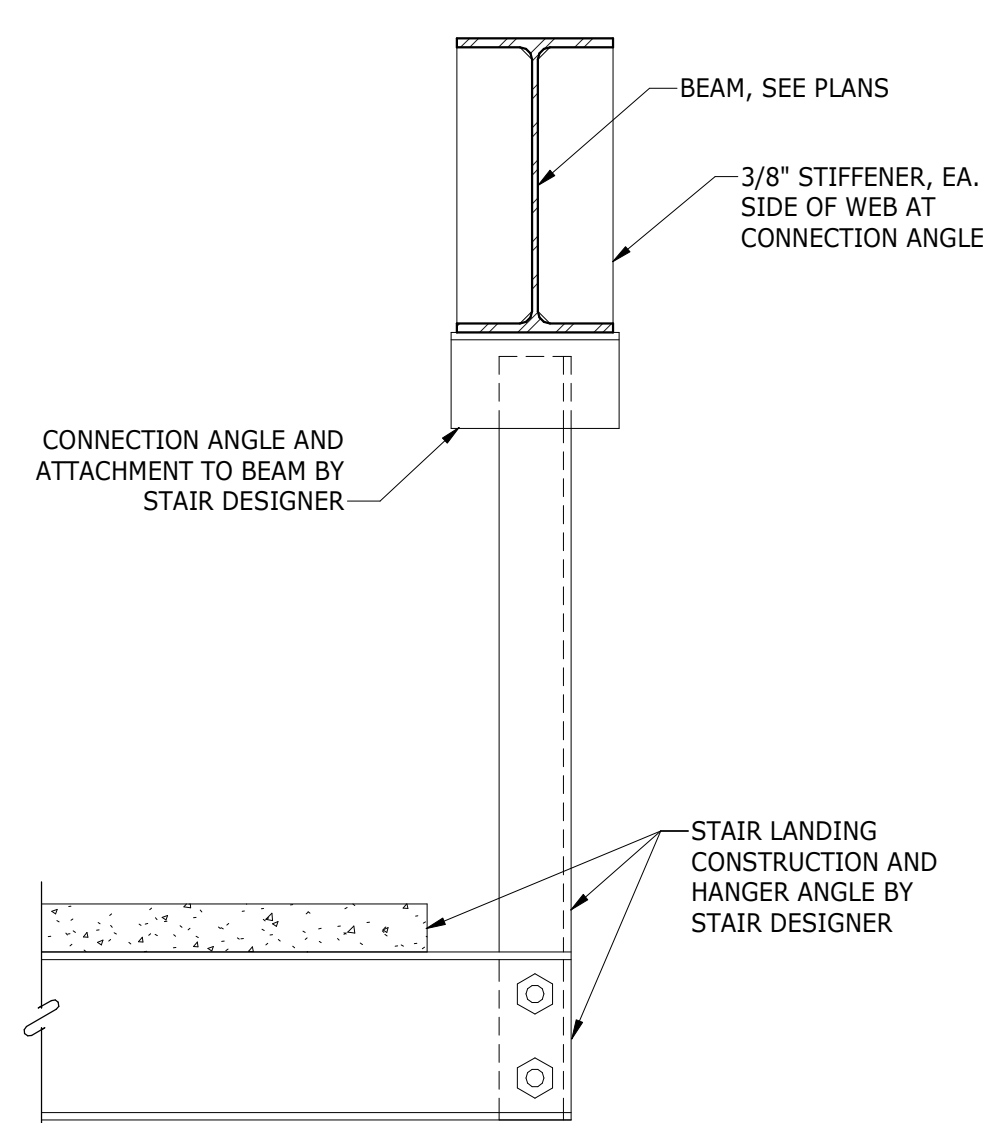
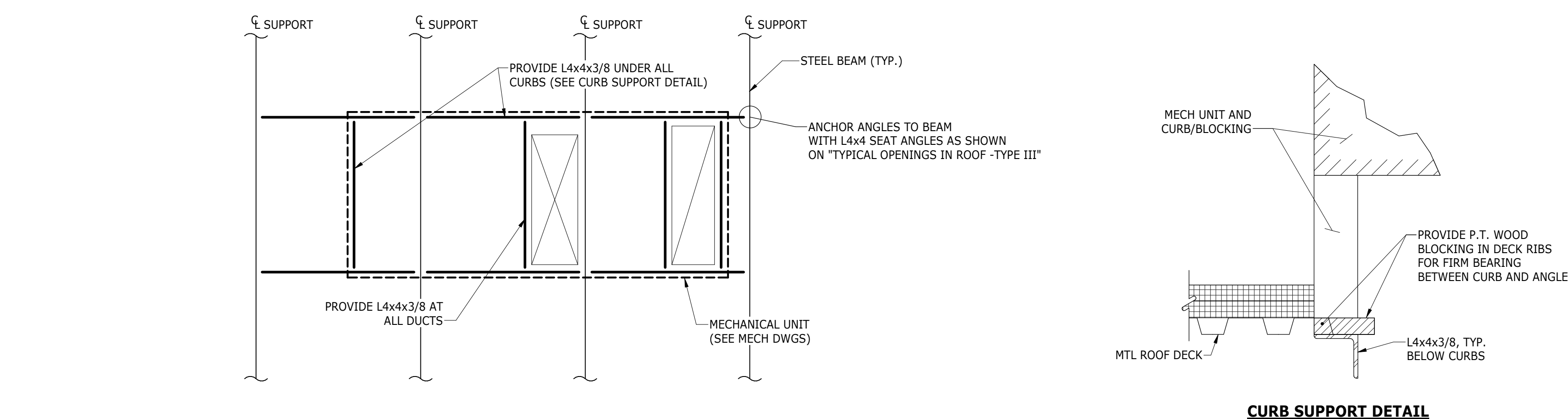
S0.22



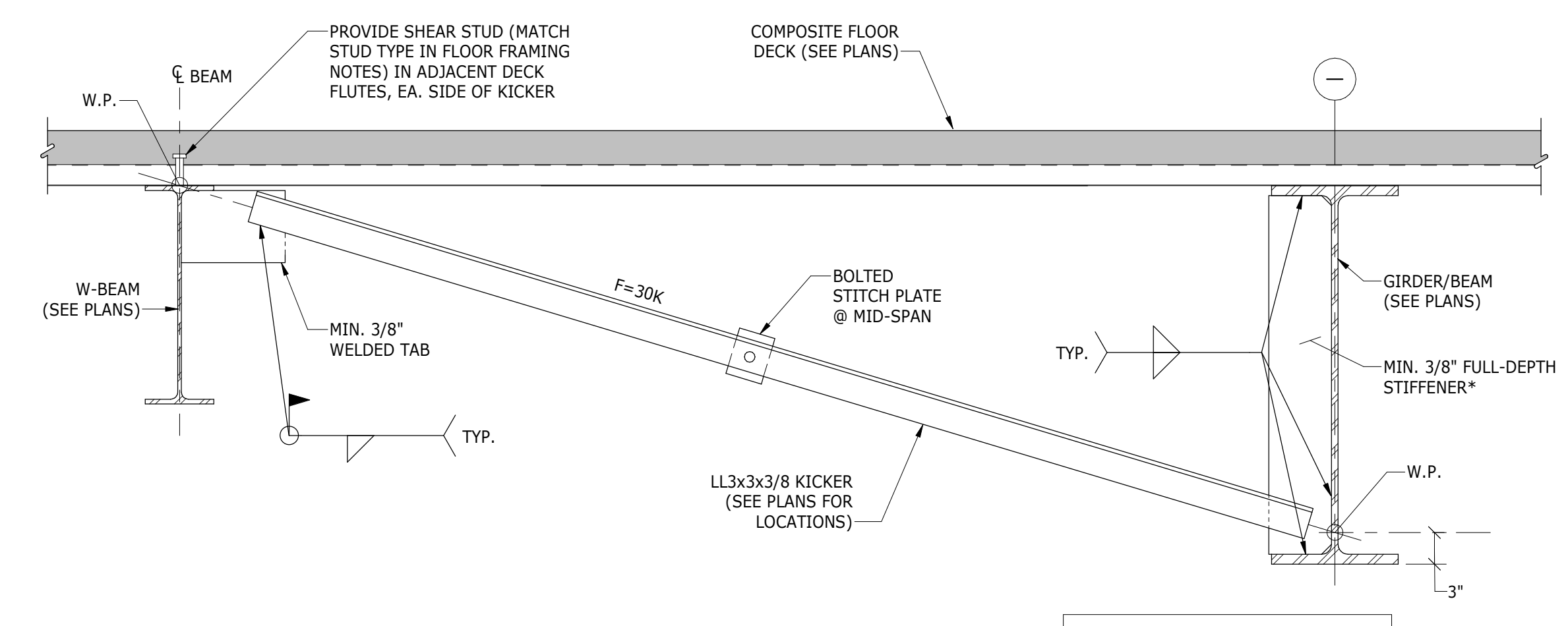
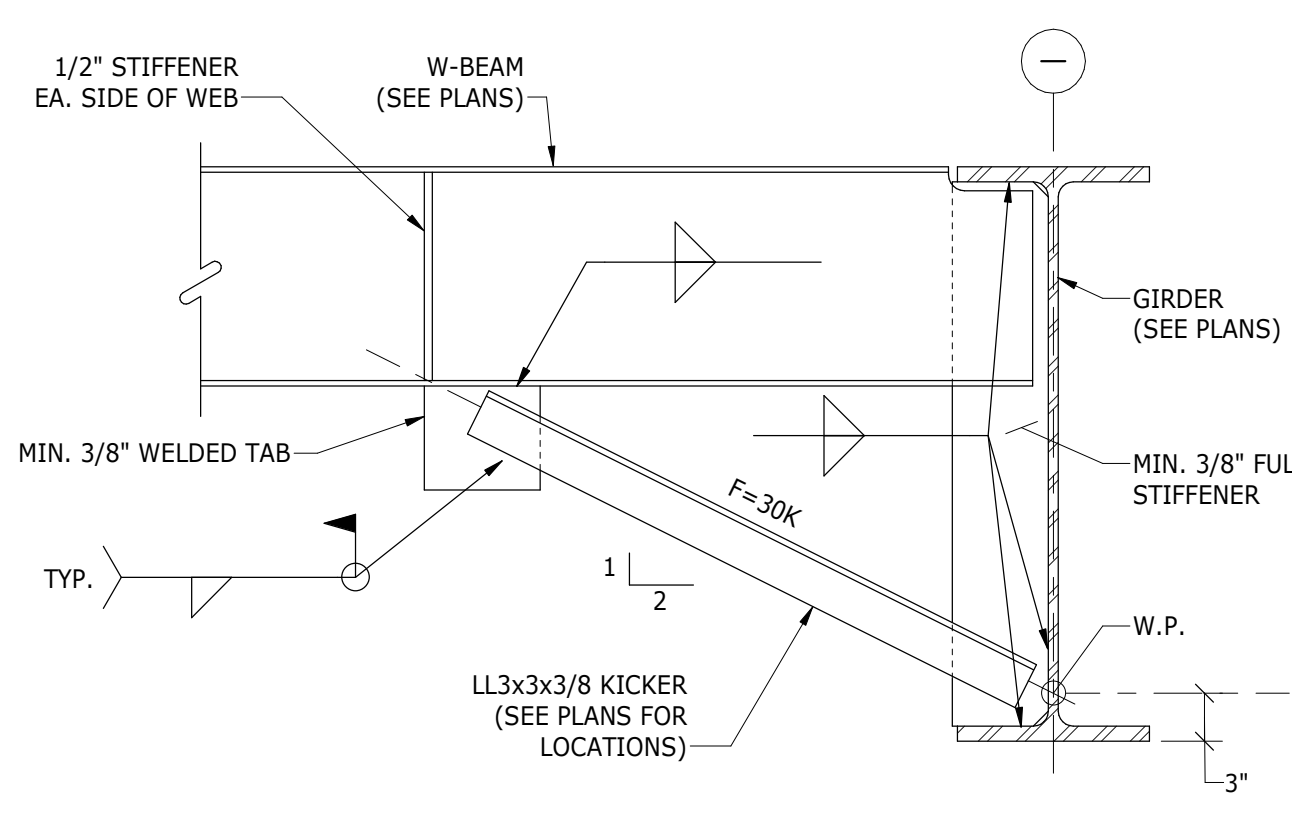
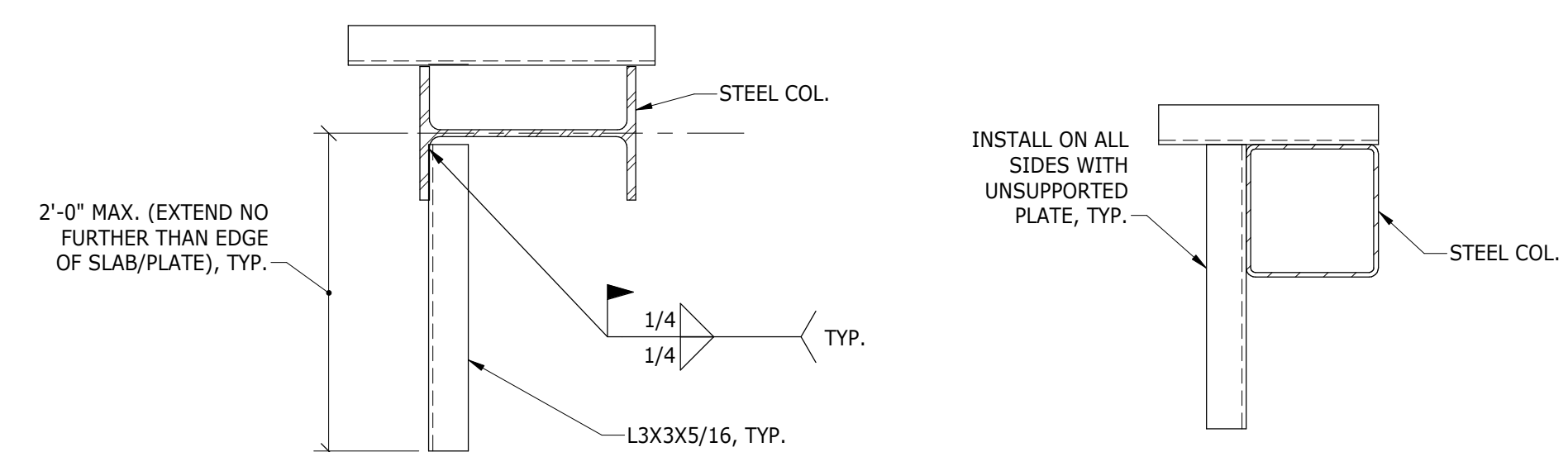
TYPICAL OPENINGS IN ROOF
NOT TO SCALE

GENERAL NOTES:

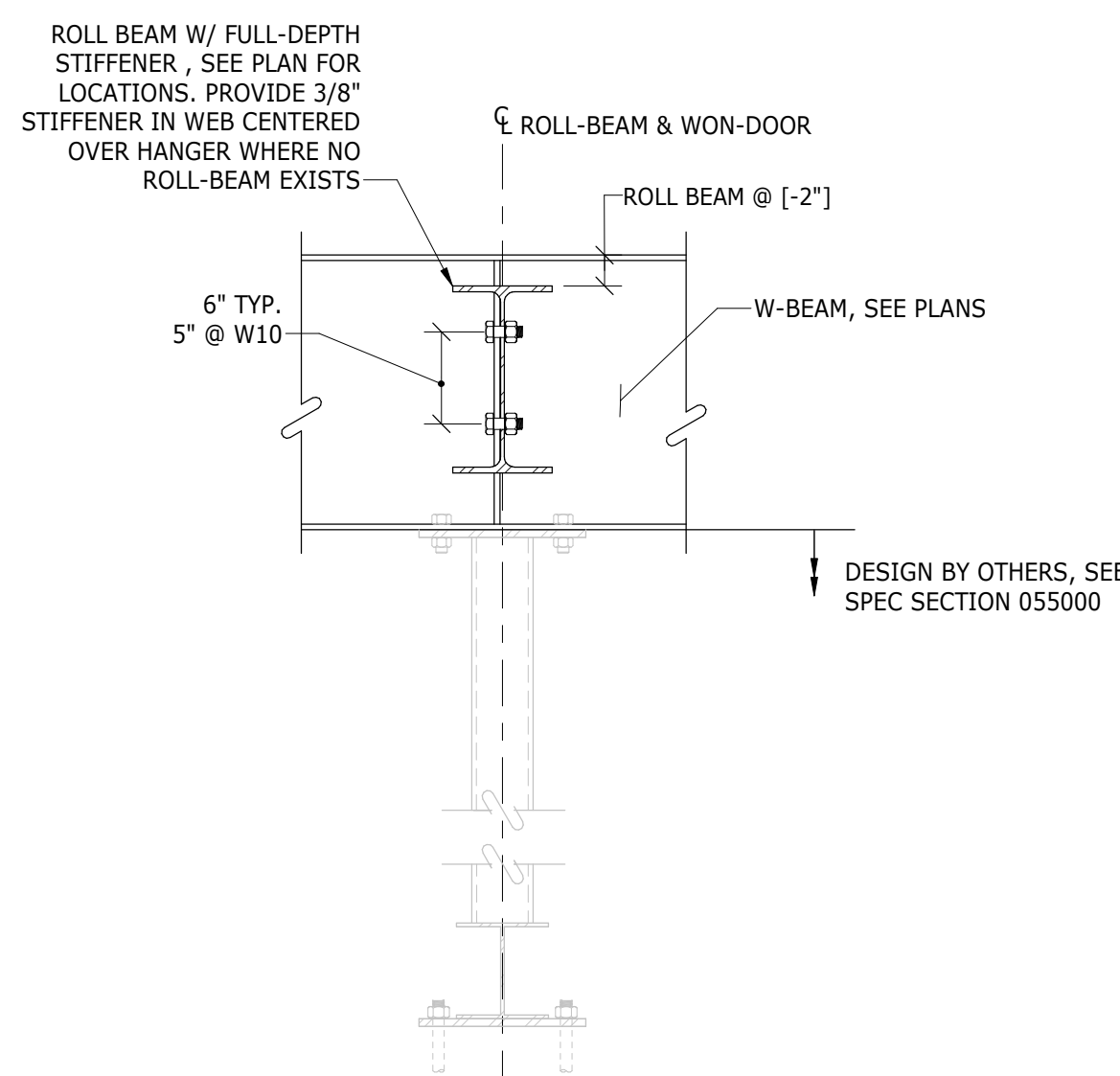
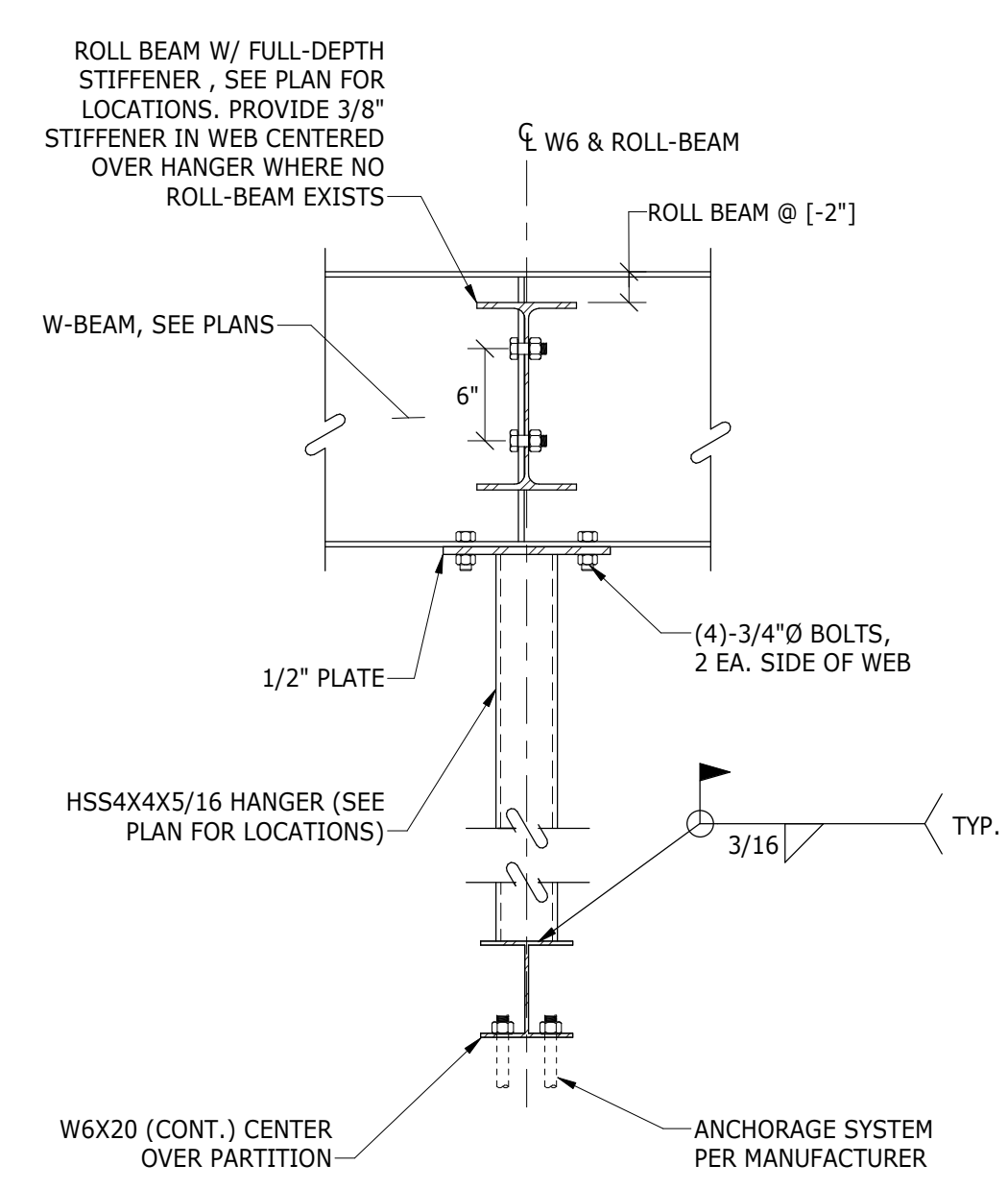
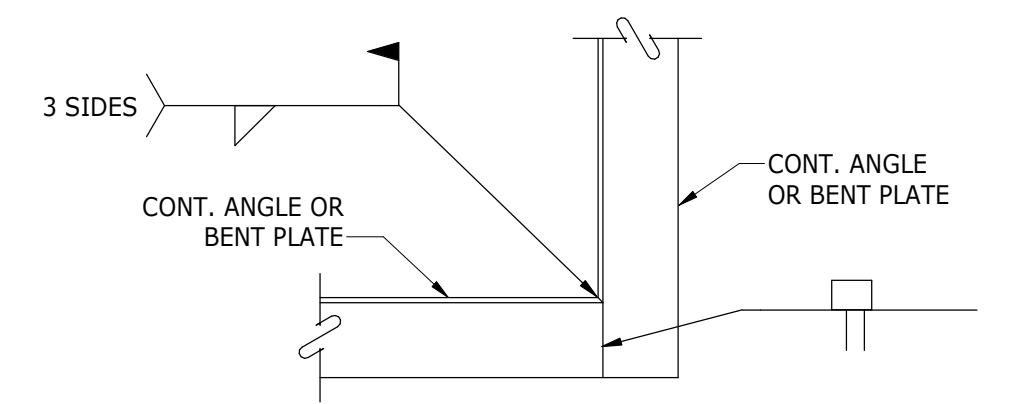
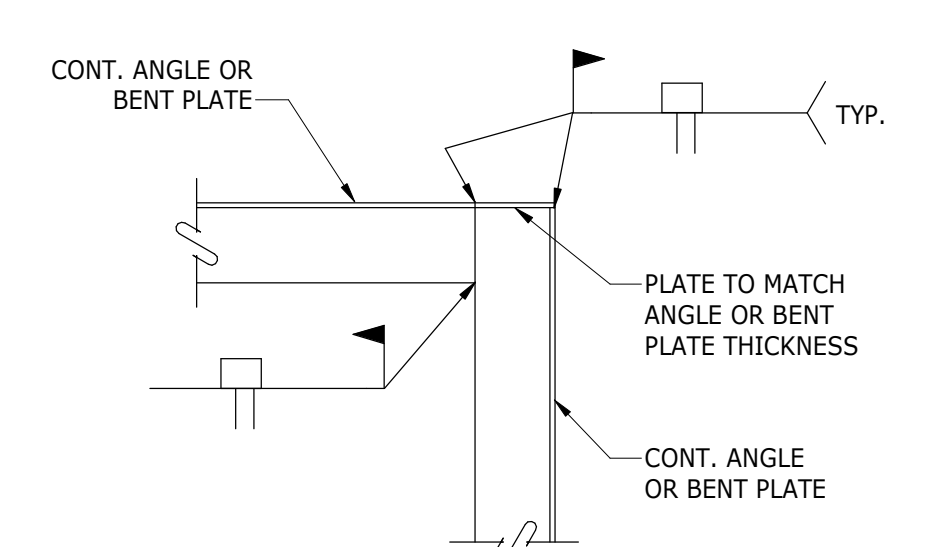
- OPENINGS OUTSIDE THE LIMITS ABOVE SHALL BE FRAMED WITH STRUCTURAL STEEL. OPENING SIZES AND LOCATIONS TO BE SUBMITTED TO STRUCTURAL ENGINEER.
- PROVIDE SOLID P.T. BLOCKING WITHIN ROOF DECK RIBS UNDER ALL CURBS FOR ROOFTOP EQUIPMENT, SKYLIGHTS, HATCHES, ETC.



- NOTES:**
- 5 1/2" NORMAL WEIGHT CONCRETE SLAB ON COMPOSITE METAL DECK (SEE PLAN FOR TYPE), REINFORCED WITH #4 @ 12" B.E.W.
 - TOP OF SLAB VARIES AT SLOPED ROOF, MAINTAIN 5 1/2" SLAB (TOTAL THICKNESS).
 - REFER TO PLANS FOR SLAB LOCATIONS.
 - REFER TO ARCHITECTURAL AND MECHANICAL DRAWINGS FOR MECHANICAL UNIT ANCHOR BOLT REQUIREMENTS AND ADDITIONAL DETAILS.



TYPICAL BOTTOM FLANGE KICKER BRACING DETAILS
NOT TO SCALE

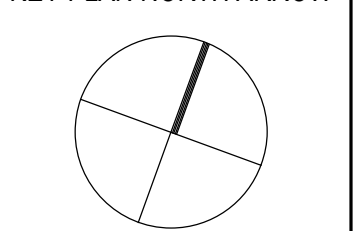


TYPICAL OPERABLE PARTITION SUPPORT DETAIL
NOT TO SCALE

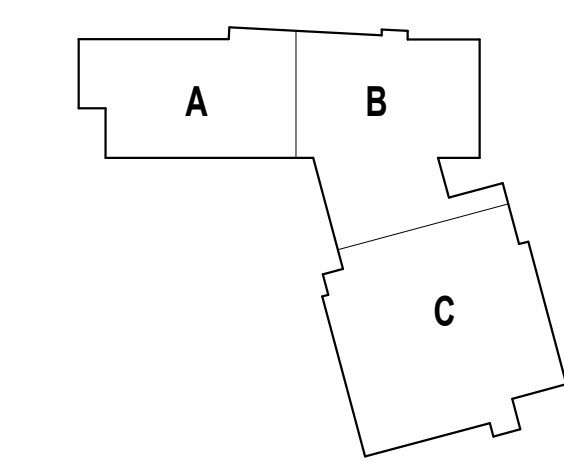
TYPICAL WON-DOOR SUPPORT DETAIL
NOT TO SCALE

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KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

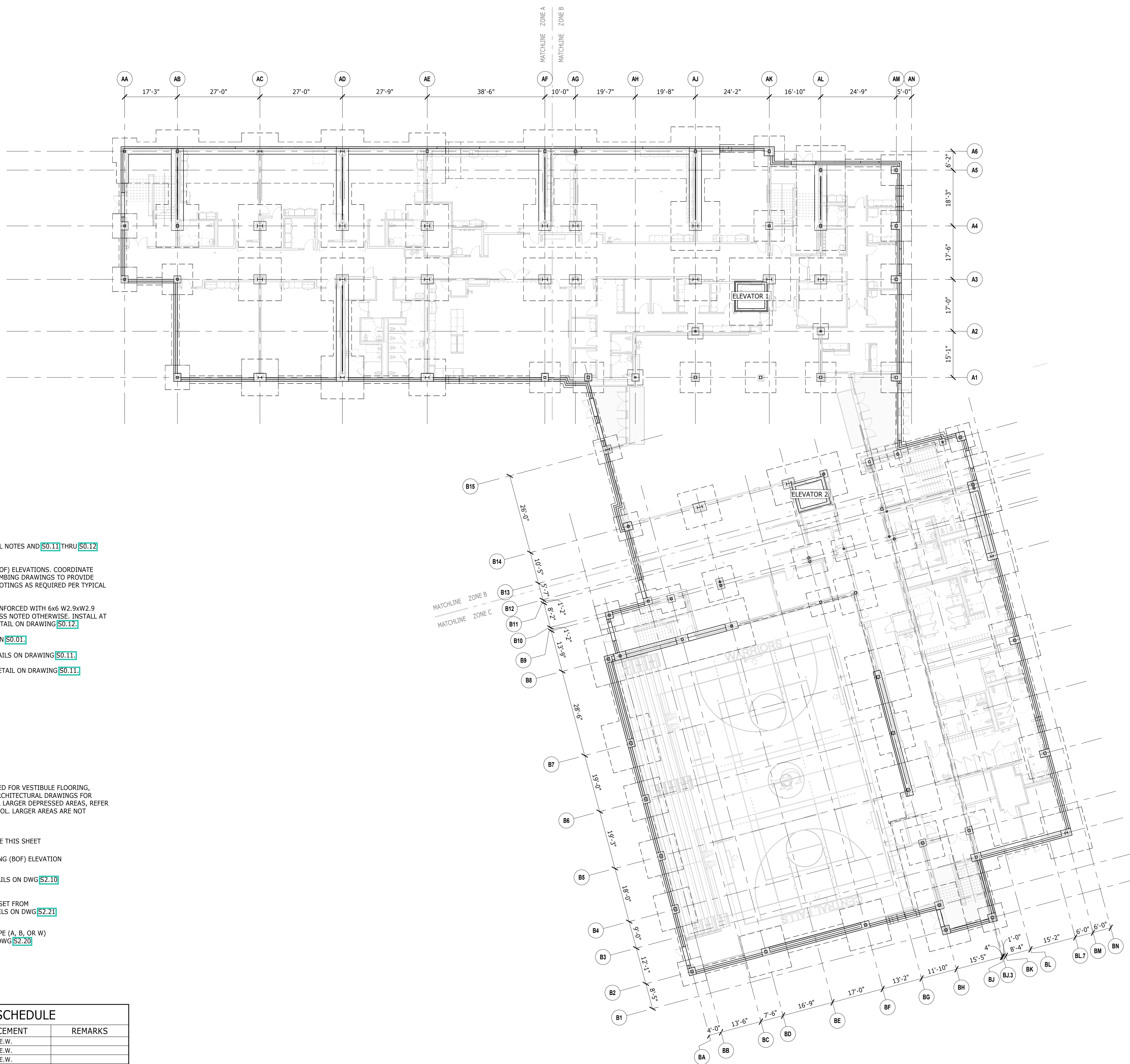
TYPICAL DETAILS - 3

DRAWN BY:	JDB / MSS
REVIEWED BY:	MGM / BP
SCALE:	AS INDICATED
JOB NO.:	2202.02
DATE:	OCTOBER 13, 2023
DRAWING NUMBER:	S0.23



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

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FOUNDATION NOTES:

1. REFER TO DRAWINGS [S0.01](#) FOR STRUCTURAL NOTES AND [S0.11](#) THRU [S0.12](#) FOR ADDITIONAL DETAILS.
2. REFER TO PLAN FOR BOTTOM OF FOOTING (BOF) ELEVATIONS. COORDINATE FINAL BOF ELEVATIONS WITH CIVIL AND PLUMBING DRAWINGS TO PROVIDE MINIMUM CLEARANCE AT UTILITIES. STEP FOOTINGS AS REQUIRED PER TYPICAL DETAIL ON DWG [S0.11](#).
3. 5" CONCRETE SLAB-ON-GRADE SHALL BE REINFORCED WITH 6x6 W2.9xW2.9 WELDED WIRE FABRIC (WWF), TYPICAL UNLESS NOTED OTHERWISE. INSTALL AT MID-DEPTH PER TYPICAL SLAB-ON-GRADE DETAIL ON DRAWING [S0.12](#).
4. PROVIDE "MVRA" IN ALL SLABS PER NOTES ON [S0.01](#).
5. STEP FOUNDATION AT ENTRYWAYS, SEE DETAILS ON DRAWING [S0.11](#).
6. SEE ENTRYWAY AT EXTERIOR SHEAR WALL DETAIL ON DRAWING [S0.11](#).

LEGEND:

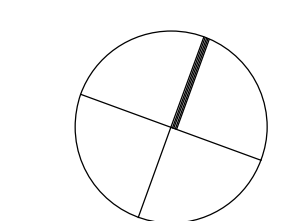
- INDICATES SLAB TO BE STEPPED FOR VESTIBULE FLOORING, EQUIPMENT, ETC. REFER TO ARCHITECTURAL DRAWINGS FOR LOCATIONS AND DETAILS. FOR LARGER DEPRESSED AREAS, REFER TO PLAN AND SLAB STEP SYMBOL. LARGER AREAS ARE NOT HATCHED FOR PLAN CLARITY.
- FOOTING MARK, SEE SCHEDULE THIS SHEET
- INDICATES BOTTOM OF FOOTING (BOF) ELEVATION
- COLUMN PIER MARK, SEE DETAILS ON DWG [S2.10](#)
- COLUMN PIER MARK, PIER OFFSET FROM FOUNDATION WALL, SEE DETAILS ON DWG [S2.21](#)
- COLUMN SIZE-(X) (X) INDICATES BASE PLATE TYPE (A, B, OR W) SEE BASE PLATE DETAILS ON DWG [S2.20](#)
- INDICATES SLAB STEP

MARK	SIZE	REINFORCEMENT	REMARKS
F5	5'-0" x 5'-0" x 1'-0"	(6) #6 E.W.	
F7	7'-0" x 7'-0" x 1'-0"	(9) #6 E.W.	
F8	8'-0" x 8'-0" x 1'-6"	(9) #6 E.W.	
F8A	8'-0" x 8'-0" x 2'-0"	(9) #6 E.W.	
F10	10'-0" x 10'-0" x 2'-0"	(11) #8 E.W.	
F12	12'-0" x 12'-0" x 2'-0"	(13) #8 E.W.	
F14	14'-0" x 14'-0" x 2'-0"	#8 @ 8" O.C., E.W., T & B	
F16	16'-0" x 16'-0" x 2'-0"	#8 @ 8" O.C., E.W., T & B	
F18	18'-0" x 18'-0" x 2'-6"	#8 @ 8" O.C., E.W., T & B	
F20x14	20'-0" x 14'-0" x 2'-0"	#8 @ 8" O.C., E.W., T & B	
F24x14	24'-0" x 14'-0" x 2'-0"	#8 @ 8" O.C., E.W., T & B	
F54x16	54'-0" x 16'-0" x 2'-6"	#8 @ 8" O.C., E.W., T & B	

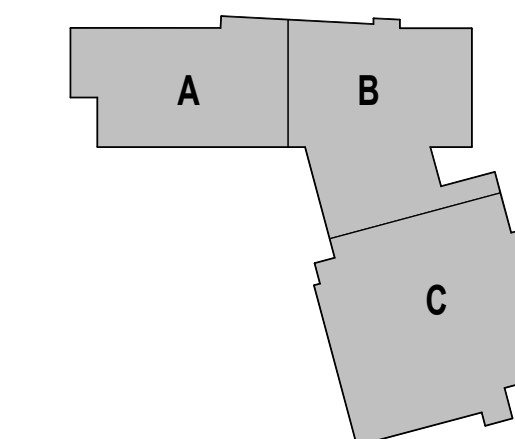
1 FOUNDATION: OVERALL PLAN
1/8" = 1'-0"

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KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**OVERALL
FOUNDATION
PLAN**

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

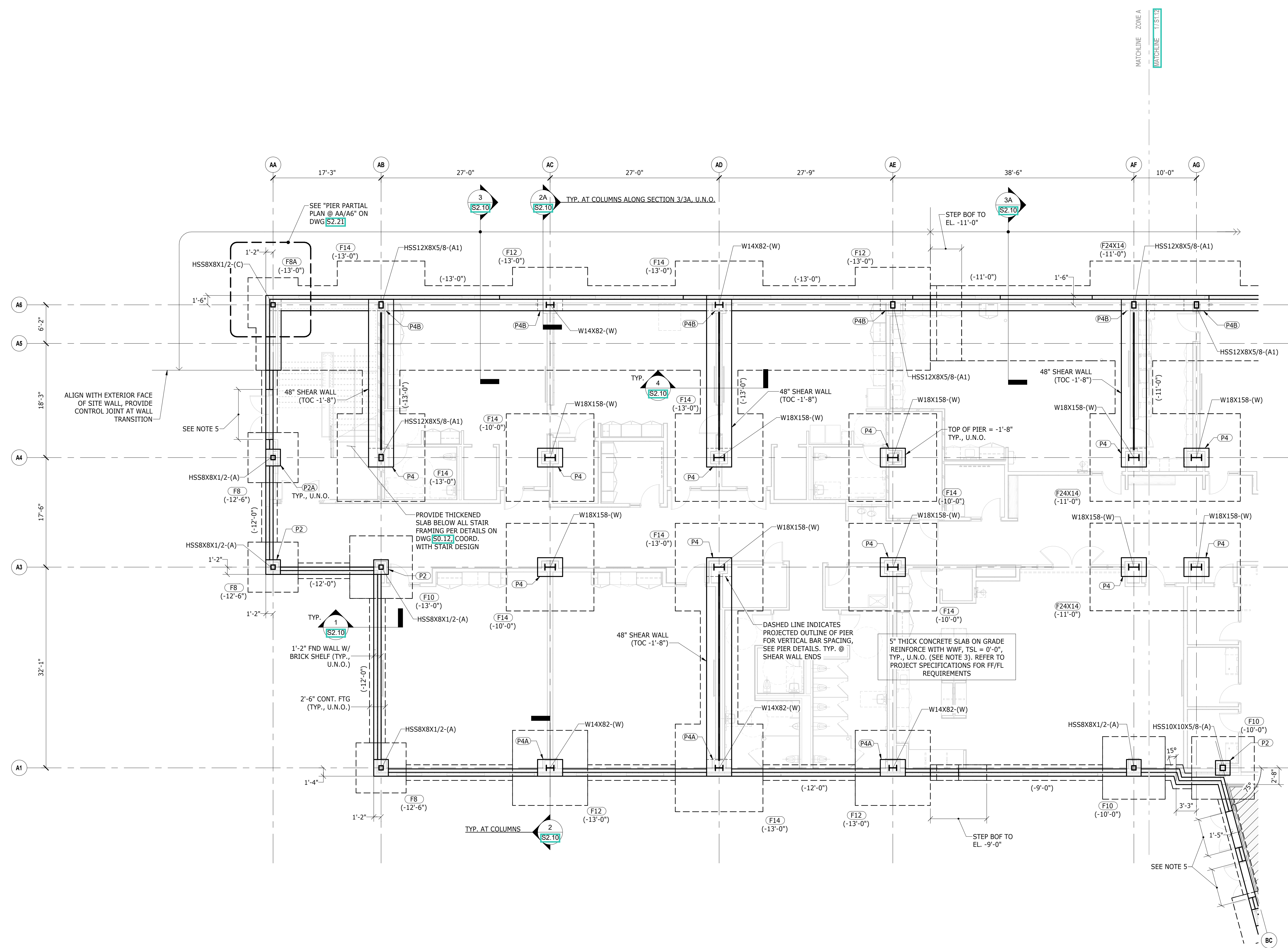
DATE: OCTOBER 13, 2023

S1.10



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

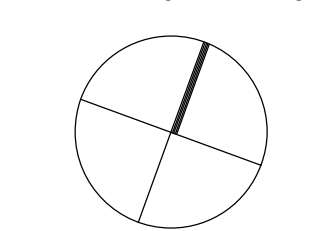
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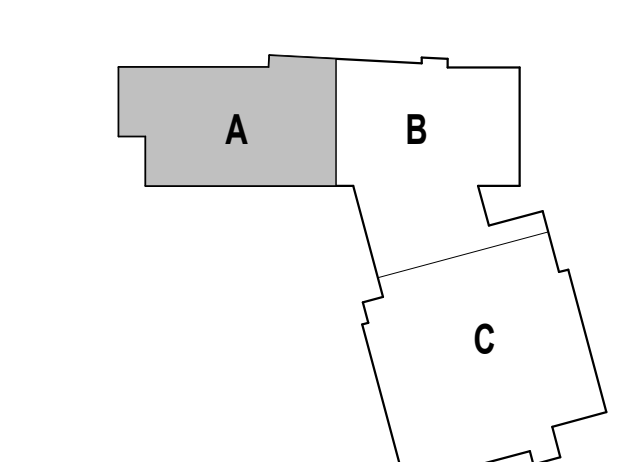
1 FOUNDATION PLAN - ZONE A
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**FOUNDATION
PLAN - ZONE A**

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

S1.11

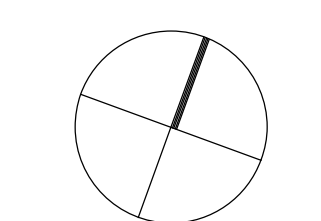


CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

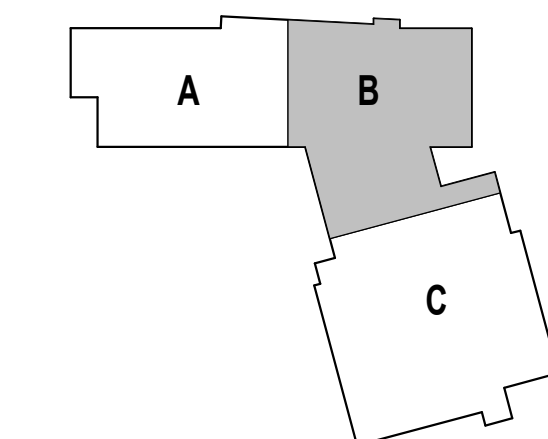
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KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**FOUNDATION
PLAN - ZONE B**

DRAWN BY: JDB / MSS

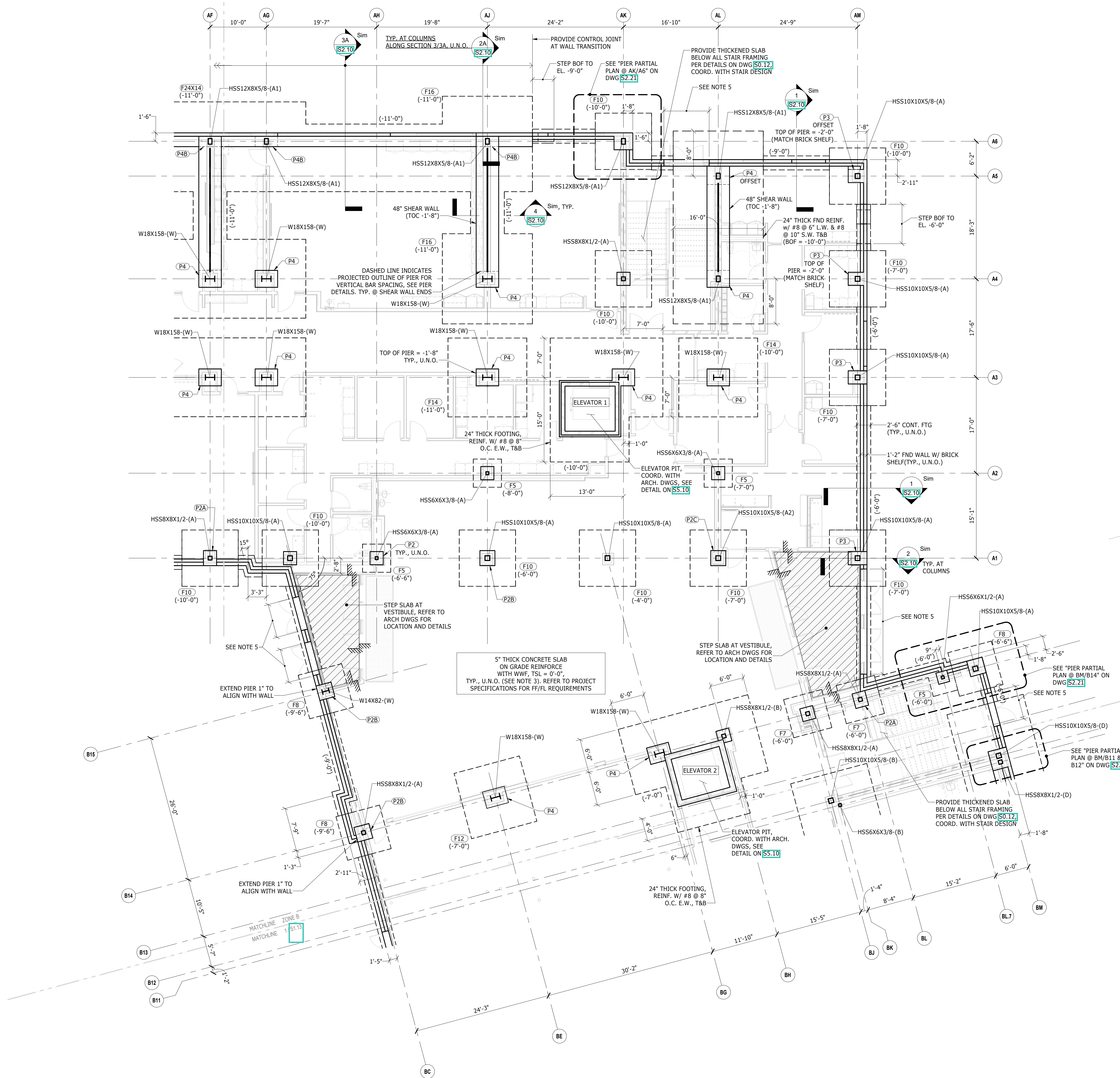
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

S1.12



1 FOUNDATION PLAN - ZONE B
1/8" = 1'-0"



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

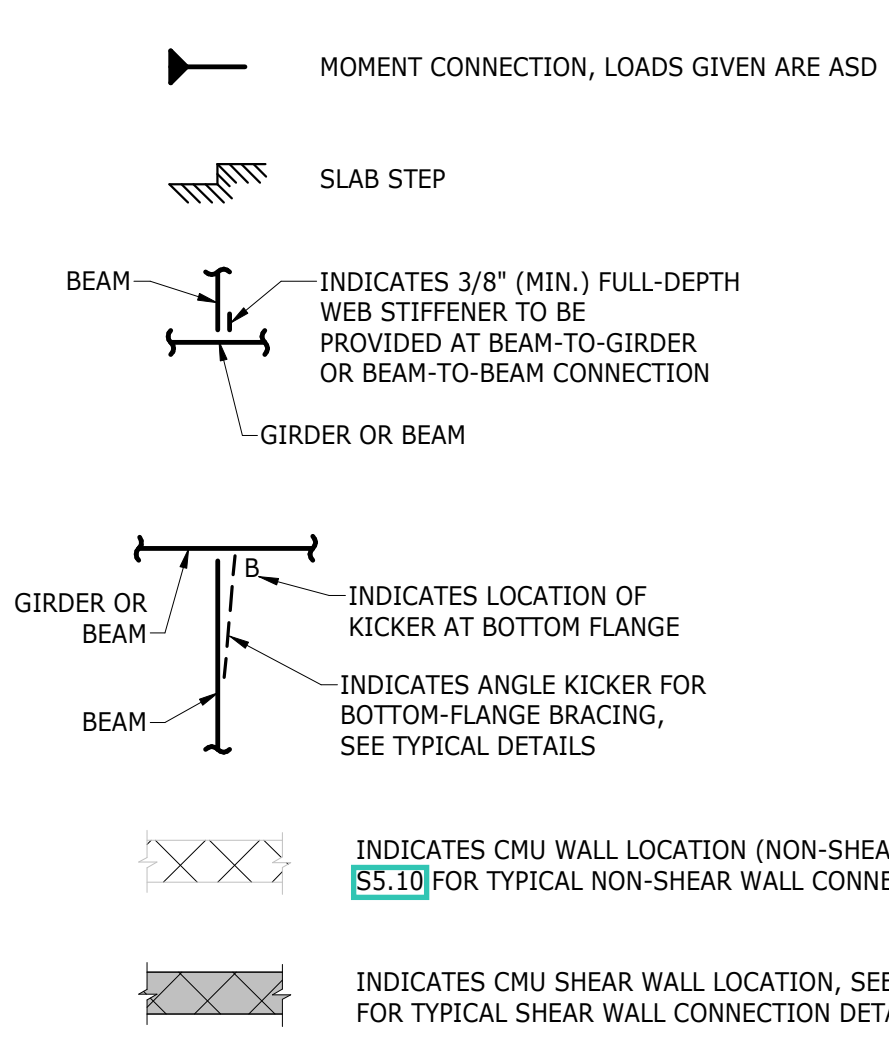
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FLOOR FRAMING NOTES:

- REFER TO DRAWINGS [S0.01](#) FOR STRUCTURAL NOTES AND [S0.21](#) THRU [S0.24](#) FOR TYPICAL DETAILS.
- 5 1/4" INDICATES SPAN DIRECTION OF 2", 18 GA. GALVANIZED COMPOSITE METAL DECK WITH 3 1/4" LIGHTWEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 W2.1xW2.1 WWF. SEE PLANS AND TYPICAL DETAILS FOR ADDITIONAL REINFORCING. CONCRETE UNIT WEIGHT AND AIR-ENTRAINMENT SHALL BE PER PLANS AND SPECIFICATIONS.
- 3" INDICATES SPAN DIRECTION OF 3", 18 GA., TYPE N GALVANIZED ROOF DECK.
- "X" SC INDICATES THE NUMBER OF 3/4" DIAMETER, 4" LONG SHEAR STUDS SPACED EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE. LENGTH SHALL BE FINAL LENGTH AFTER WELDING (i.e. 2" ABOVE TOP OF DECK).
- TOP OF FLOOR SLAB (TSL) = 114'-0", TYP., U.N.O.
TOP OF STEEL (TST) = 113'-6 3/4", TYP., U.N.O.
a. [-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST.
- [X] DENOTES CONNECTION DESIGN FORCES (SERVICE/ASD LOAD) IN KIPS. FORCES ARE VERTICAL UNLESS NOTED OTHERWISE AS FOLLOWS:
(H) = HORIZONTAL
(A) = AXIAL (NOTE AXIAL FORCES ON PLANS ARE IN ADDITION TO ANY AXIAL COMPONENT OF BRACE FRAME FORCES)
(M) = BENDING MOMENT IN STRONG DIRECTION
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS AND PROVIDE REINFORCEMENT PER TYPICAL DETAILS ON [S0.21](#) THRU [S0.23](#).
- SEE TYPICAL SECTION AT ISOLATED SLAB-ON-DECK DETAIL ON DWG [S0.21](#) COORDINATE ISOLATED SLAB-ON-DECK REQUIREMENTS WITH LOCAL SLAB EDGE, ROOF DECK SUPPORT, AND SHEAR WALL DETAILS AS REQUIRED.
- ALIGN BEAM CENTERLINE 9" OFF EDGE OF SLAB AT STAIR OPENING. REFER TO ARCH DWGS FOR EDGE OF SLAB LOCATIONS/DIMENSIONS.

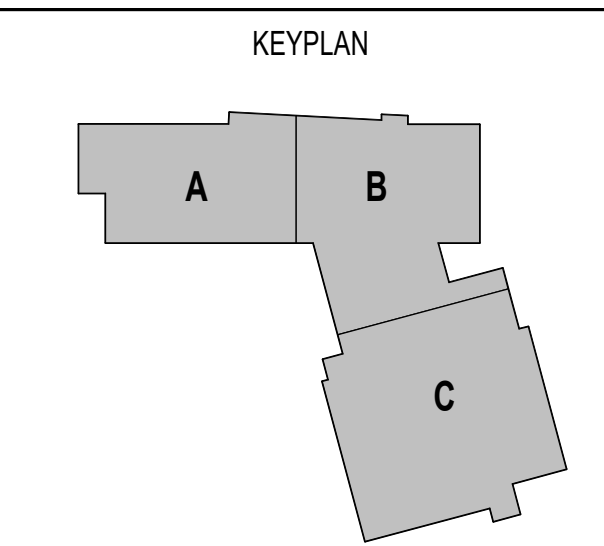
LEGEND:



1 SECOND FLOOR FRAMING: OVERALL PLAN
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



DRAWING NAME:
OVERALL SECOND FLOOR FRAMING PLAN

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S1.20**



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

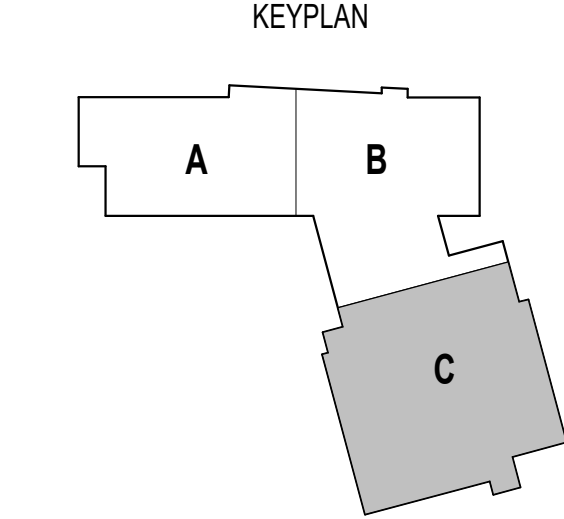
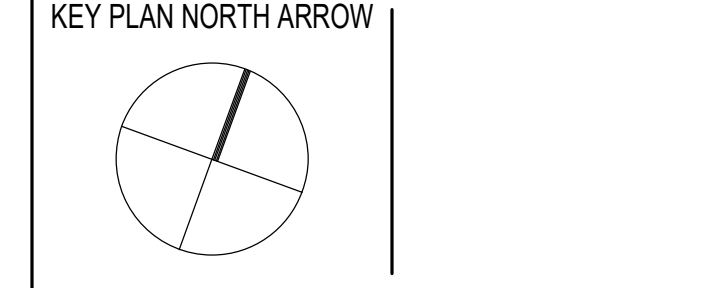
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REFER TO DRAWING S1.20 FOR SCHEDULES, NOTES, ETC.

1 SECOND FLOOR FRAMING PLAN - ZONE C
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS



DRAWING NAME:	
SECOND FLOOR FRAMING PLAN - ZONE C	
DRAWN BY:	JDB / MSS
REVIEWED BY:	MGM / BP
SCALE:	AS INDICATED DRAWING NUMBER:
JOB NO.:	2202.02
DATE:	OCTOBER 13, 2023
S1.23	



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

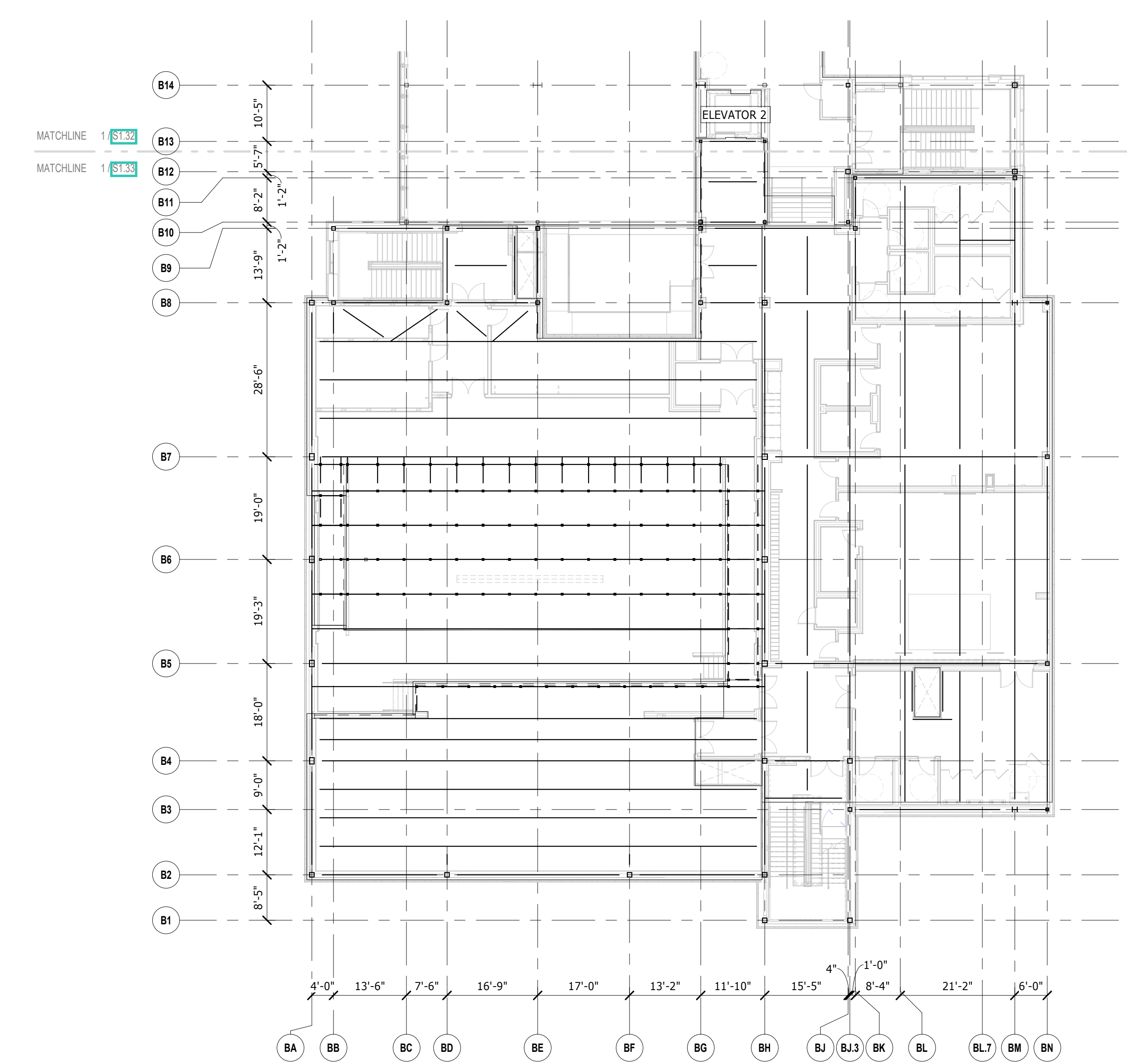
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FLOOR FRAMING NOTES:

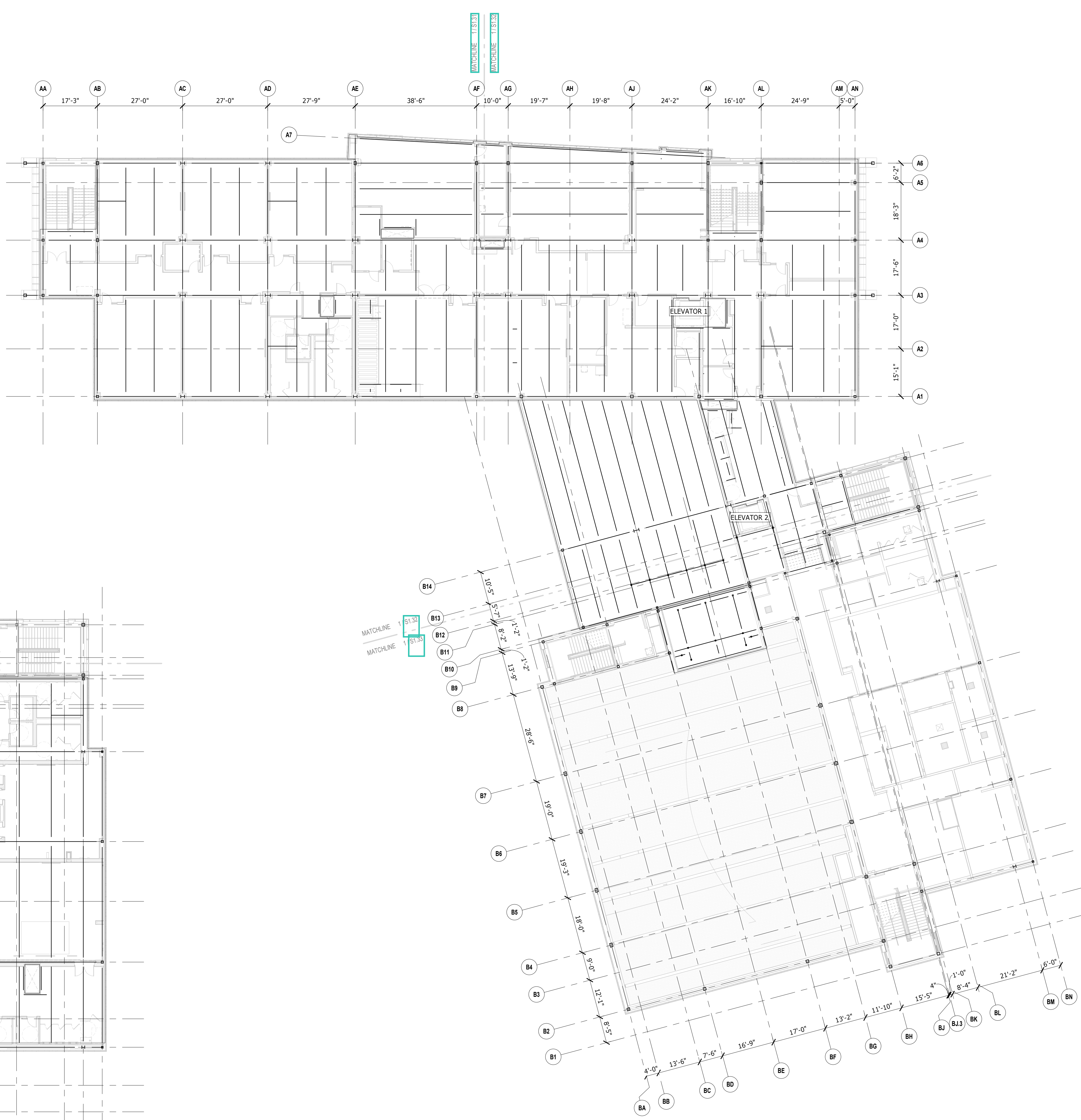
- REFER TO DRAWINGS [S0.01](#) FOR STRUCTURAL NOTES AND [S0.21](#) THRU [S0.24](#) FOR TYPICAL DETAILS.
- 5 1/4" INDICATES SPAN DIRECTION OF 2", 18 GA. GALVANIZED COMPOSITE METAL DECK WITH 3 1/4" LIGHTWEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 W2.1xW2.1 WWF. SEE PLANS AND TYPICAL DETAILS FOR ADDITIONAL REINFORCING. CONCRETE UNIT WEIGHT AND AIR-ENTRAINMENT SHALL BE PER PLANS AND SPECIFICATIONS.
- 8 1/4" INDICATES SPAN DIRECTION OF 2", 18 GA. GALVANIZED COMPOSITE METAL DECK WITH 6 1/4" LIGHTWEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 8 1/4". REINFORCE WITH 6x6 W2.1xW2.1 WWF. SEE PLANS AND TYPICAL DETAILS FOR ADDITIONAL REINFORCING. CONCRETE UNIT WEIGHT AND AIR-ENTRAINMENT SHALL BE PER PLANS AND SPECIFICATIONS.
- 3" INDICATES SPAN DIRECTION OF 3", 18 GA., TYPE N GALVANIZED ROOF DECK.
- "X" SC INDICATES THE NUMBER OF 3/4" DIAMETER, 4" LONG SHEAR STUDS SPACED EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE. LENGTH SHALL BE FINAL LENGTH AFTER WELDING (I.E. 2" ABOVE TOP OF DECK).
- TOP OF FLOOR SLAB (TSL) = 128'-0" (ACADEMIC SECTOR) & 132'-0" (PERFORMING SECTOR), TYP., U.N.O.
TOP OF STEEL (TST) = 127'-6 3/4" (ACADEMIC SECTOR) & 131'-6 3/4" (PERFORMING SECTOR), TYP., U.N.O.
a. [-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST.
- [X] DENOTES CONNECTION DESIGN FORCES (SERVICE/ASD LOAD) IN KIPS. FORCES ARE VERTICAL UNLESS NOTED OTHERWISE AS FOLLOWS:
(H) = HORIZONTAL
(A) = AXIAL (NOTE AXIAL FORCES ON PLANS ARE IN ADDITION TO ANY AXIAL COMPONENT OF BRACE FRAME FORCES)
(M) = BENDING MOMENT IN STRONG DIRECTION
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS AND PROVIDE REINFORCEMENT PER TYPICAL DETAILS ON [S0.21](#) THRU [S0.23](#).
- SEE TYPICAL SECTION AT ISOLATED SLAB-ON-DECK DETAIL ON DWG [S0.21](#) COORDINATE ISOLATED SLAB-ON-DECK REQUIREMENTS WITH LOCAL SLAB EDGE, ROOF DECK SUPPORT, AND SHEAR WALL DETAILS AS REQUIRED.
- ALIGN BEAM CENTERLINE 9" OFF EDGE OF SLAB AT STAIR OPENING. REFER TO ARCH DWGS FOR EDGE OF SLAB LOCATIONS/DIMENSIONS.

LEGEND:

- MOMENT CONNECTION, LOADS GIVEN ARE ASD
- SLAB STEP
- BEAM: INDICATES 3/8" (MIN.) FULL-DEPTH WEB STIFFENER TO BE PROVIDED AT BEAM-TO-GIRDER OR BEAM-TO-BEAM CONNECTION
- GIRDER OR BEAM
- INDICATES LOCATION OF KICKER AT BOTTOM FLANGE
- INDICATES ANGLE KICKER FOR BOTTOM-FLANGE BRACING, SEE TYPICAL DETAILS
- INDICATES CMU WALL LOCATION (NON-SHEAR WALL), SEE [S5.10](#) FOR TYPICAL NON-SHEAR WALL CONNECTION DETAIL
- INDICATES CMU SHEAR WALL LOCATION, SEE [S5.10](#) FOR TYPICAL SHEAR WALL CONNECTION DETAIL



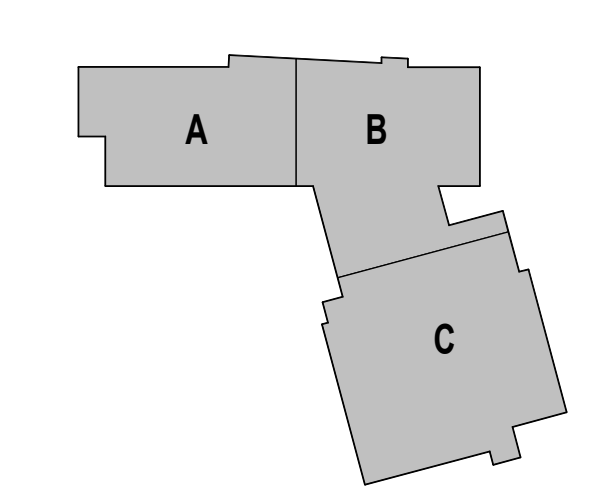
2 THIRD FLOOR PERFORMING FRAMING: OVERALL
PLAN
1/16" = 1'-0"



1 THIRD FLOOR ACADEMIC FRAMING: OVERALL PLAN
1/16" = 1'-0"

100% CONSTRUCTION DOCUMENTS
KEY PLAN NORTH ARROW

KEYPLAN



DRAWING NAME:
OVERALL THIRD FLOOR FRAMING PLAN

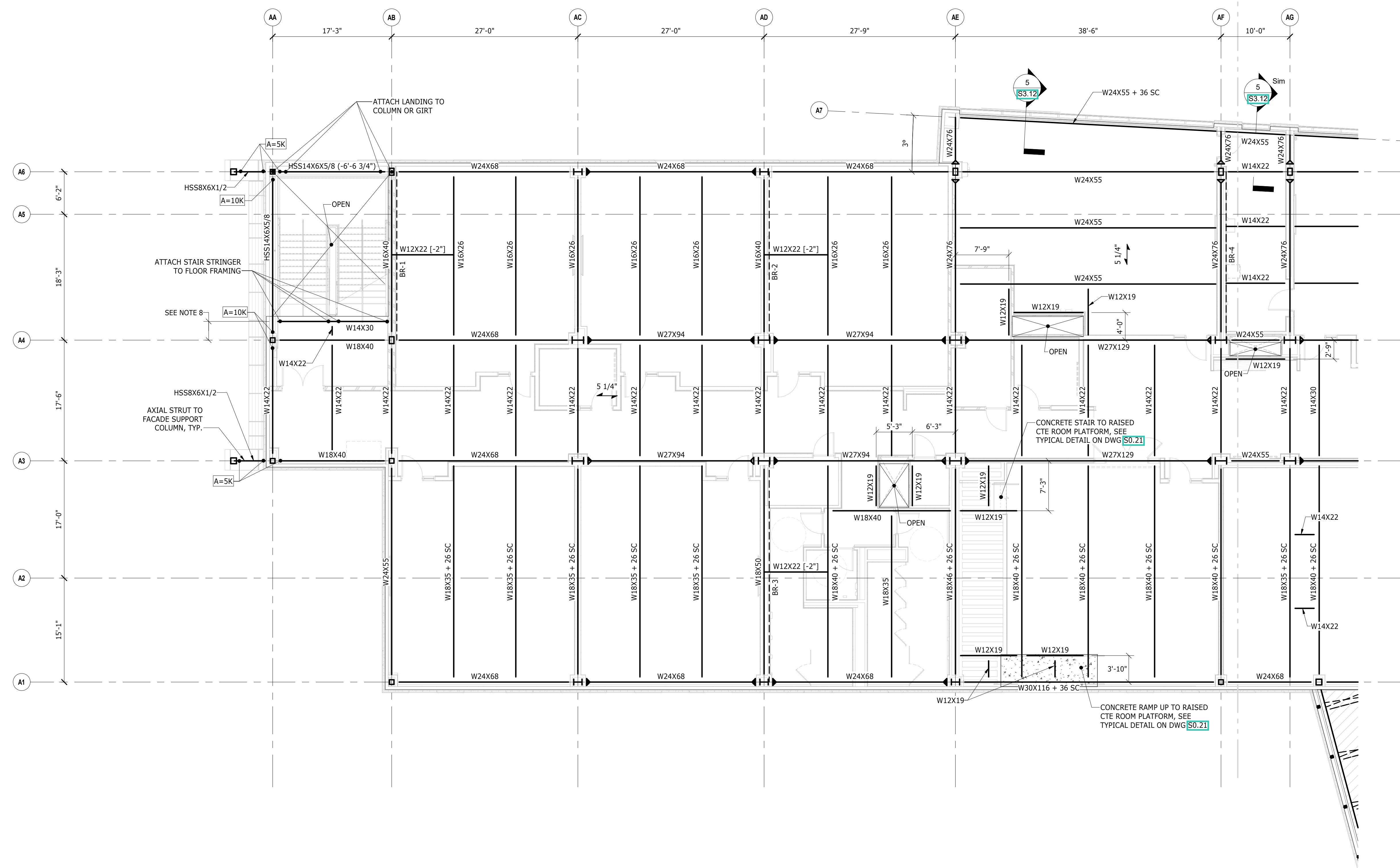
DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S1.30**



CENTRAL FALLS HIGH SCHOOL
10 HIGGINSON AVE, CENTRAL FALLS, RI

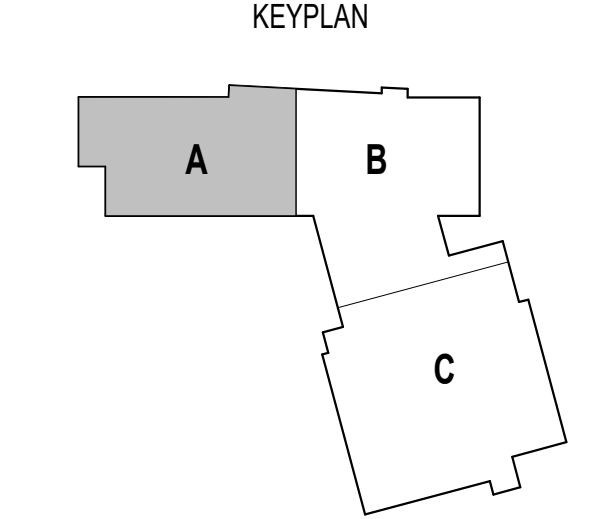
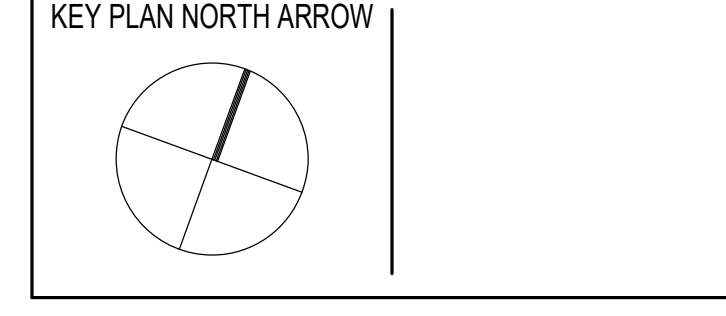
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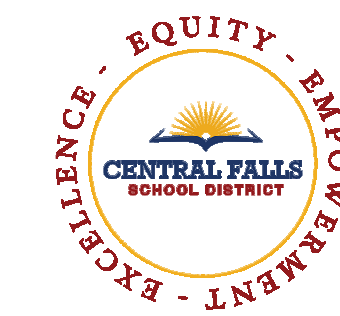
1 THIRD FLOOR ACADEMIC FRAMING PLAN - ZONE A
1/8" = 1'-0"

REFER TO DRAWING S1.30 FOR SCHEDULES, NOTES, ETC.

100% CONSTRUCTION DOCUMENTS

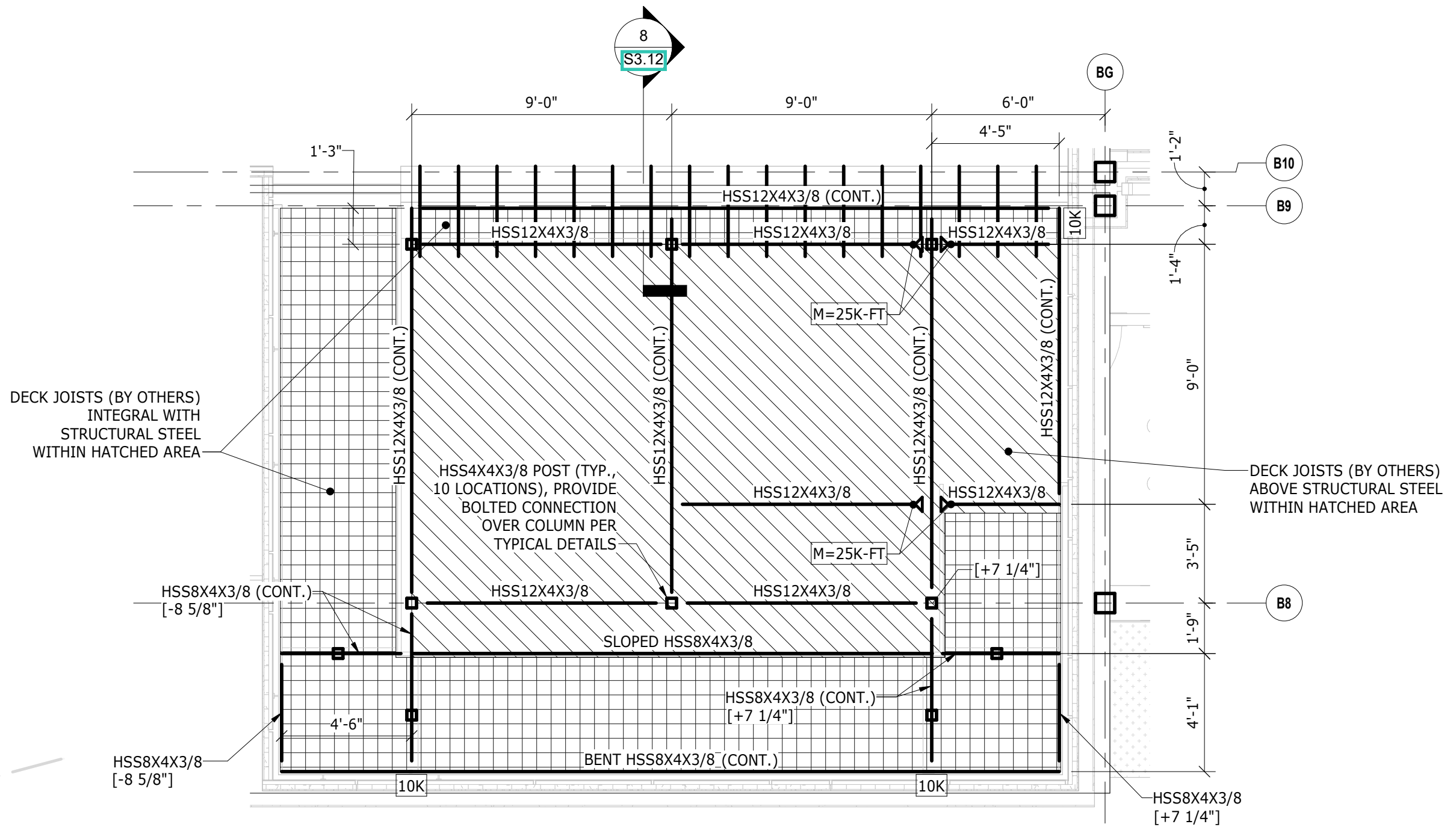


DRAWING NAME:	
THIRD FLOOR FRAMING PLAN - ZONE A	
DRAWN BY:	JDB / MSS
REVIEWED BY:	MGM / BP
SCALE:	AS INDICATED DRAWING NUMBER:
JOB NO.:	2202.02
DATE:	OCTOBER 13, 2023
S1.31	



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- NOTES:**
1. TYPICAL TST = 130'-11", G.C. TO COORDINATE ALL DIMENSIONS AND ELEVATIONS WITH SHIMMING/INSTALLATION REQUIREMENTS.
a. [+/-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST
 2. REFER TO ARCH/LANDSCAPE DWGS FOR ADDITIONAL DETAILS.
 3. ALL FRAMING, CONNECTORS, COMPONENTS, ETC. SHALL BE HOT-DIPPED GALVANIZED.

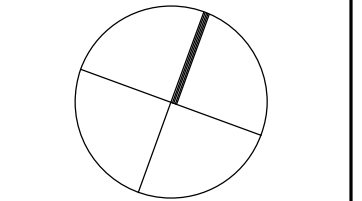
2 PARTIAL PLAN
1/4" = 1'-0"

REFER TO DRAWING S1.30 FOR SCHEDULES, NOTES, ETC.

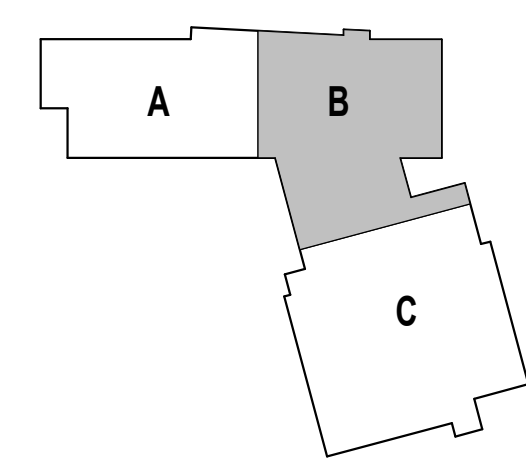
1 THIRD FLOOR ACADEMIC FRAMING PLAN - ZONE B
3/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

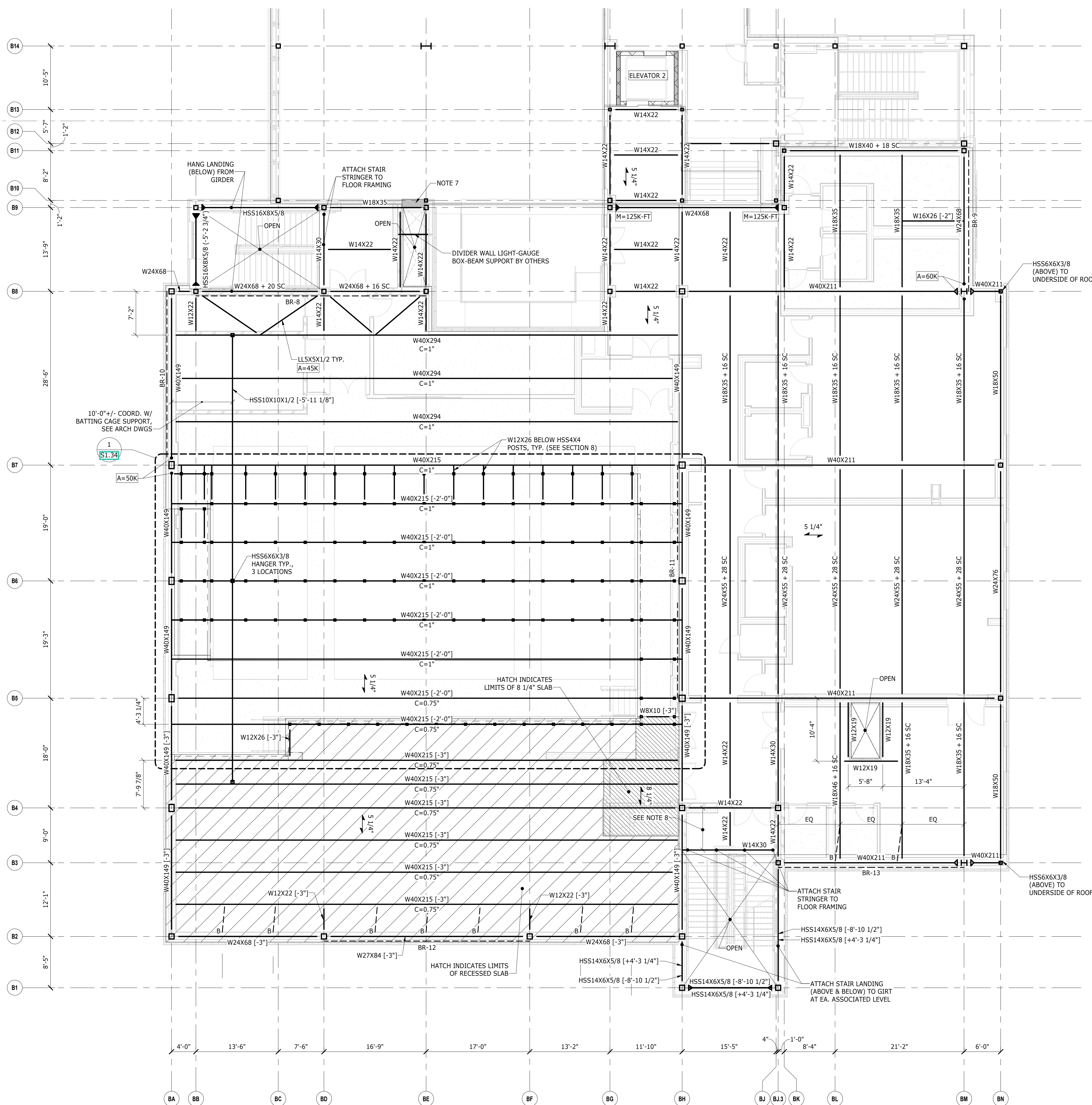
**THIRD FLOOR
FRAMING PLAN -
ZONE B**

DRAWN BY:	JDB / MSS
REVIEWED BY:	MGM / BP
SCALE:	AS INDICATED DRAWING NUMBER:
JOB NO.:	2202.02
DATE:	OCTOBER 13, 2023
S1.32	



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10 HIGGINSON AVE, CENTRAL FALLS, RI

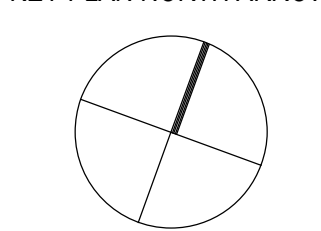
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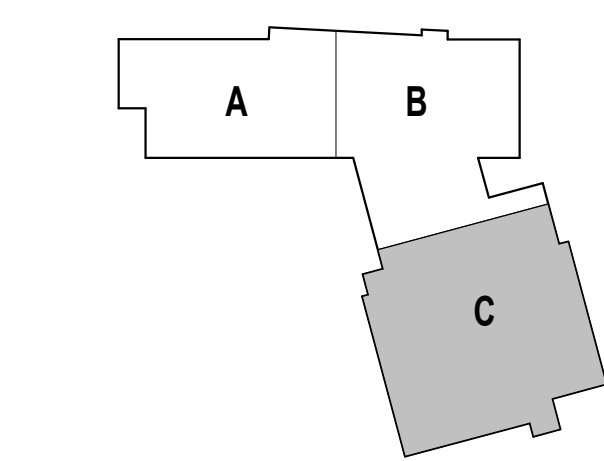
REFER TO DRAWING S1.30
FOR SCHEDULES, NOTES, ETC.

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**THIRD FLOOR
FRAMING PLAN -
ZONE C**

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

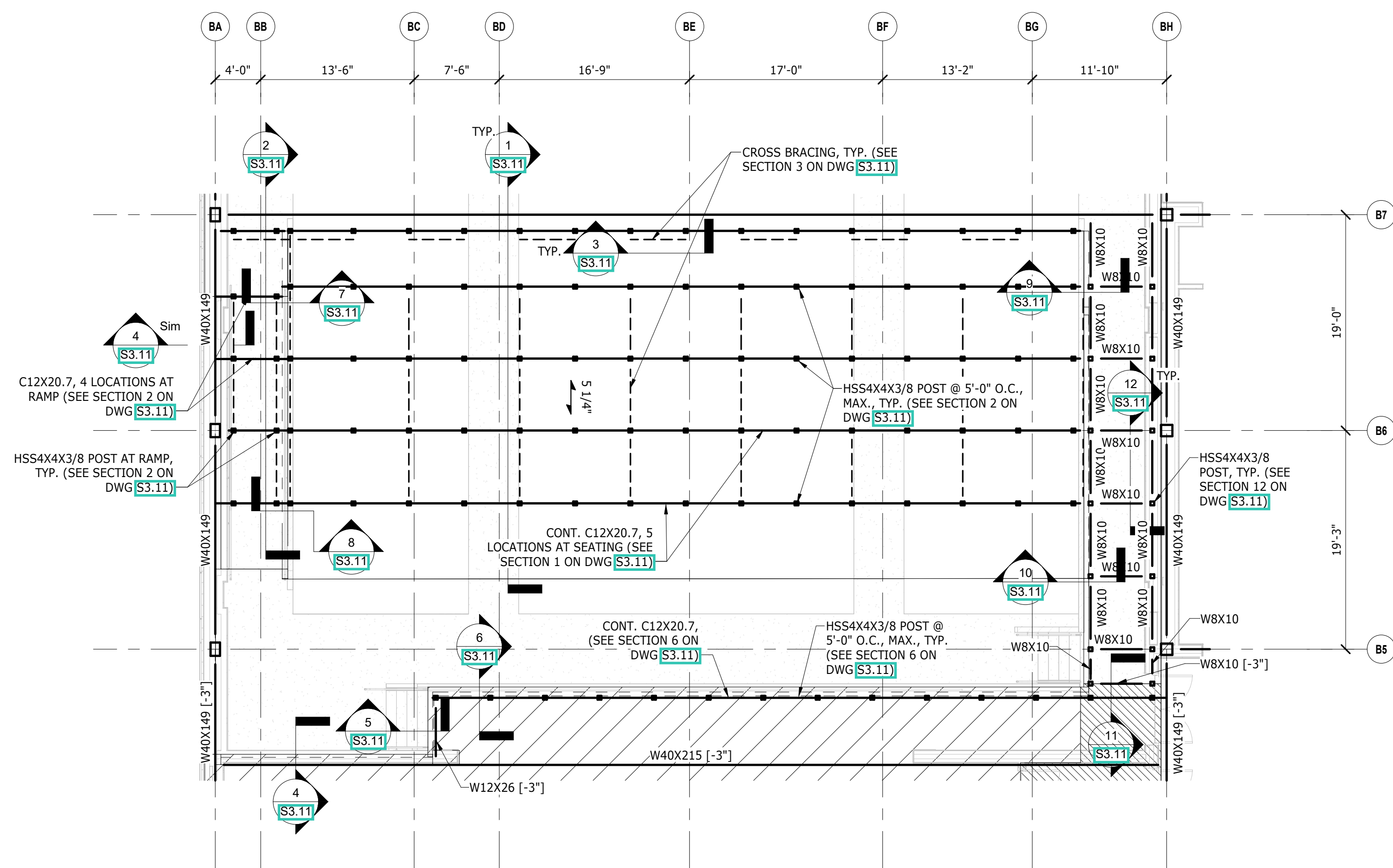
S1.33



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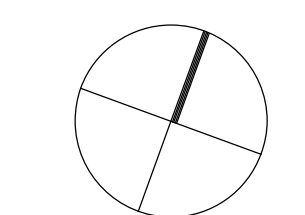
KEYNOTE LEGEND:



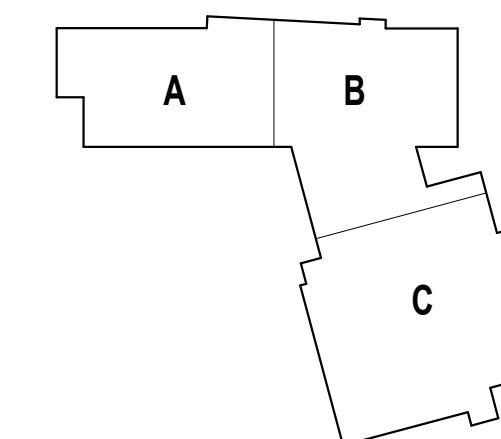
1 THIRD FLOOR ACADEMIC FRAMING PARTIAL PLAN
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

THIRD FLOOR
FRAMING
PARTIAL PLANS

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

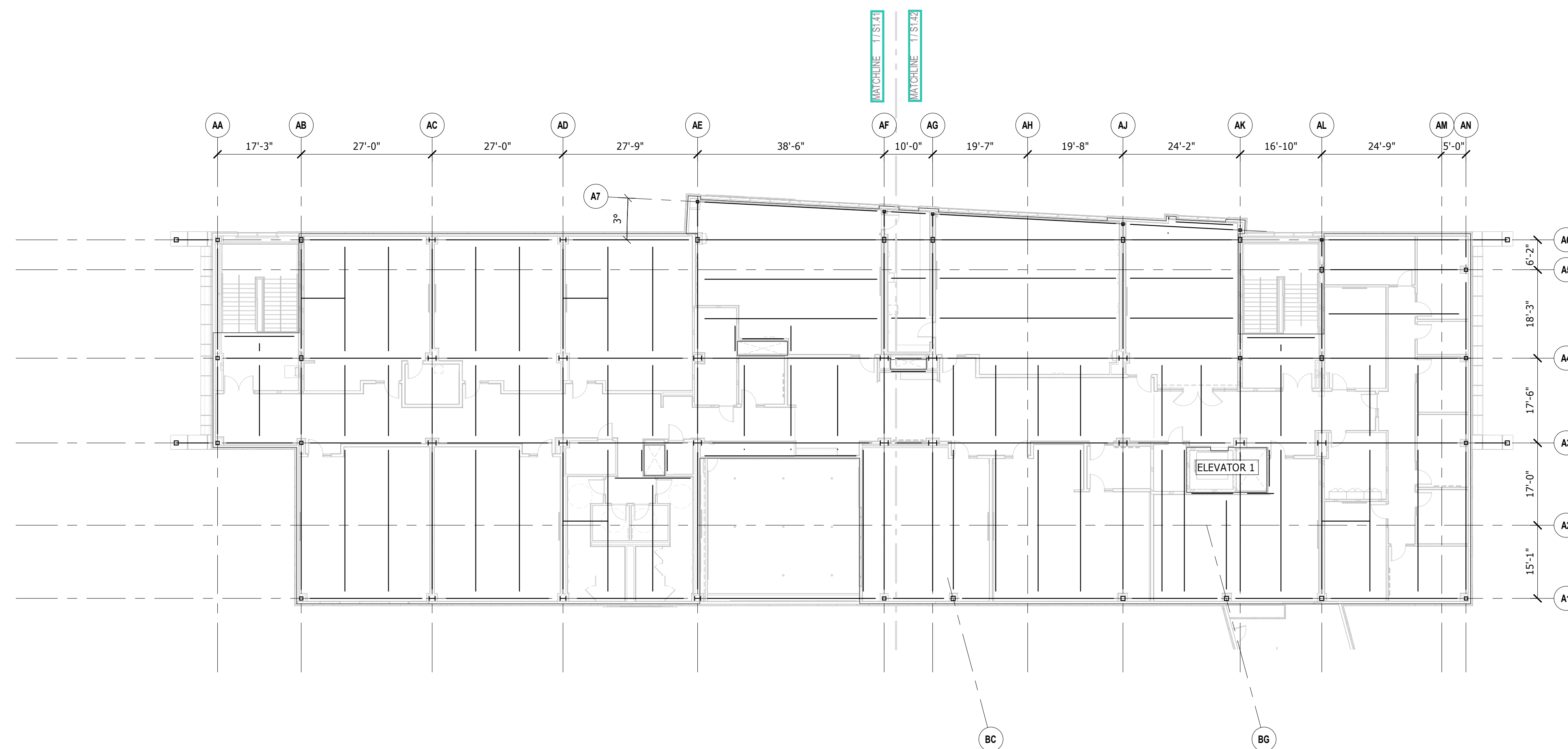
DATE: OCTOBER 13, 2023

S1.34



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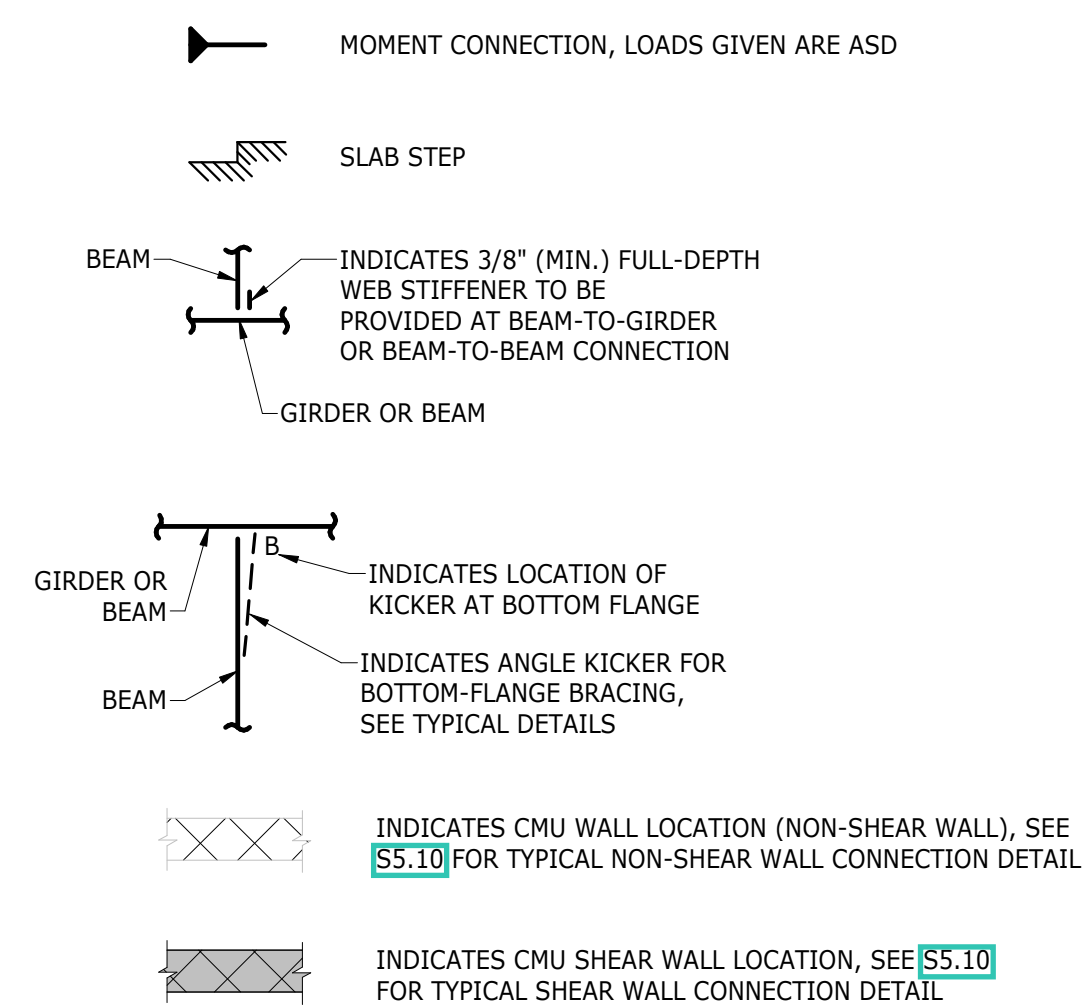
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FLOOR FRAMING NOTES:

- REFER TO DRAWINGS [S0.01](#) FOR STRUCTURAL NOTES AND [S0.21](#) THRU [S0.24](#) FOR TYPICAL DETAILS.
- 5 1/4" INDICATES SPAN DIRECTION OF 2", 18 GA. GALVANIZED COMPOSITE METAL DECK WITH 3 1/4" LIGHTWEIGHT CONCRETE TOPPING. TOTAL THICKNESS = 5 1/4". REINFORCE WITH 6x6 W2.1xW2.1 WWF. SEE PLANS AND TYPICAL DETAILS FOR ADDITIONAL REINFORCING. CONCRETE UNIT WEIGHT AND AIR-ENTRAIMENT SHALL BE PER PLANS AND SPECIFICATIONS.
- "X" SC INDICATES THE NUMBER OF 3/4" DIAMETER, 4" LONG SHEAR STUDS SPACED EVENLY ALONG THE BEAM UNLESS NOTED OTHERWISE. LENGTH SHALL BE FINAL LENGTH AFTER WELDING (i.e. 2" ABOVE TOP OF DECK).
- TOP OF FLOOR SLAB (TSL) = 141'-4", TYP., U.N.O.
TOP OF STEEL (TST) = 140'-10 3/4", TYP., U.N.O.
a. [-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST.
- [X] DENOTES CONNECTION DESIGN FORCES (SERVICE/ASD LOAD) IN KIPS. FORCES ARE VERTICAL UNLESS NOTED OTHERWISE AS FOLLOWS:
(H) = HORIZONTAL
(A) = AXIAL (NOTE AXIAL FORCES ON PLANS ARE IN ADDITION TO ANY AXIAL COMPONENT OF BRACE FRAME FORCES)
(M) = BENDING MOMENT IN STRONG DIRECTION
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS AND PROVIDE REINFORCEMENT PER TYPICAL DETAILS ON [S0.21](#) THRU [S0.23](#).
- SEE TYPICAL SECTION AT ISOLATED SLAB-ON-DECK DETAIL ON DWG [S0.21](#) COORDINATE ISOLATED SLAB-ON-DECK REQUIREMENTS WITH LOCAL SLAB EDGE, ROOF DECK SUPPORT, AND SHEAR WALL DETAILS AS REQUIRED.
- ALIGN BEAM CENTERLINE 9" OFF EDGE OF SLAB AT STAIR OPENING. REFER TO ARCH DWGS FOR EDGE OF SLAB LOCATIONS/DIMENSIONS.

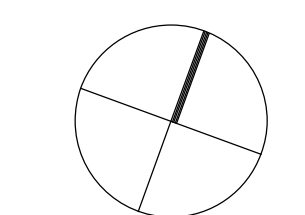
LEGEND:



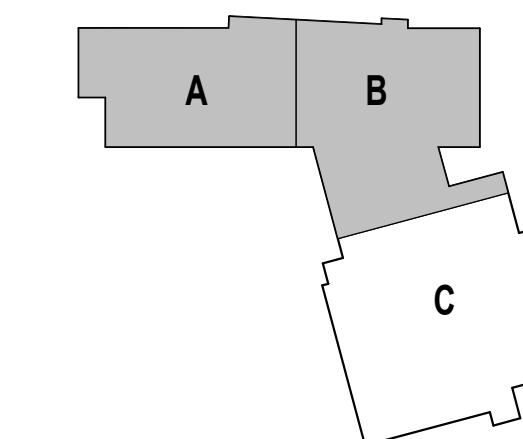
1 FOURTH FLOOR FRAMING: OVERALL PLAN
1/16" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**OVERALL
FOURTH FLOOR
FRAMING PLAN**

DRAWN BY: JDB / MSS

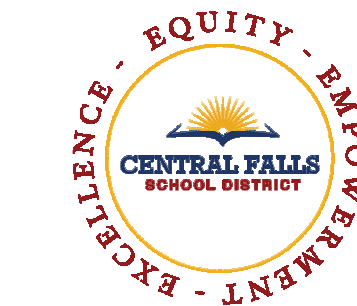
REVIEWED BY: MGM / BP

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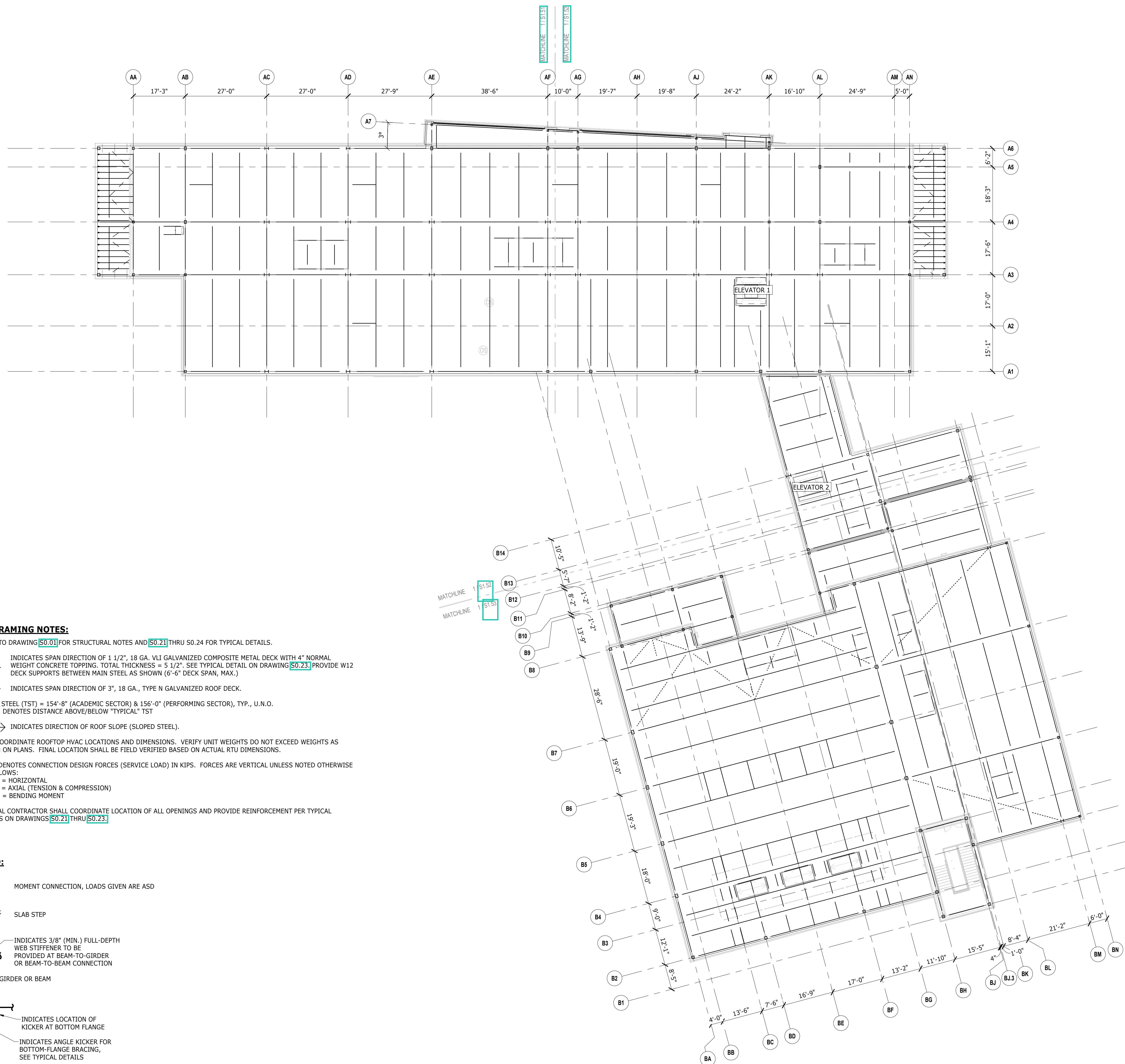
DATE: OCTOBER 13, 2023

S1.40



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ROOF FRAMING NOTES:

- REFER TO DRAWING [S0.01](#) FOR STRUCTURAL NOTES AND [S0.21](#) THRU [S0.24](#) FOR TYPICAL DETAILS.
- $5\frac{1}{2}$ " INDICATES SPAN DIRECTION OF 1 1/2", 18 GA. VLI GALVANIZED COMPOSITE METAL DECK WITH 4" NORMAL WEIGHT CONCRETE TOPPING, TOTAL THICKNESS = 5 1/2". SEE TYPICAL DETAIL ON DRAWING [S0.23](#) PROVIDE W12 DECK SUPPORTS BETWEEN MAIN STEEL AS SHOWN (6'-6" DECK SPAN, MAX.)
 3 " INDICATES SPAN DIRECTION OF 3", 18 GA., TYPE N GALVANIZED ROOF DECK.
- TOP OF STEEL (TST) = 154'-8" (ACADEMIC SECTOR) & 156'-0" (PERFORMING SECTOR), TYP., U.N.O.
 a. [-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST
- INDICATES DIRECTION OF ROOF SLOPE (SLOPED STEEL).
- G.C.: COORDINATE ROOFTOP HVAC LOCATIONS AND DIMENSIONS. VERIFY UNIT WEIGHTS DO NOT EXCEED WEIGHTS AS SHOWN ON PLANS. FINAL LOCATION SHALL BE FIELD VERIFIED BASED ON ACTUAL RTU DIMENSIONS.
- [X] DENOTES CONNECTION DESIGN FORCES (SERVICE LOAD) IN KIPS. FORCES ARE VERTICAL UNLESS NOTED OTHERWISE AS FOLLOWS:
 (H) = HORIZONTAL
 (A) = AXIAL (TENSION & COMPRESSION)
 (M) = BENDING MOMENT
- GENERAL CONTRACTOR SHALL COORDINATE LOCATION OF ALL OPENINGS AND PROVIDE REINFORCEMENT PER TYPICAL DETAILS ON DRAWINGS [S0.21](#) THRU [S0.23](#).

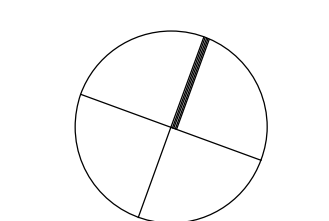
LEGEND:

- MOMENT CONNECTION, LOADS GIVEN ARE ASD
- SLAB STEP
- INDICATES 3/8" (MIN.) FULL-DEPTH WEB STIFFENER TO BE PROVIDED AT BEAM-TO-GIRDER OR BEAM-TO-BEAM CONNECTION
- INDICATES LOCATION OF KICKER AT BOTTOM FLANGE
- INDICATES ANGLE KICKER FOR BOTTOM-FLANGE BRACING, SEE TYPICAL DETAILS
- INDICATES CMU WALL LOCATION (NON-SHEAR WALL), SEE [S5.10](#) FOR TYPICAL NON-SHEAR WALL CONNECTION DETAIL
- INDICATES CMU SHEAR WALL LOCATION, SEE [S5.10](#) FOR TYPICAL SHEAR WALL CONNECTION DETAIL

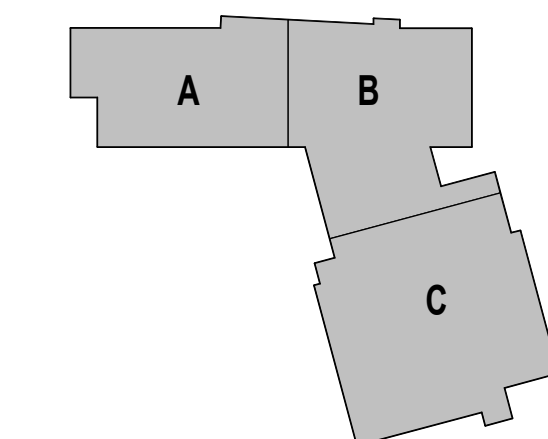
1 ROOF FRAMING: OVERALL PLAN
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

OVERALL ROOF FRAMING PLAN

DRAWN BY: JDB / MSS

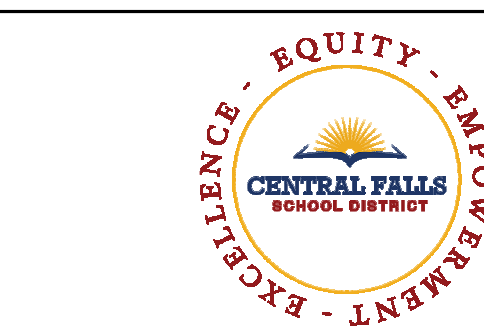
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

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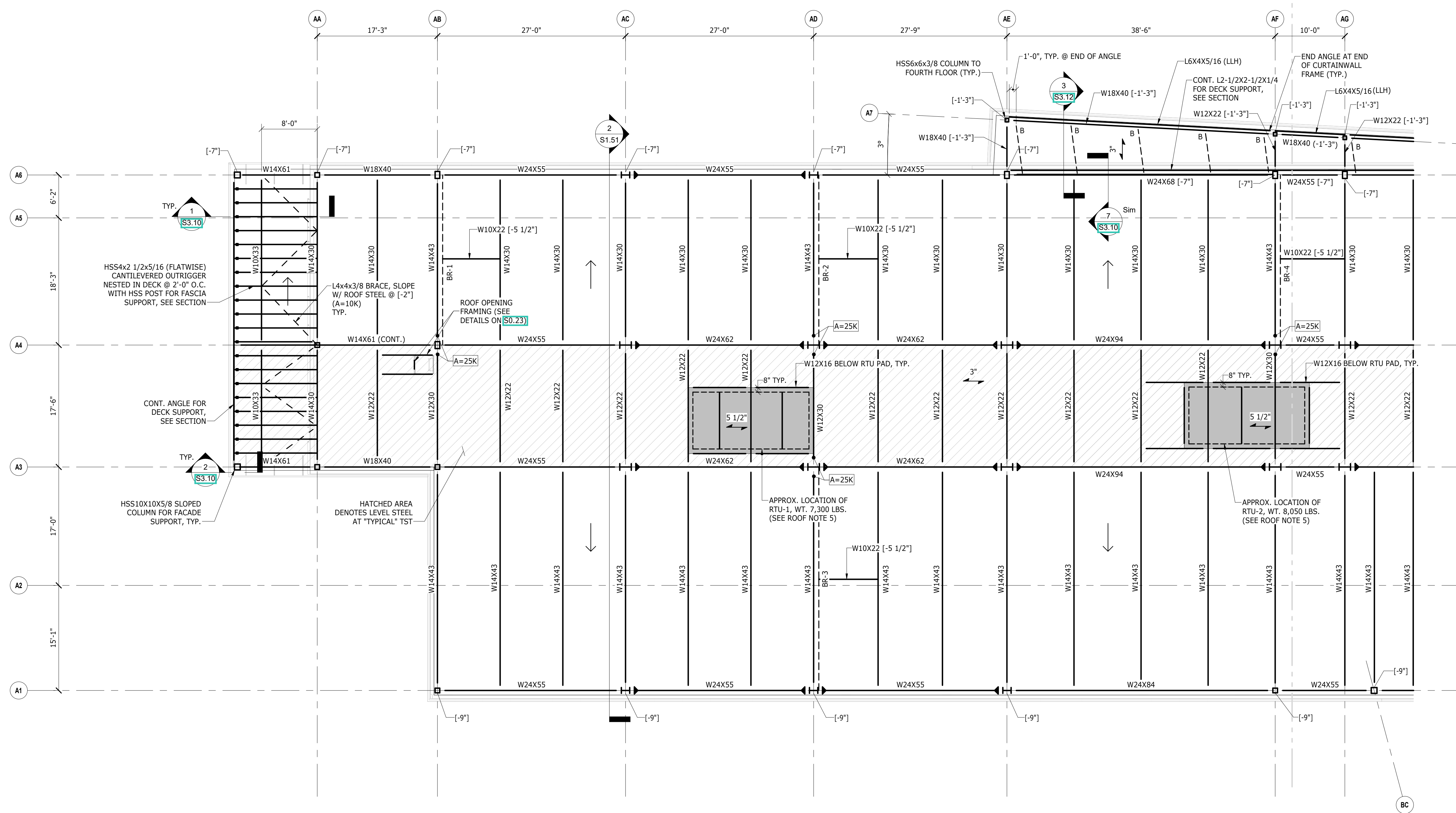
DATE: OCTOBER 13, 2023

S1.50

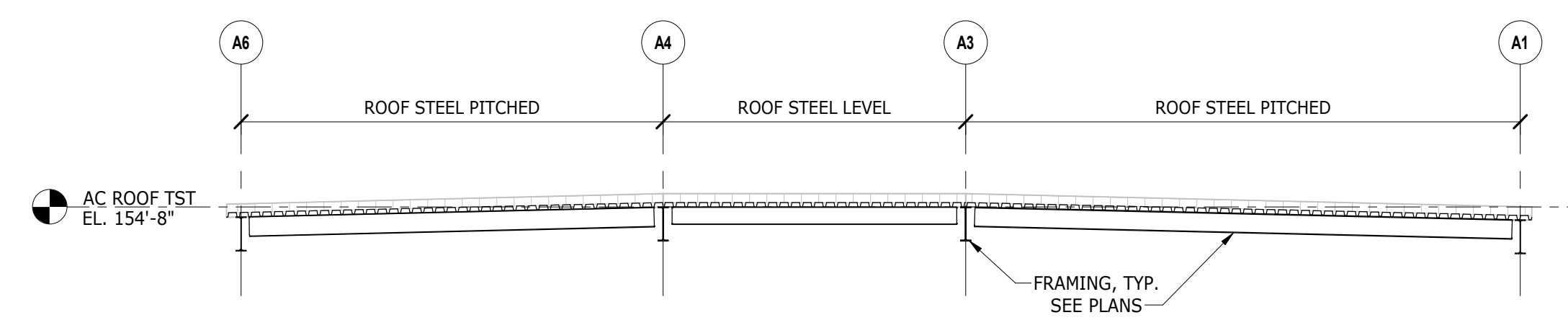


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1 ROOF FRAMING PLAN - ZONE A
1/8" = 1'-0"

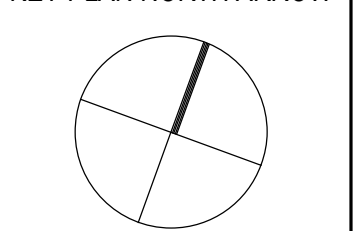


2 SECTION - MAIN ROOF
1/8" = 1'-0"

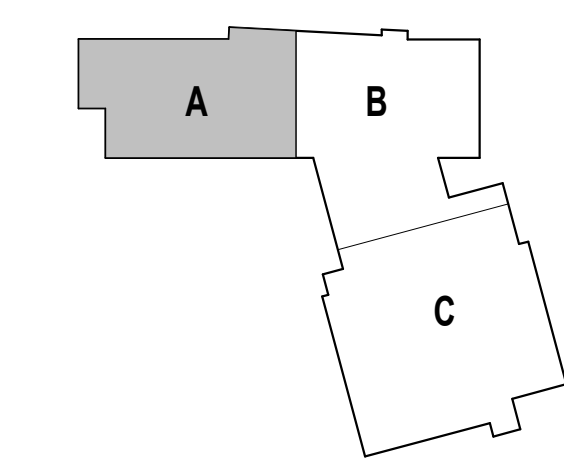
REFER TO DRAWING S1.30
FOR SCHEDULES, NOTES, ETC.

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



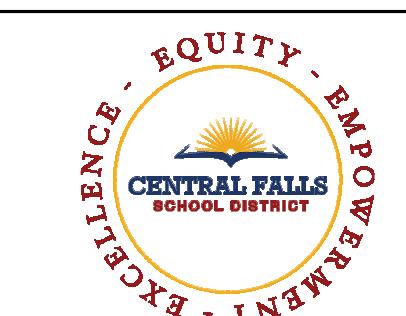
KEYPLAN



DRAWING NAME:

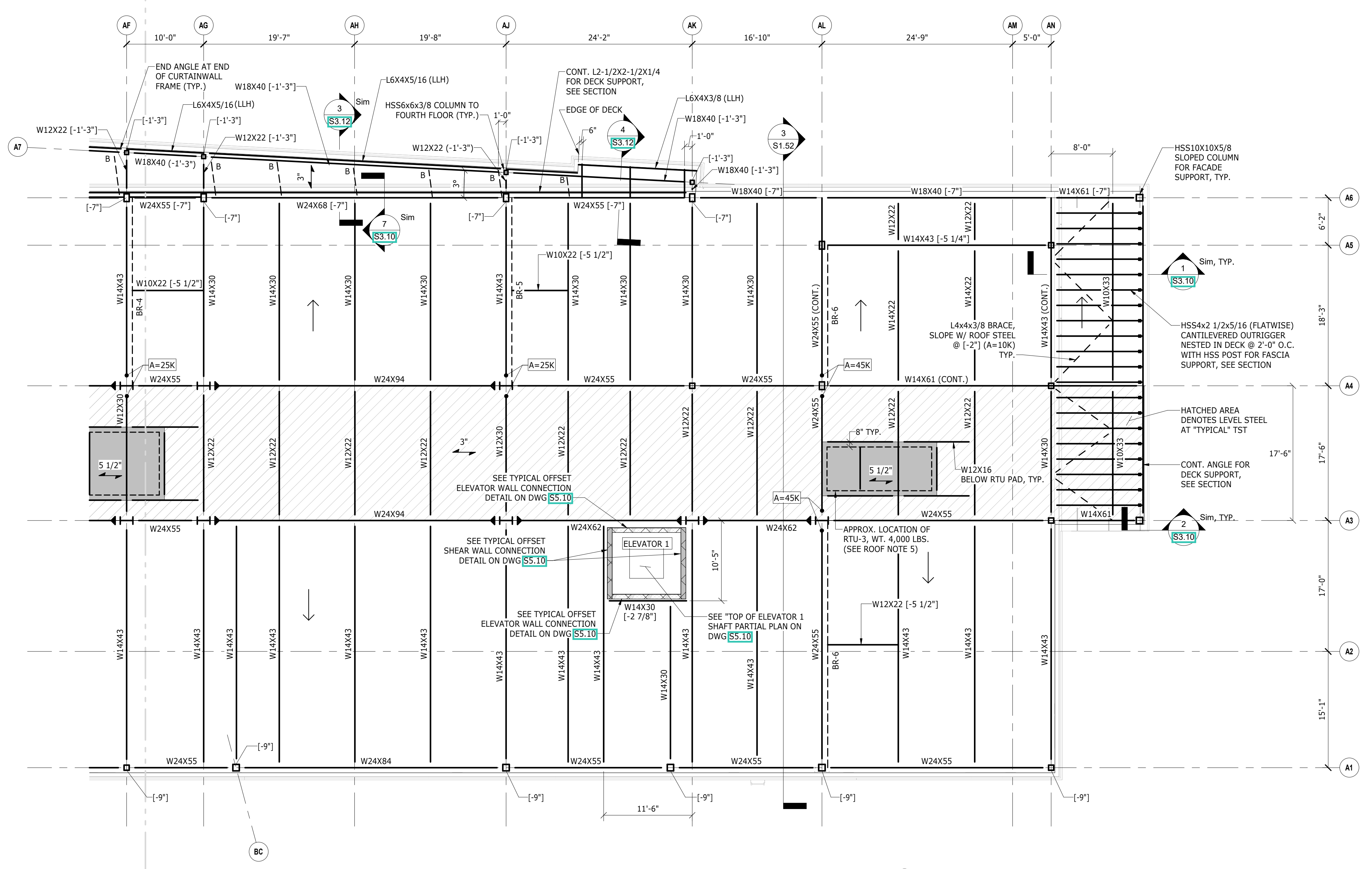
**ROOF FRAMING
PLAN - ZONE A**

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S1.51**

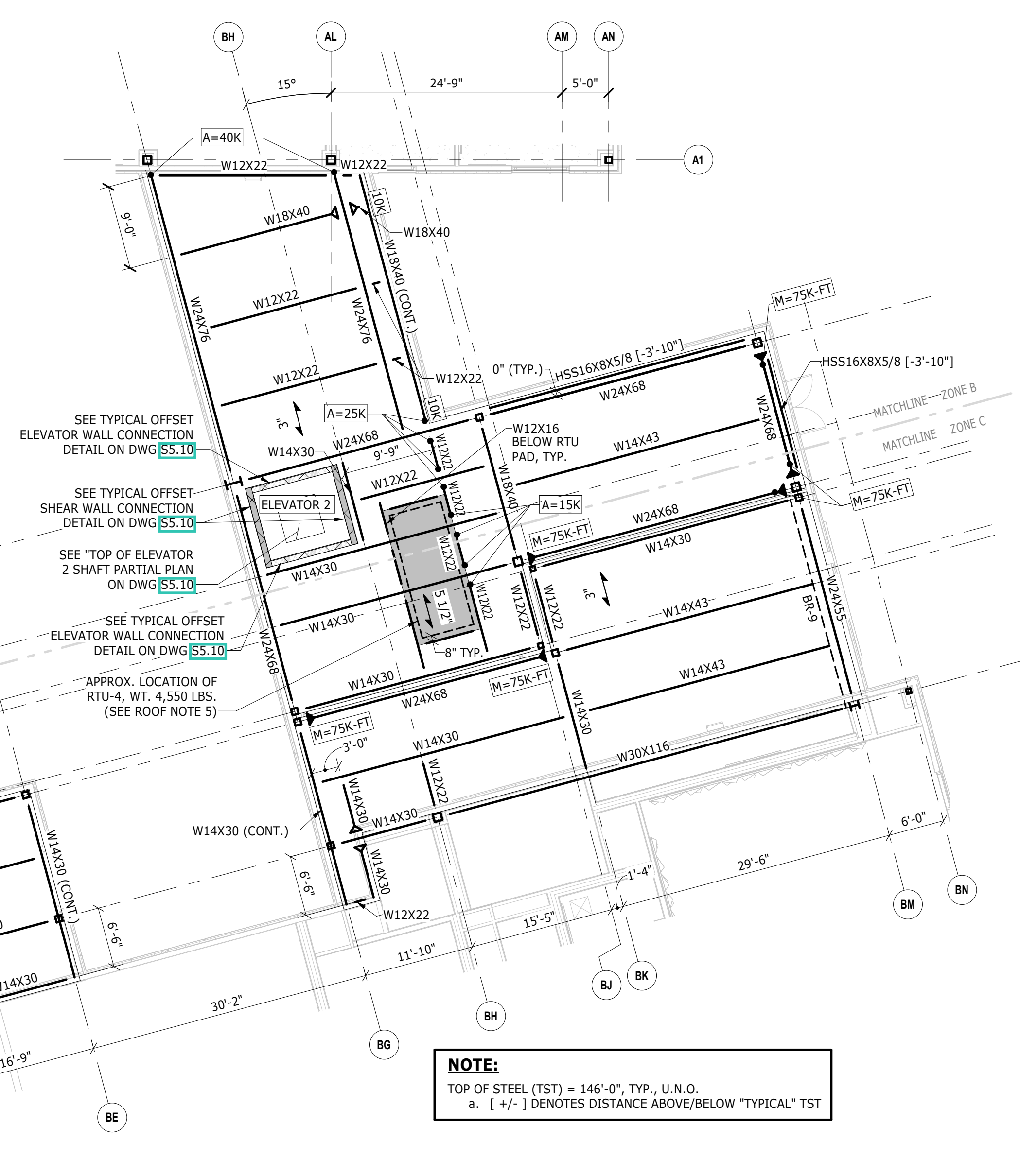


CENTRAL FALLS HIGH SCHOOL
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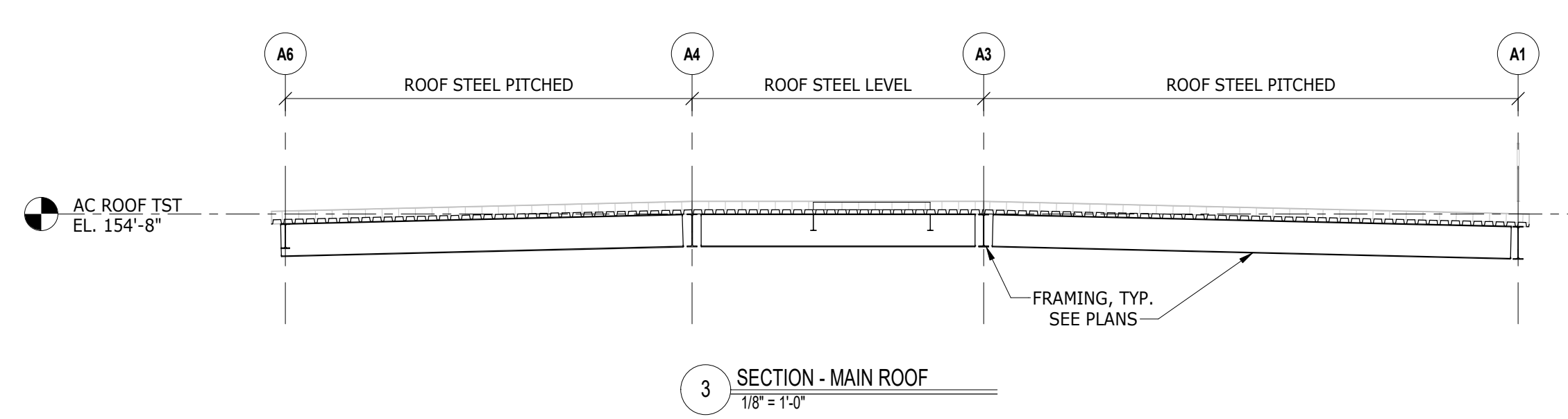
1 ROOF FRAMING PLAN - ZONE B
1/8" = 1'-0"



NOTE:
TOP OF STEEL (TST) = 146'-0", TYP., U.N.O.
a. [+/-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST

2 LOW ROOF FRAMING: OVERALL PLAN
1/8" = 1'-0"

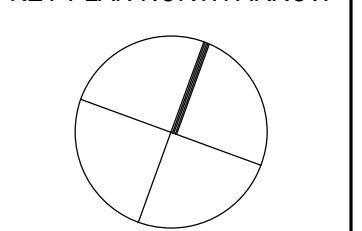
REFER TO DRAWING S1.30
FOR SCHEDULES, NOTES, ETC.



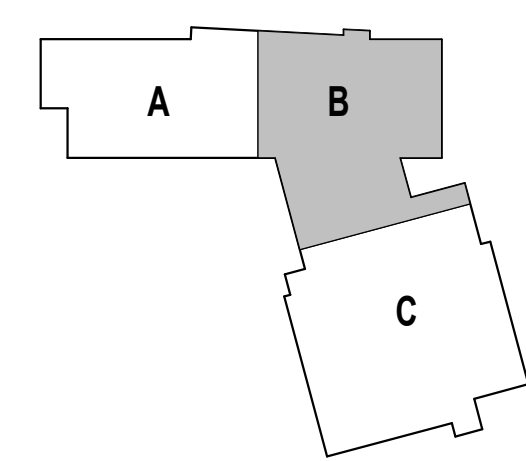
3 SECTION - MAIN ROOF
1/8" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



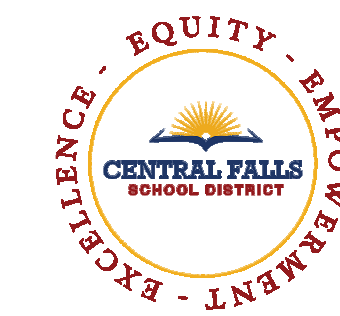
KEYPLAN



DRAWING NAME:

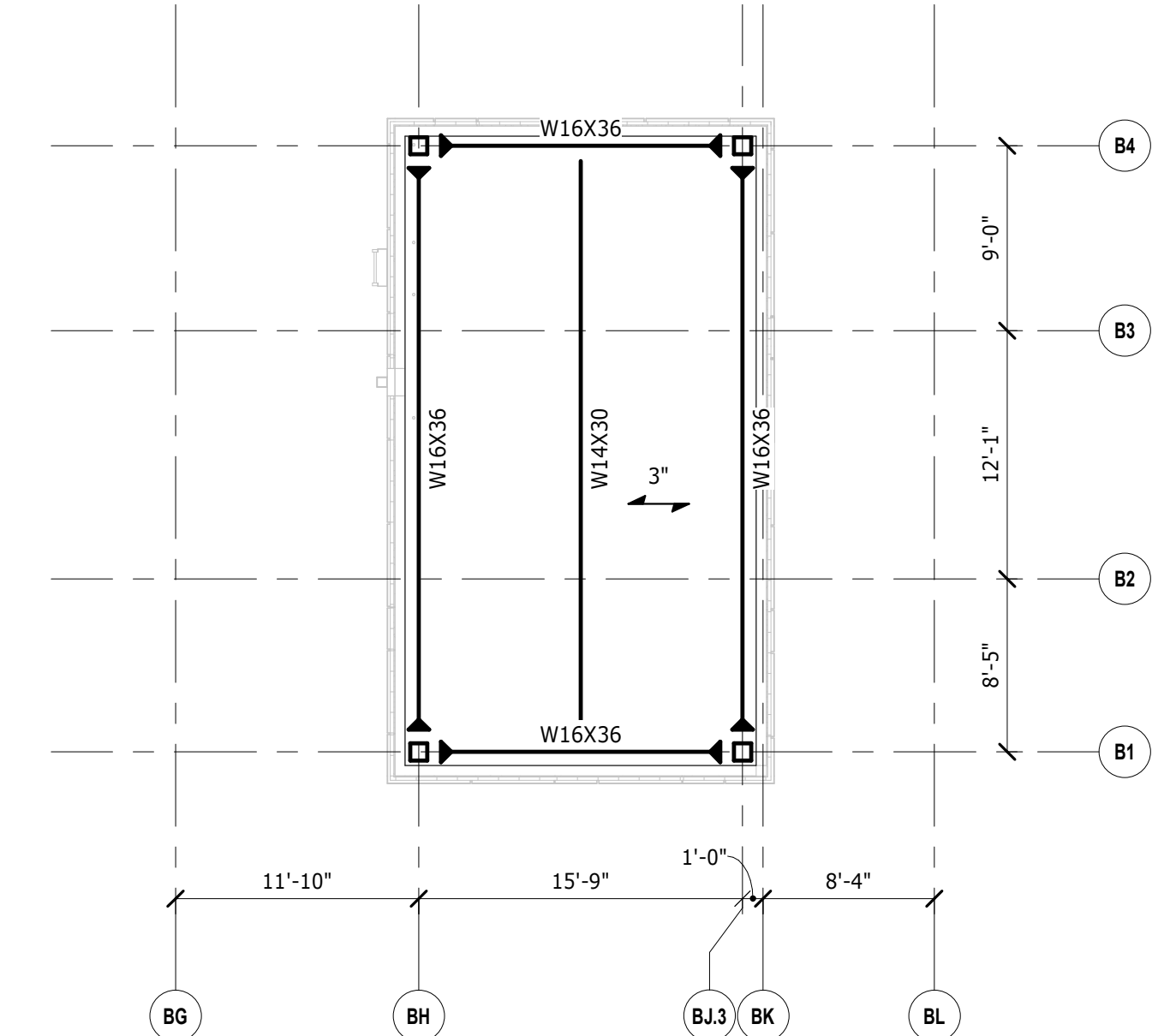
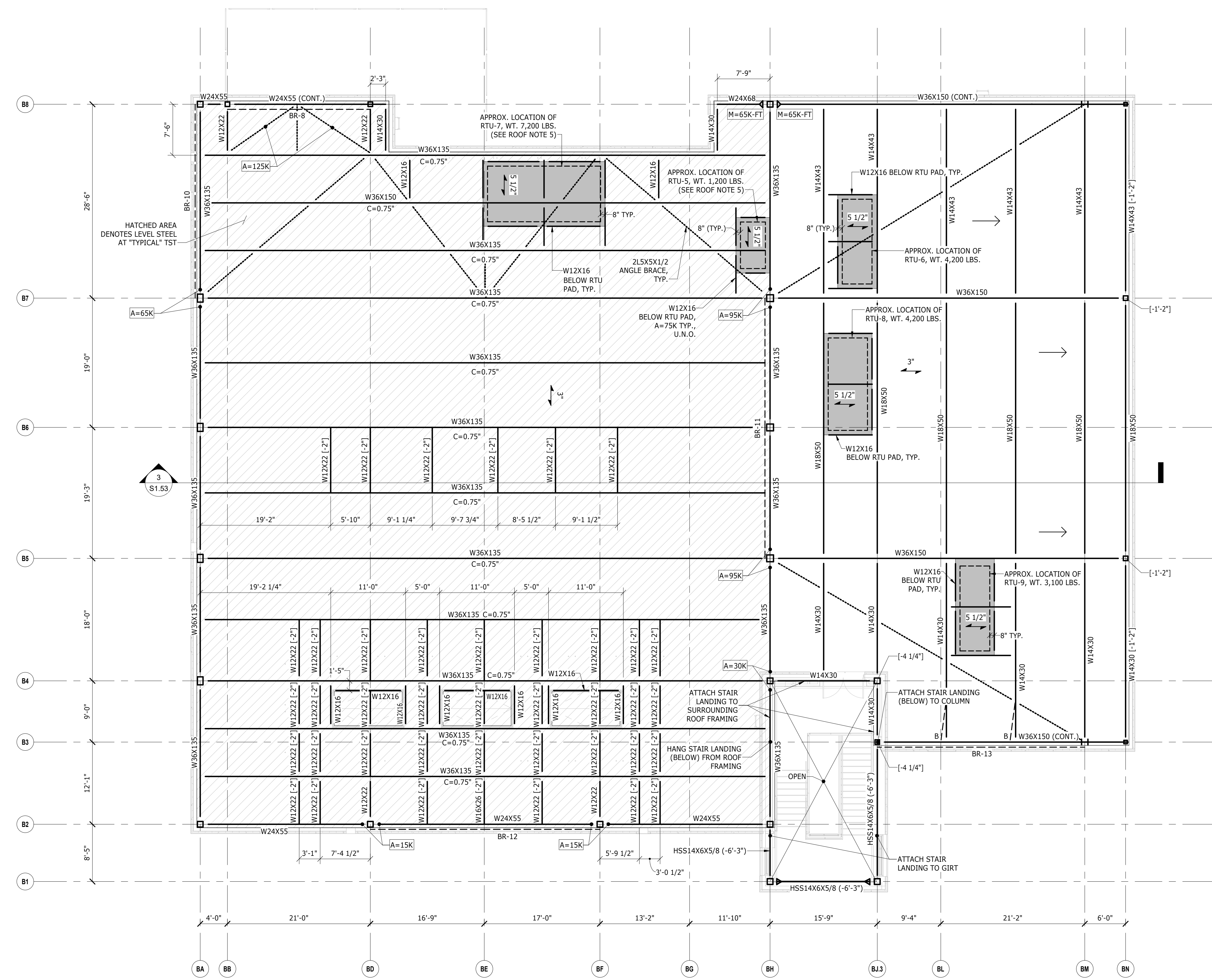
**ROOF FRAMING
PLAN - ZONE B**

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S1.52**



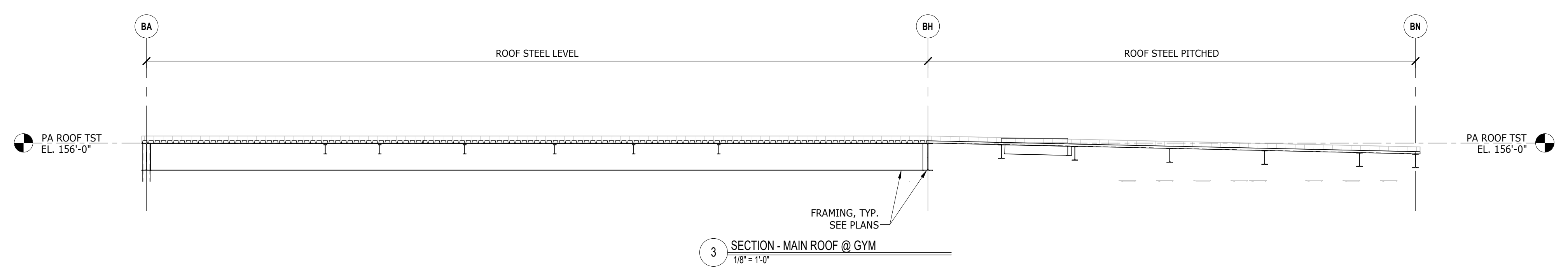
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NOTE:
TOP OF STEEL (TST) = 167'-0", TYP., U.N.O.
a. [+/-] DENOTES DISTANCE ABOVE/BELOW "TYPICAL" TST

2 STAIR TOWER ROOF FRAMING
1/8" = 1'-0"



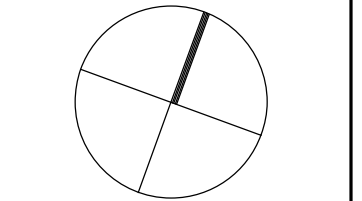
3 SECTION - MAIN ROOF @ GYM
1/8" = 1'-0"

1 ZONE PLAN 5 ROOF TST - ZONE C
1/8" = 1'-0"

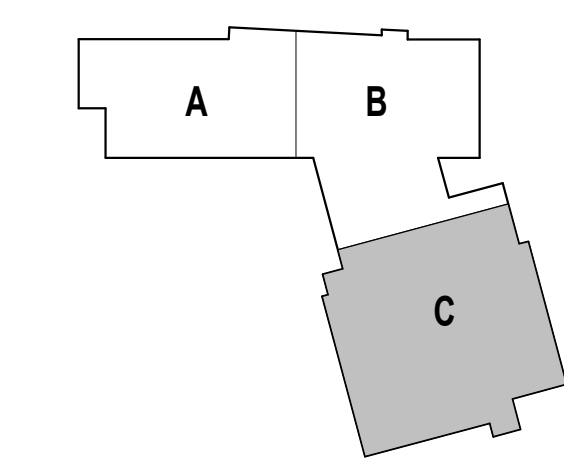
REFER TO DRAWING S1.30
FOR SCHEDULES, NOTES, ETC.

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



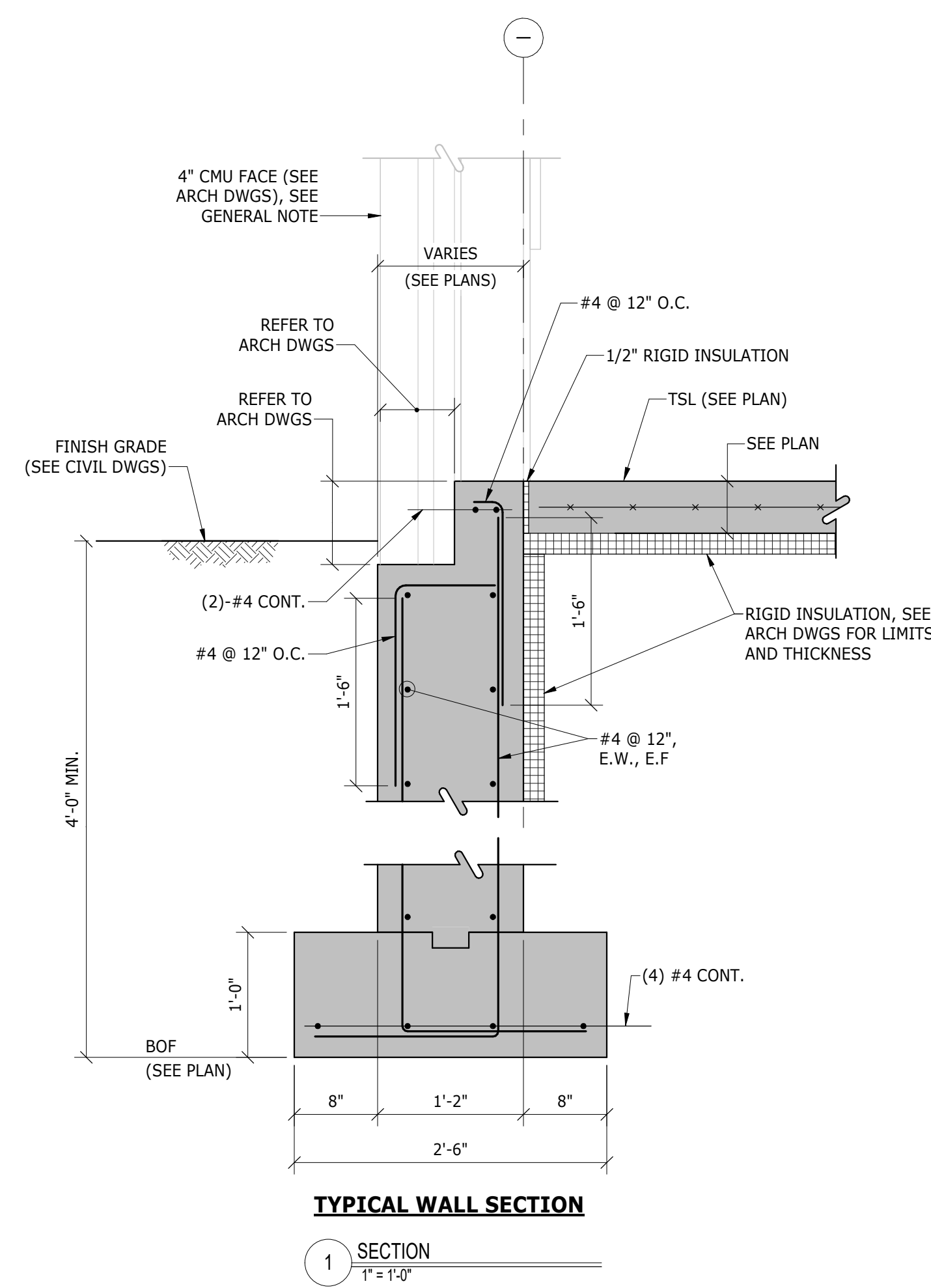
KEYPLAN



DRAWING NAME:

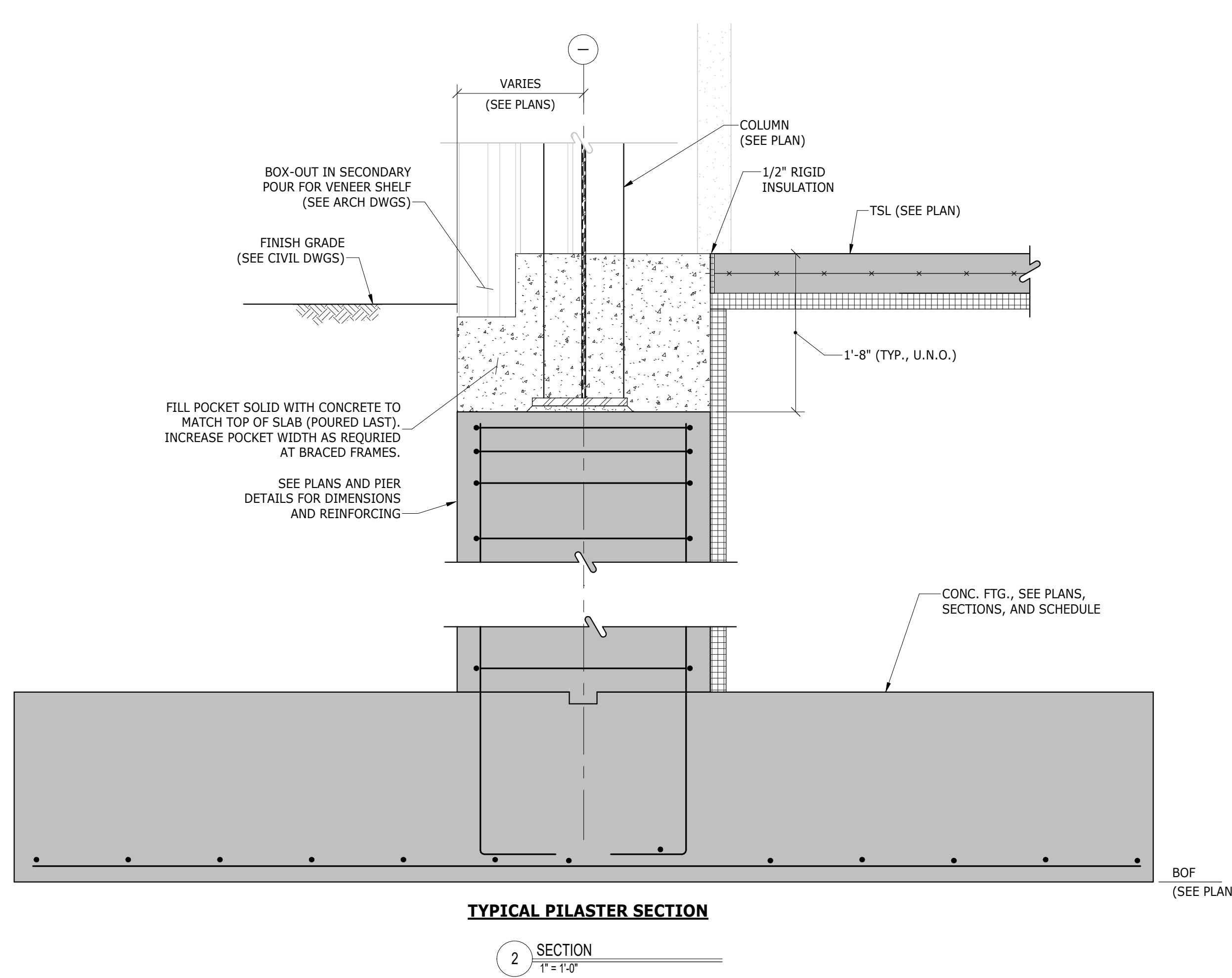
**ROOF FRAMING
PLAN - ZONE C**

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S1.53**



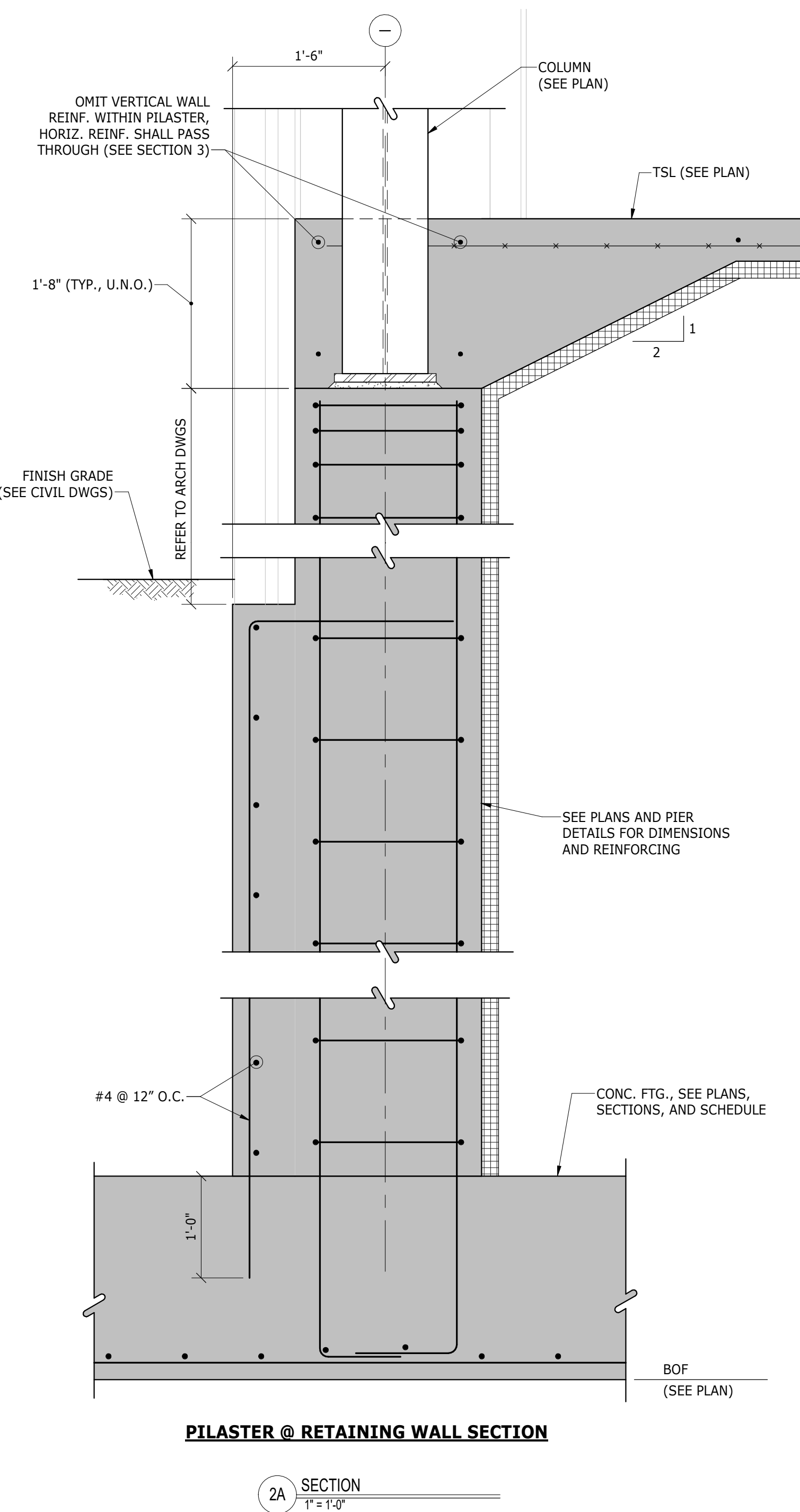
TYPICAL WALL SECTION

1 SECTION
1" = 1'-0"



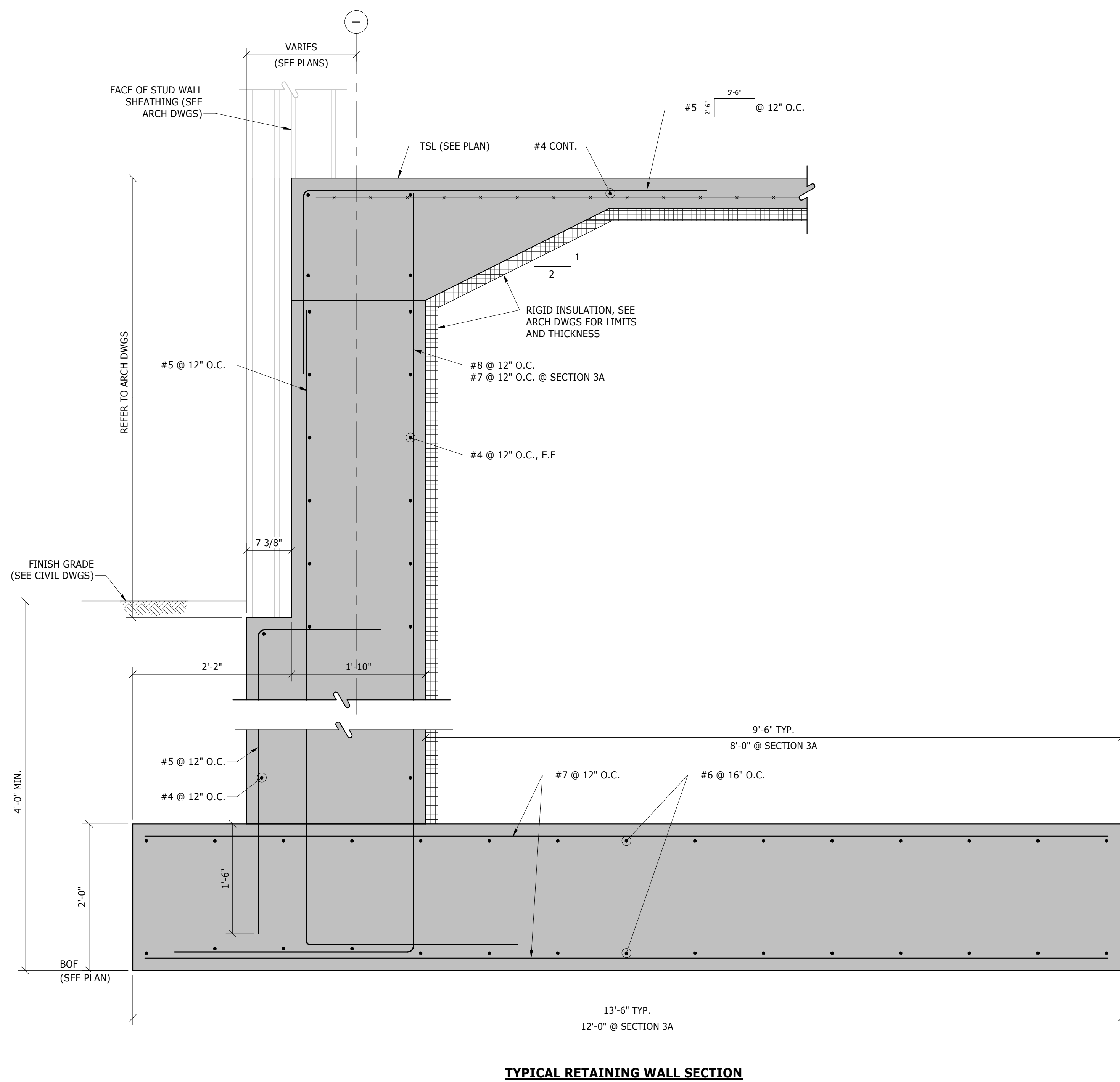
TYPICAL PILASTER SECTION

2 SECTION
1" = 1'-0"



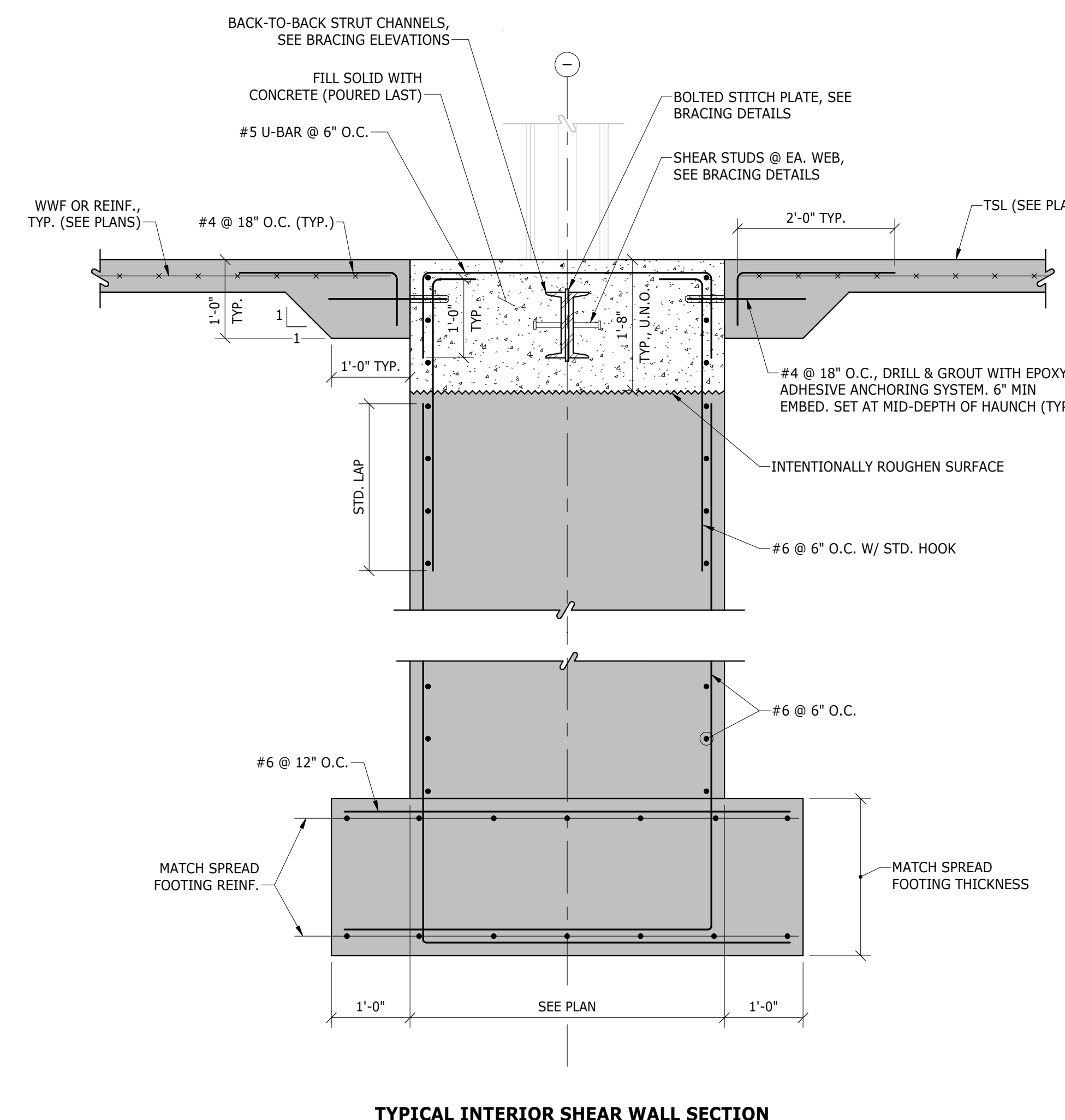
PILASTER @ RETAINING WALL SECTION

2A SECTION
1" = 1'-0"



TYPICAL RETAINING WALL SECTION

3A SECTION
3" = 1'-0"



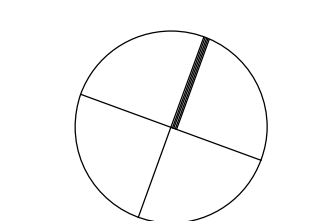
TYPICAL INTERIOR SHEAR WALL SECTION

4 SECTION
3/4" = 1'-0"

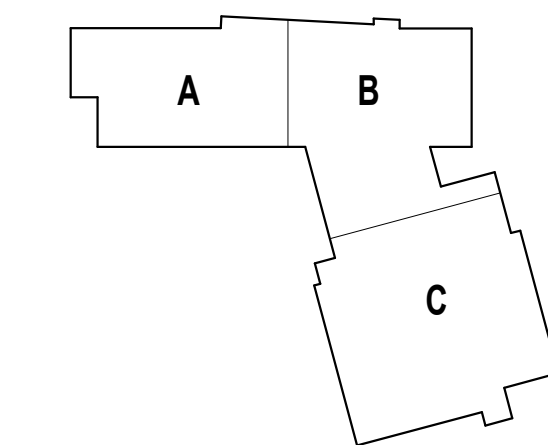
GENERAL NOTE:
REFER TO ARCHITECTURAL DRAWINGS FOR VENEER GEOMETRY AND LOCATION. OUTSIDE FACE OF VENEER DOES NOT NECESSARILY ALIGN WITH OUTSIDE FACE OF FOUNDATION

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEY PLAN



DRAWING NAME:

FOUNDATION SECTIONS AND DETAILS - 1

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

S2.10



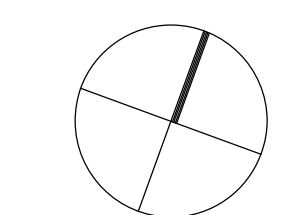
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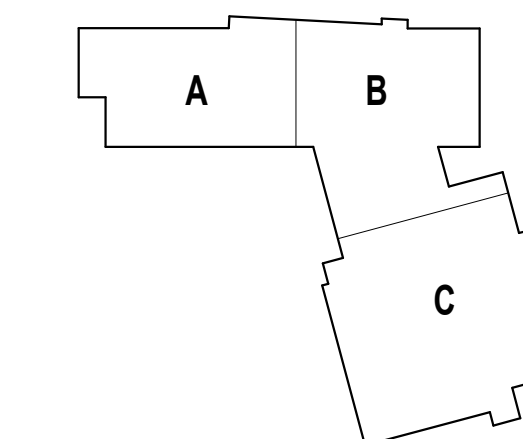
KEYNOTE LEGEND:

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

**FOUNDATION
SECTIONS AND
DETAILS - 2**

DRAWN BY: JDB / MSS

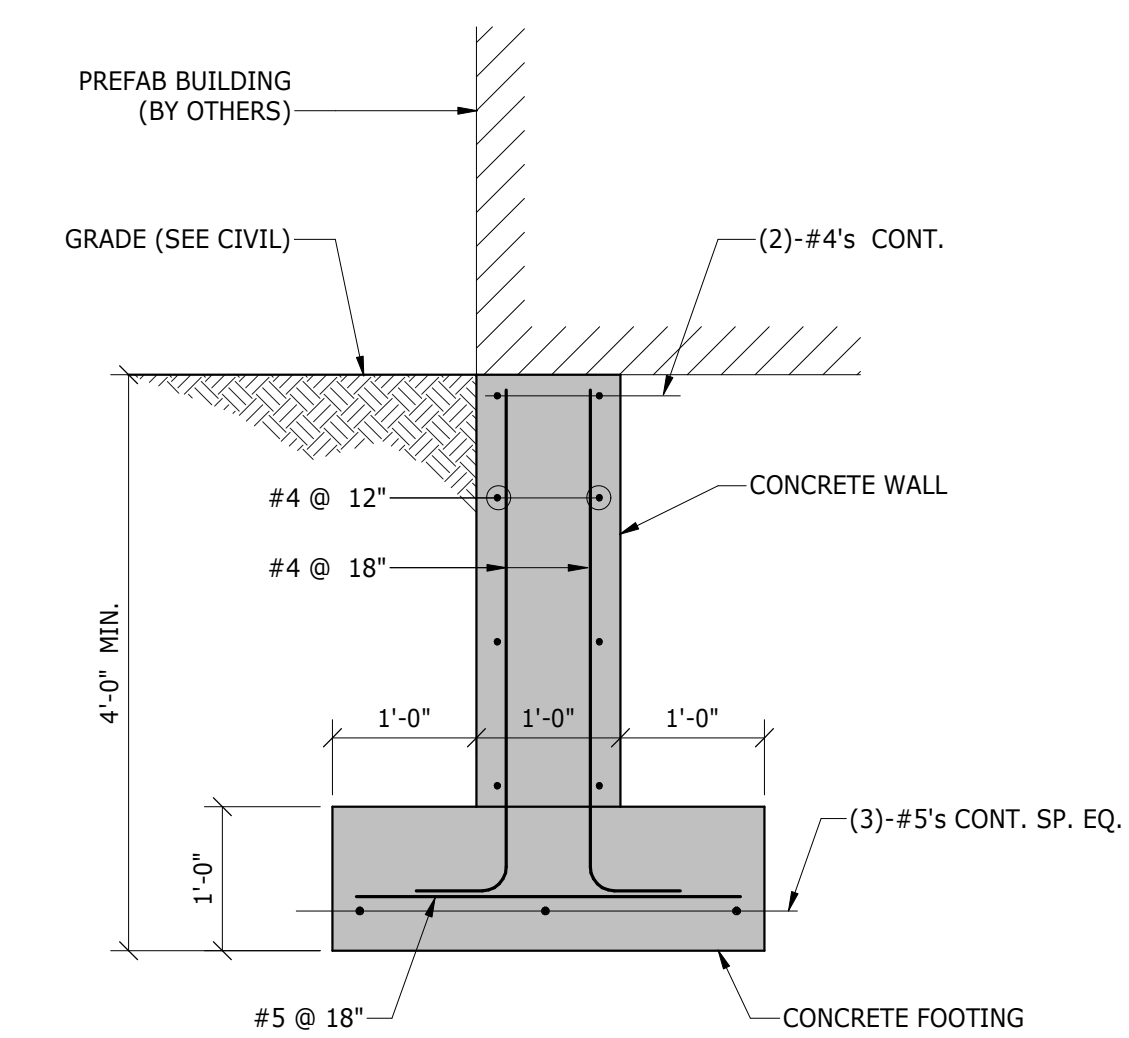
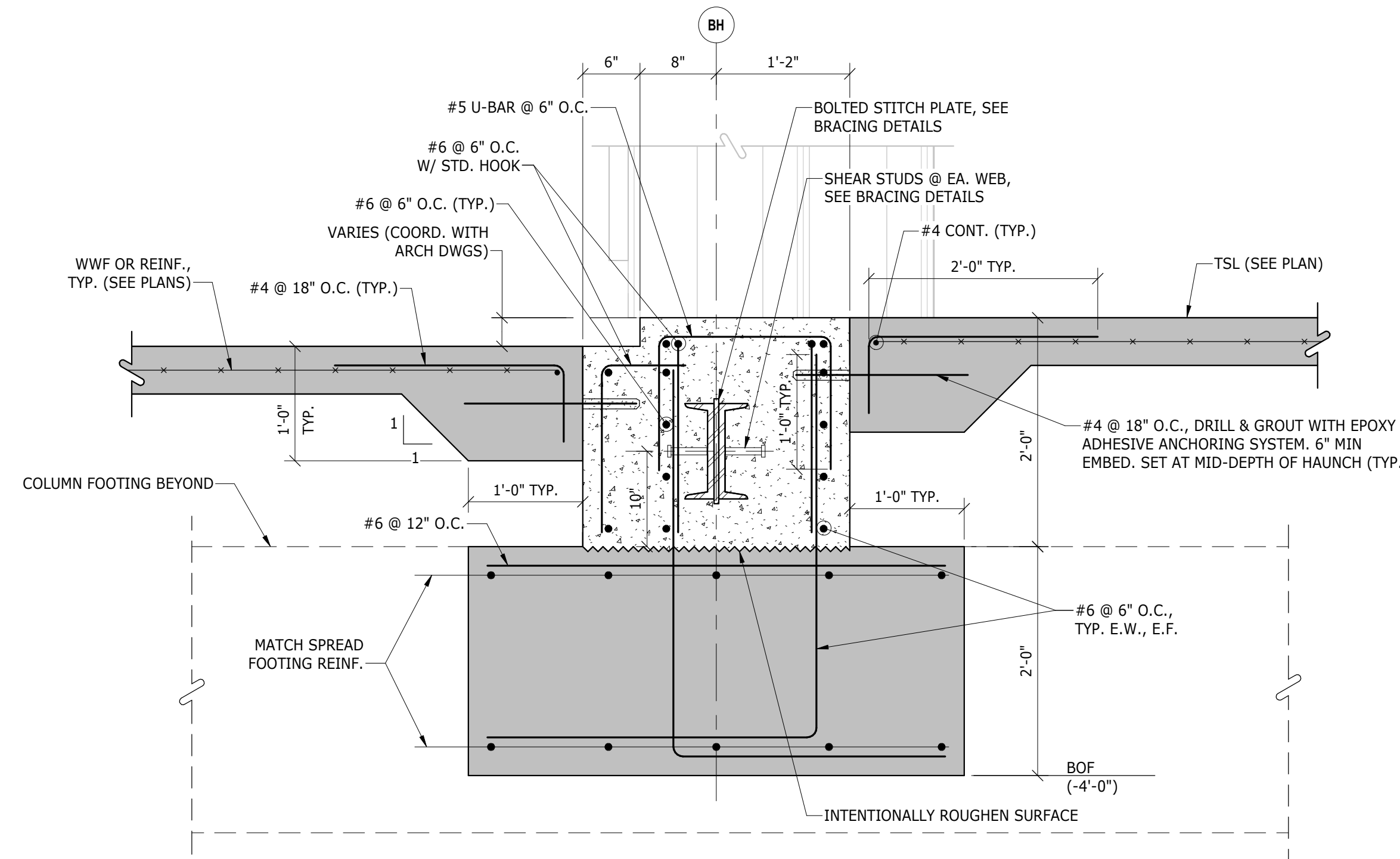
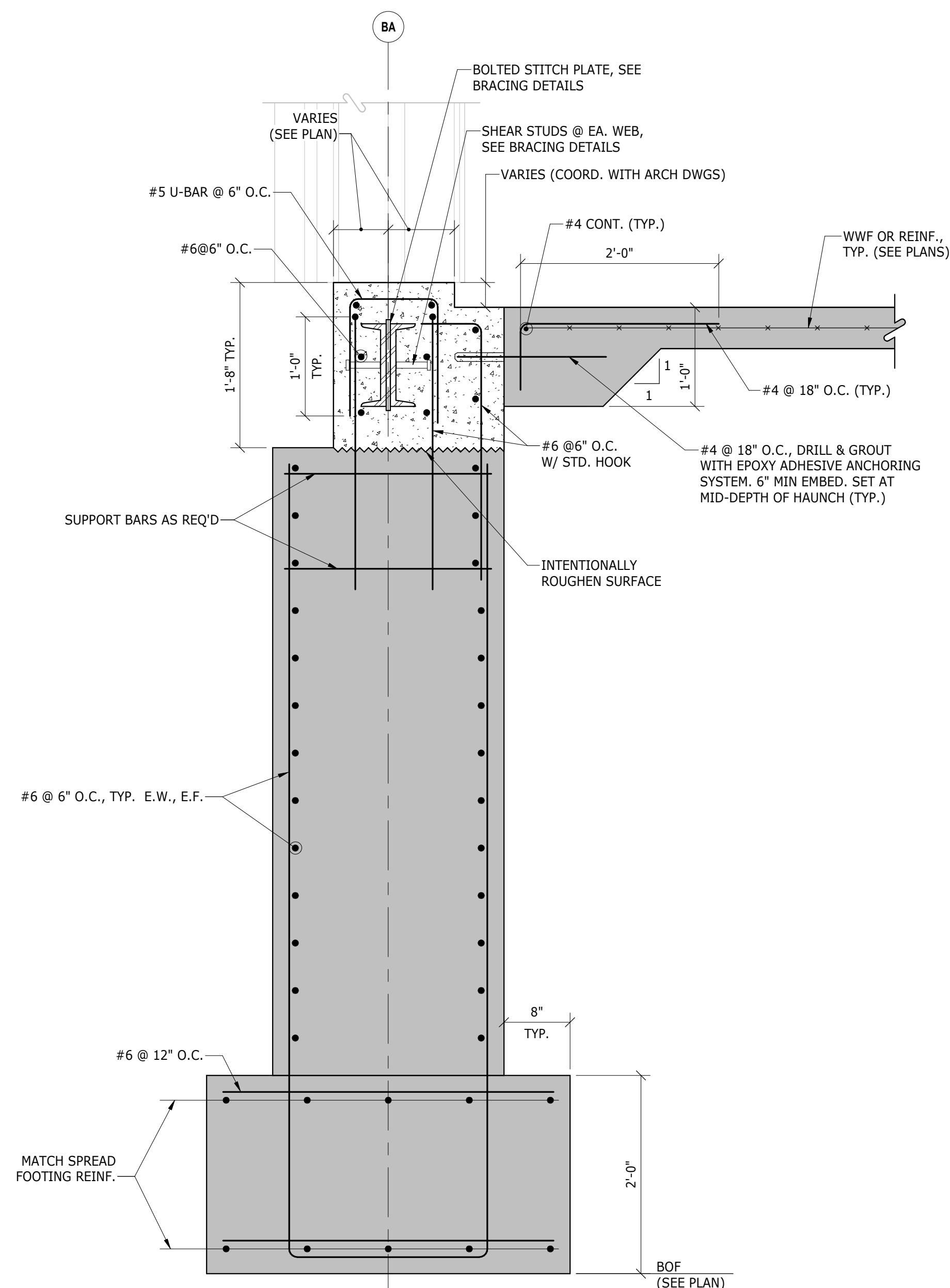
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

S2.11

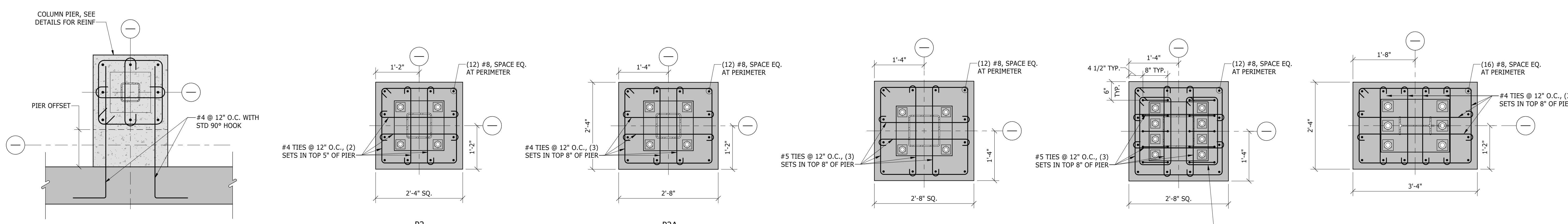


CONCESSIONS BUILDING FOUNDATION

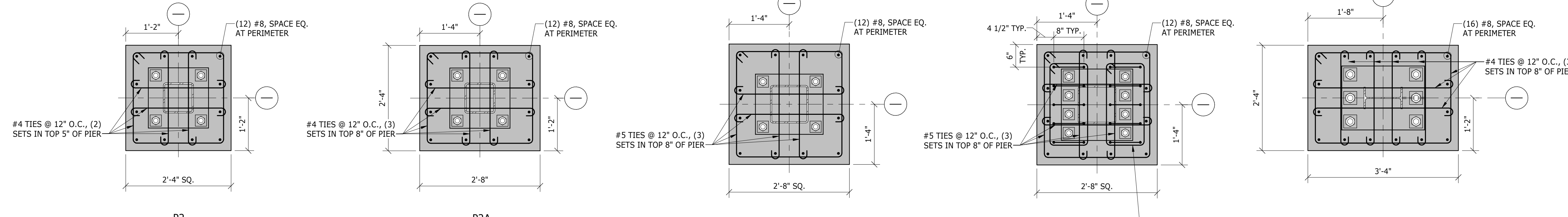
NOTES:

- REFER TO ARCHITECTURAL DRAWINGS FOR BUILDING DIMENSIONS.
- REFER TO CIVIL DRAWINGS FOR UTILITY LOCATION AND INVERTS. UTILITIES SHALL NOT PASS THROUGH FOOTINGS. CONTRACTOR SHALL COORDINATE FOUNDATION STEPS AS REQUIRED. REFER TO DRAWING [SD.11](#) TYPICAL ELEVATION OF STEPPED WALL FOOTING AT UTILITY OPENING DETAIL* FOR STEP REQUIREMENTS AND DETAILS.

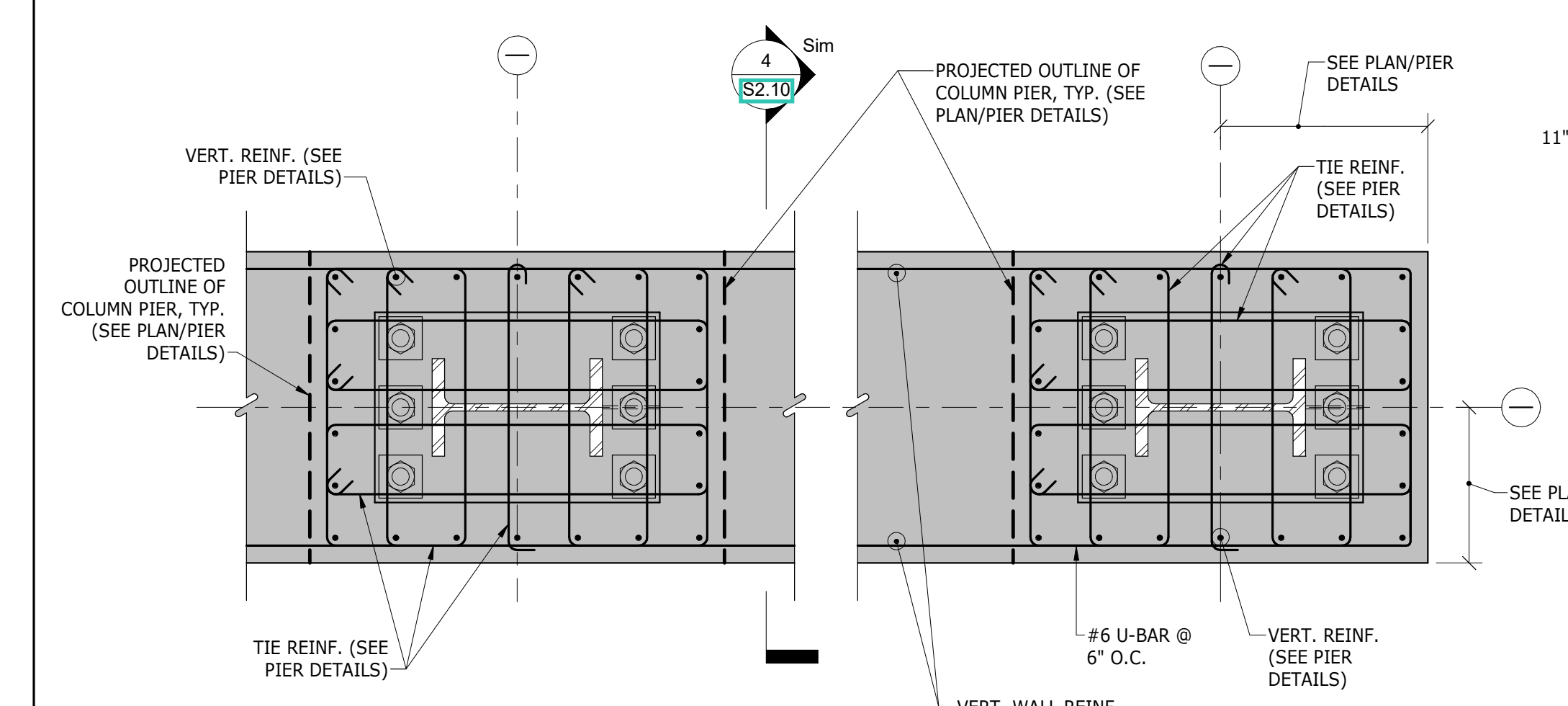
CONCESSION BUILDING FOUNDATION SECTION
NOT TO SCALE



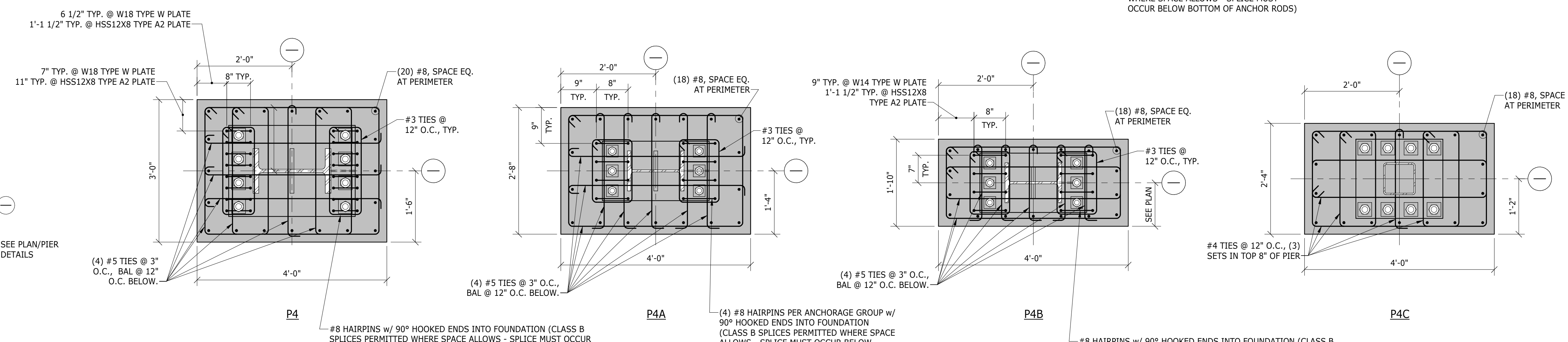
PIER OFFSET FROM END WALL



P2 P2A P2B P2C P3

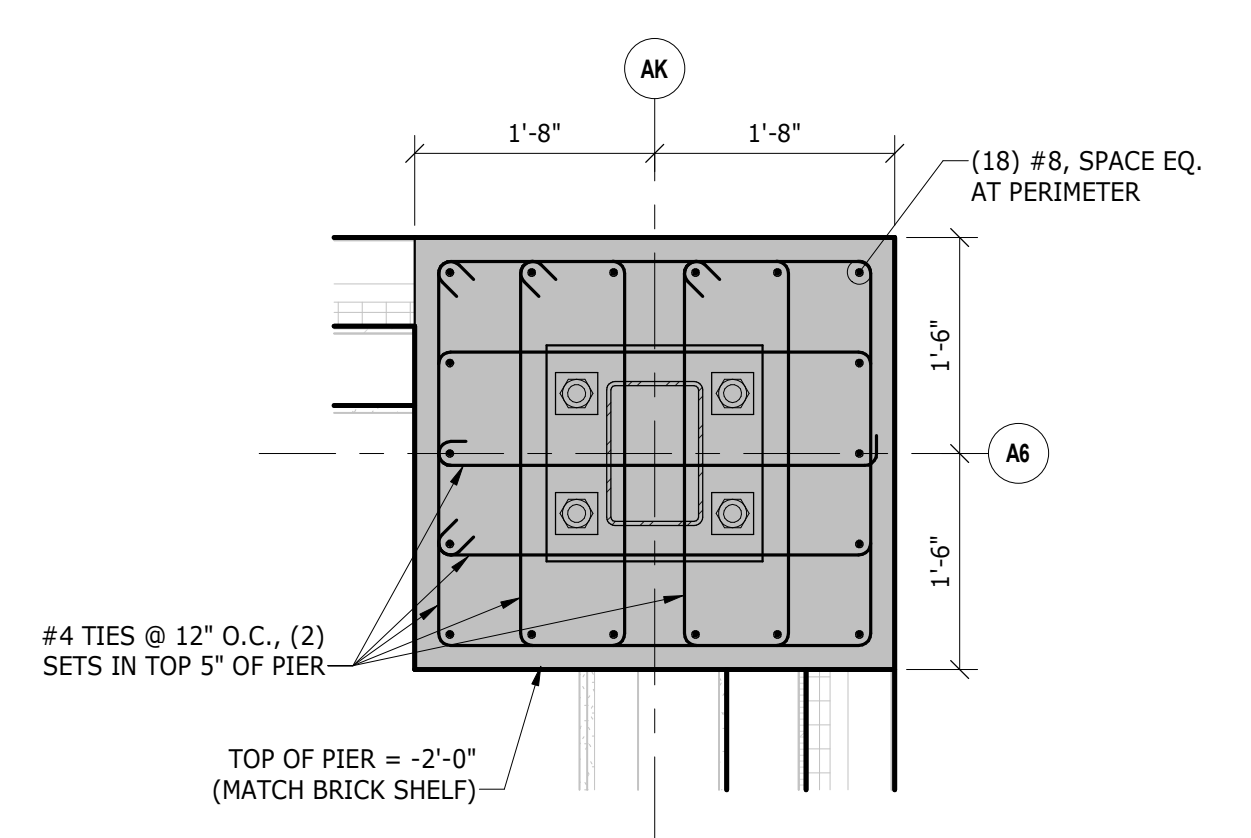


PIER AT BRACED FRAME SHEAR WALL

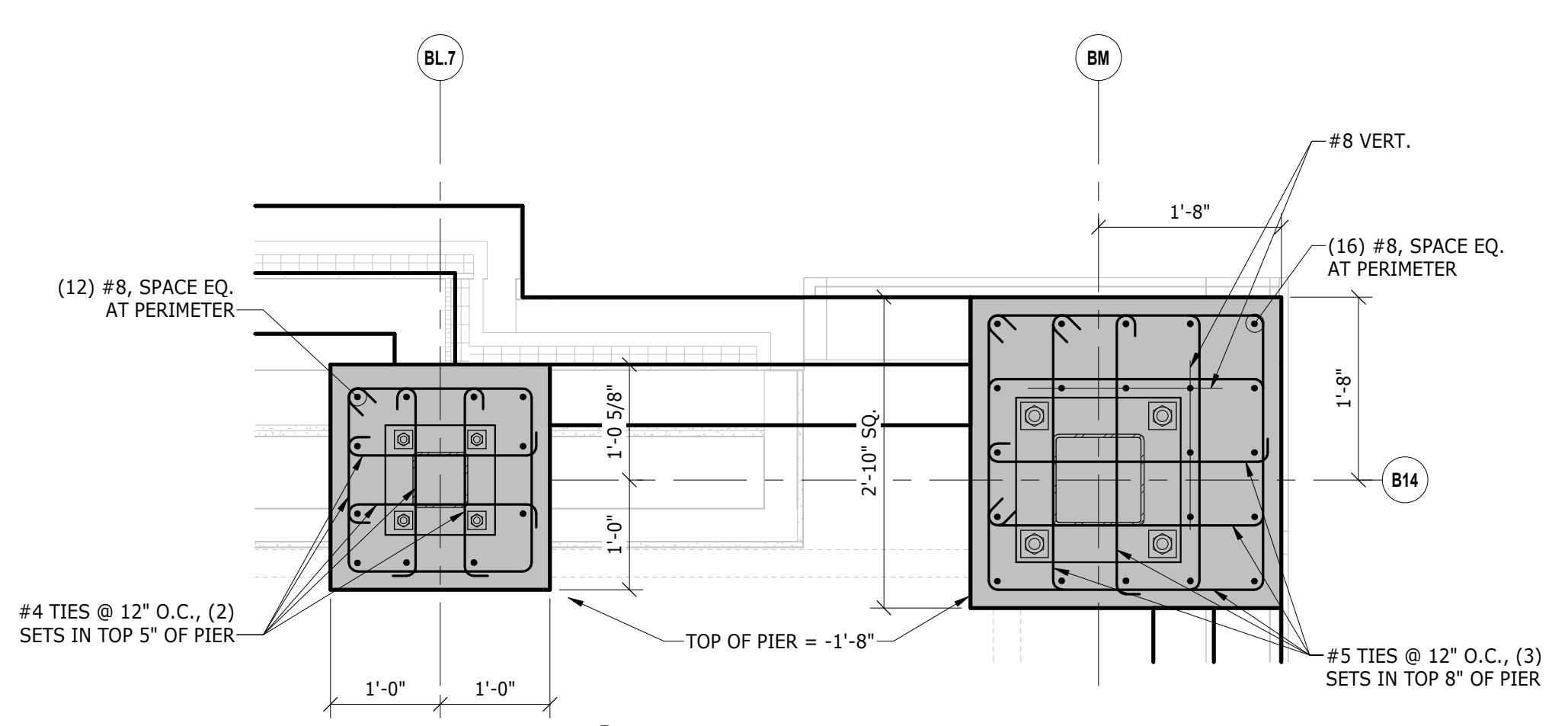


P4 P4A P4B P4C

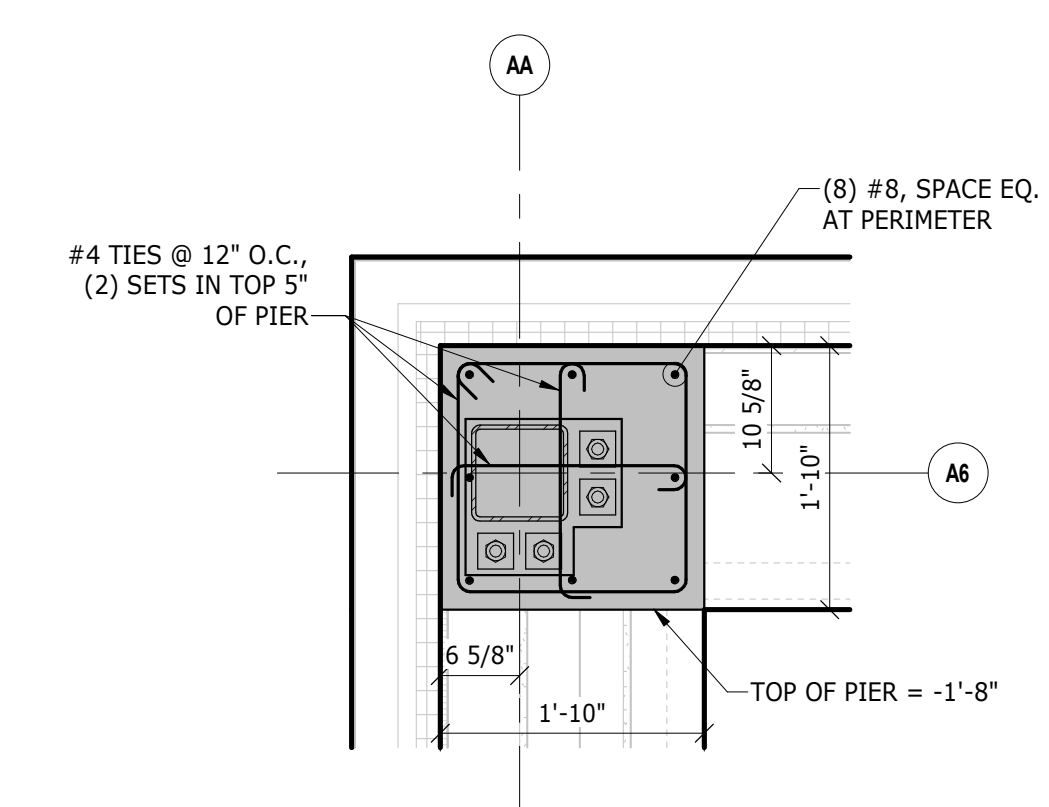
PILASTER AND PIER DETAILS
NOT TO SCALE



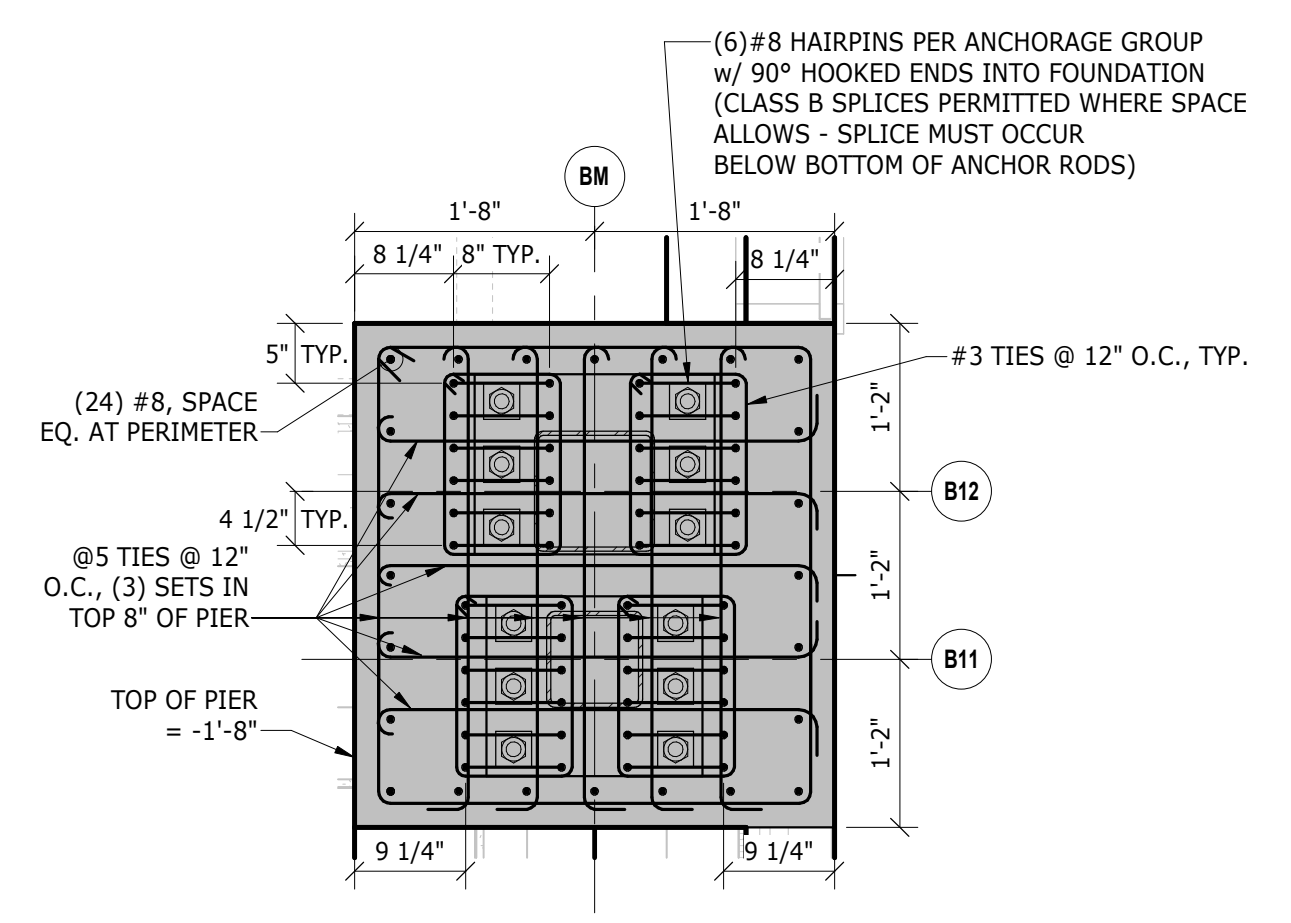
1 PIER PARTIAL PLAN @ AK/A6
3/4" = 1'-0"



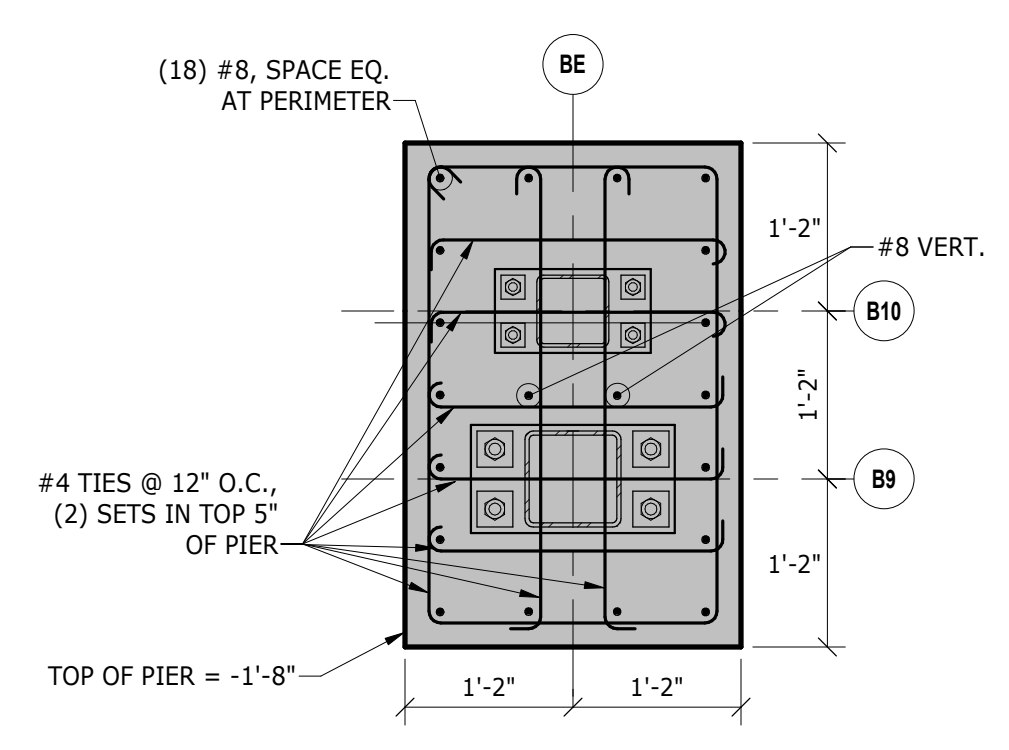
3 PIER PARTIAL PLAN @ BMB14
3/4" = 1'-0"



2 PIER PARTIAL PLAN @ AA/A6
3/4" = 1'-0"



4 PIER PARTIAL PLAN @ BMB11 & B12
3/4" = 1'-0"



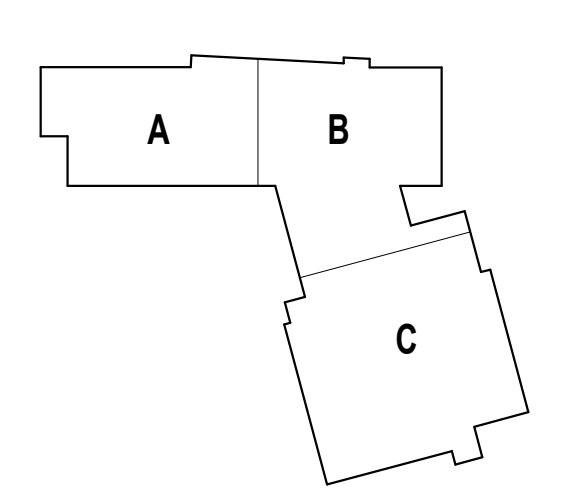
5 PIER PARTIAL PLAN @ BE/B9-B10
3/4" = 1'-0"

- NOTES:**
1. WALL HORIZONTAL, VERTICAL, AND CORNER REINFORCING NOT SHOWN FOR CLARITY. SEE DETAILS.
 2. REFER TO FOUNDATION PLANS FOR LOCATIONS AND DIMENSIONS.
 3. REFER TO FOUNDATION PLAN AND SECTIONS FOR TOP OF PIER/PILASTER ELEVATIONS.
 4. ALL VERTICAL REINFORCING BARS SHALL RECEIVE A 90-DEGREE STANDARD HOOK AT THE BOTTOM, INTO THE FOUNDATION.
 5. ORIENT CROSS-TIES ON EITHER SIDE OF VERTICAL REINF. TO CLEAR COLUMN ANCHORS. WHERE TIES INTERFERE WITH ANCHORS ON EITHER SIDE OF VERTICALS, ADJUST VERTICALS TO CLEAR ANCHORS. SUBMIT DETAILED VIEWS IN SHOP DRAWINGS.

KEYNOTE LEGEND:

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



DRAWING NAME:

BASE PLATE AND PIER DETAILS - 2

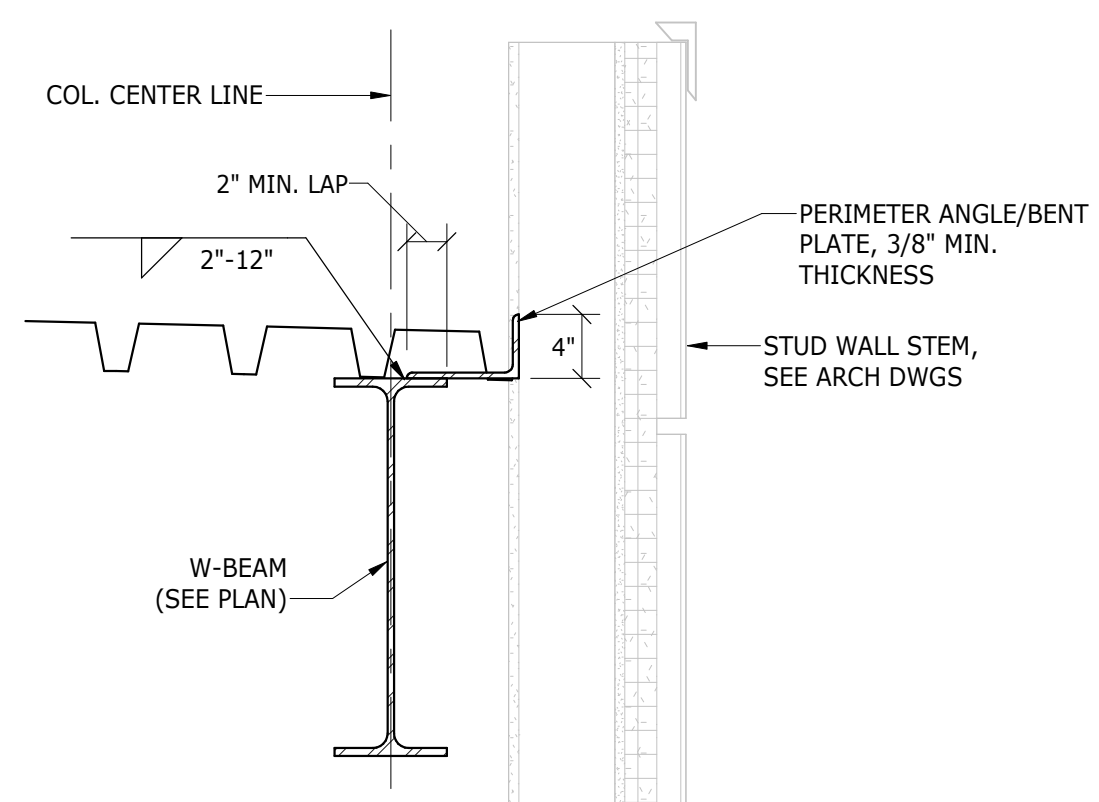
DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:
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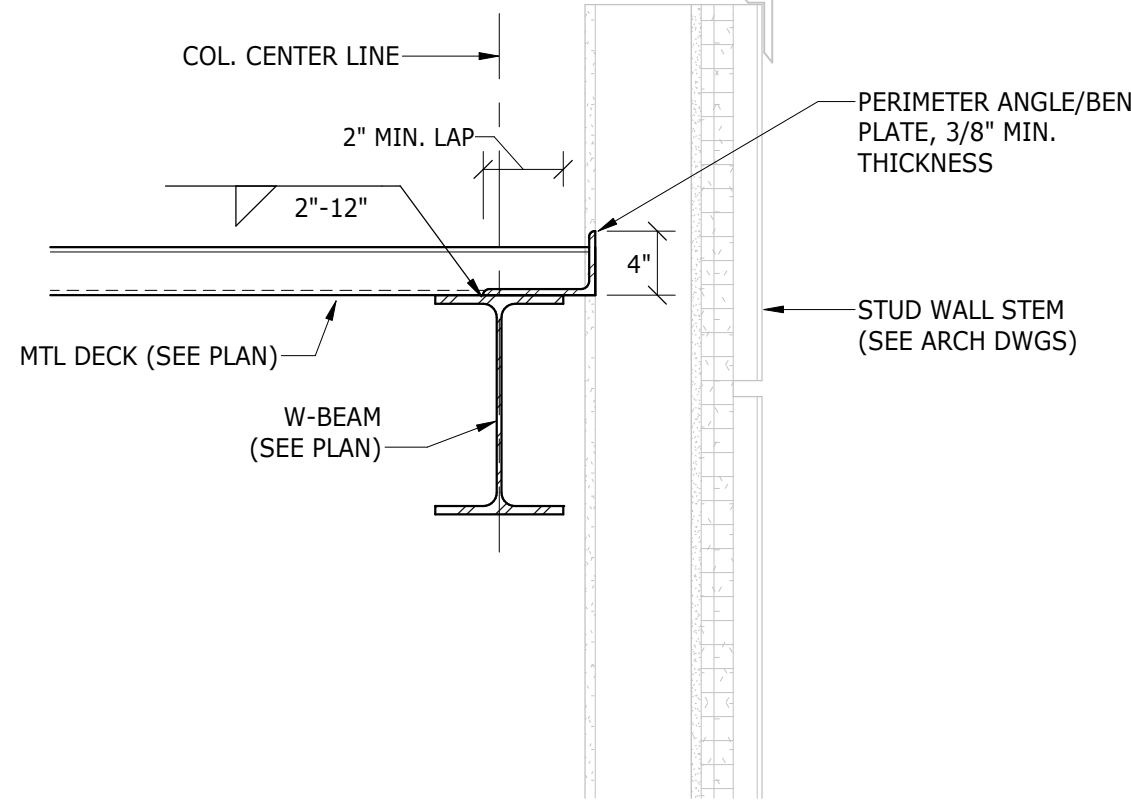


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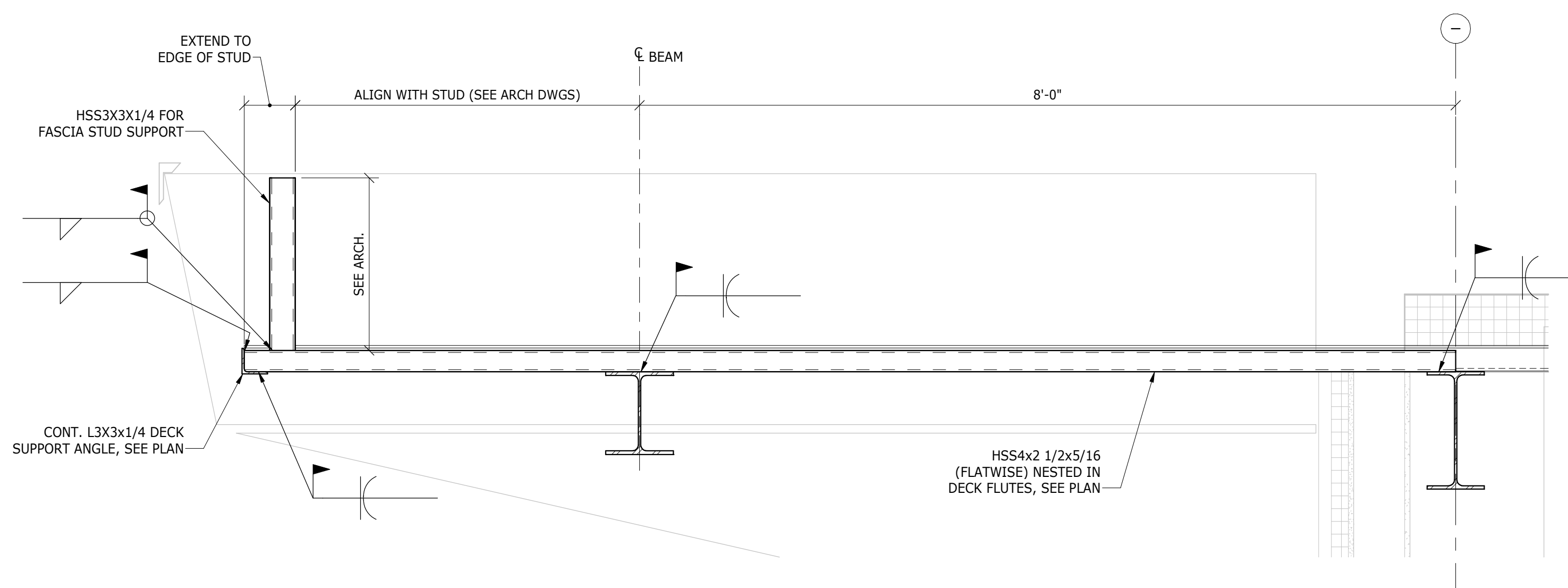


DECK PARALLEL TO BEAM

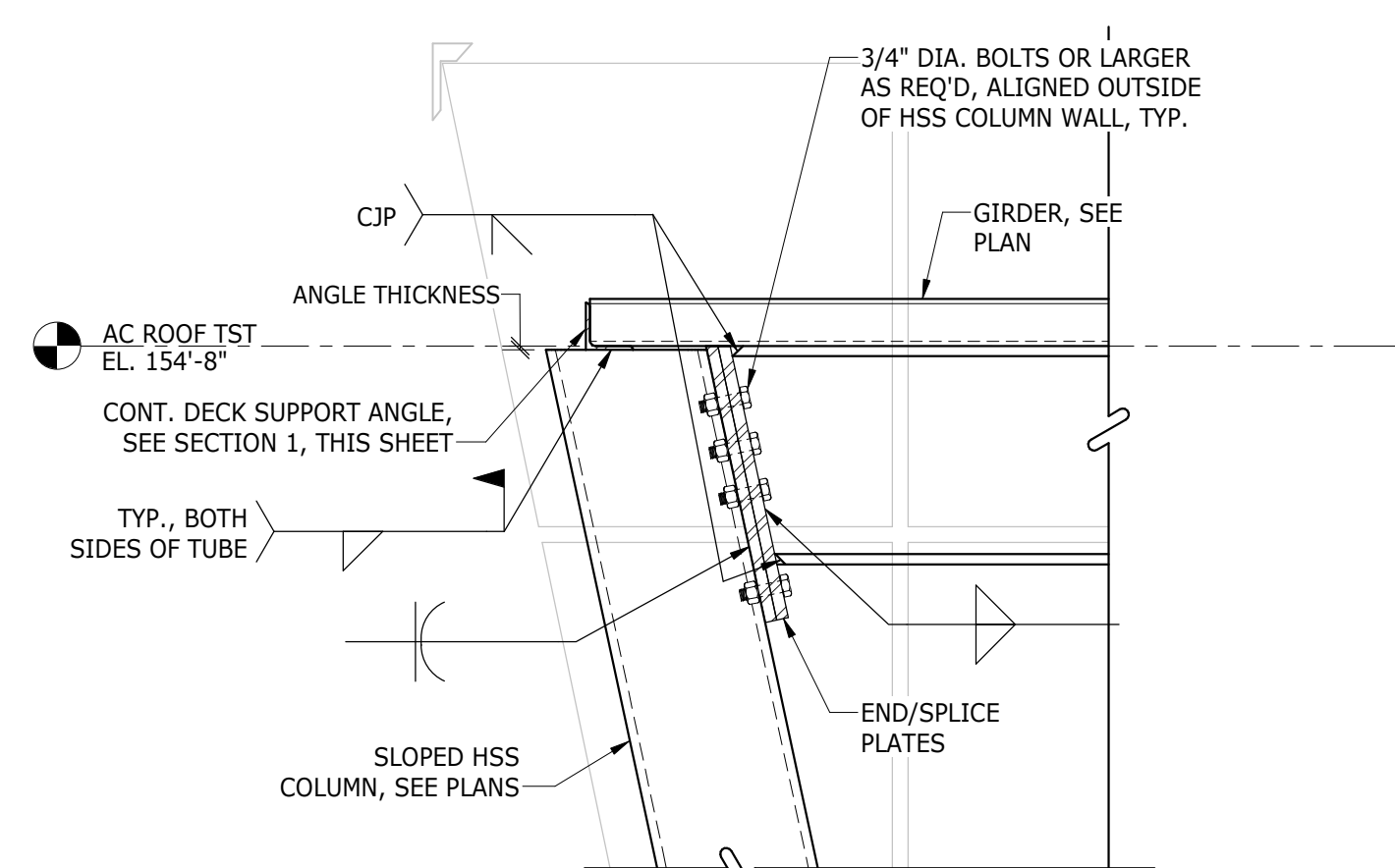


DECK PERPENDICULAR TO BEAM

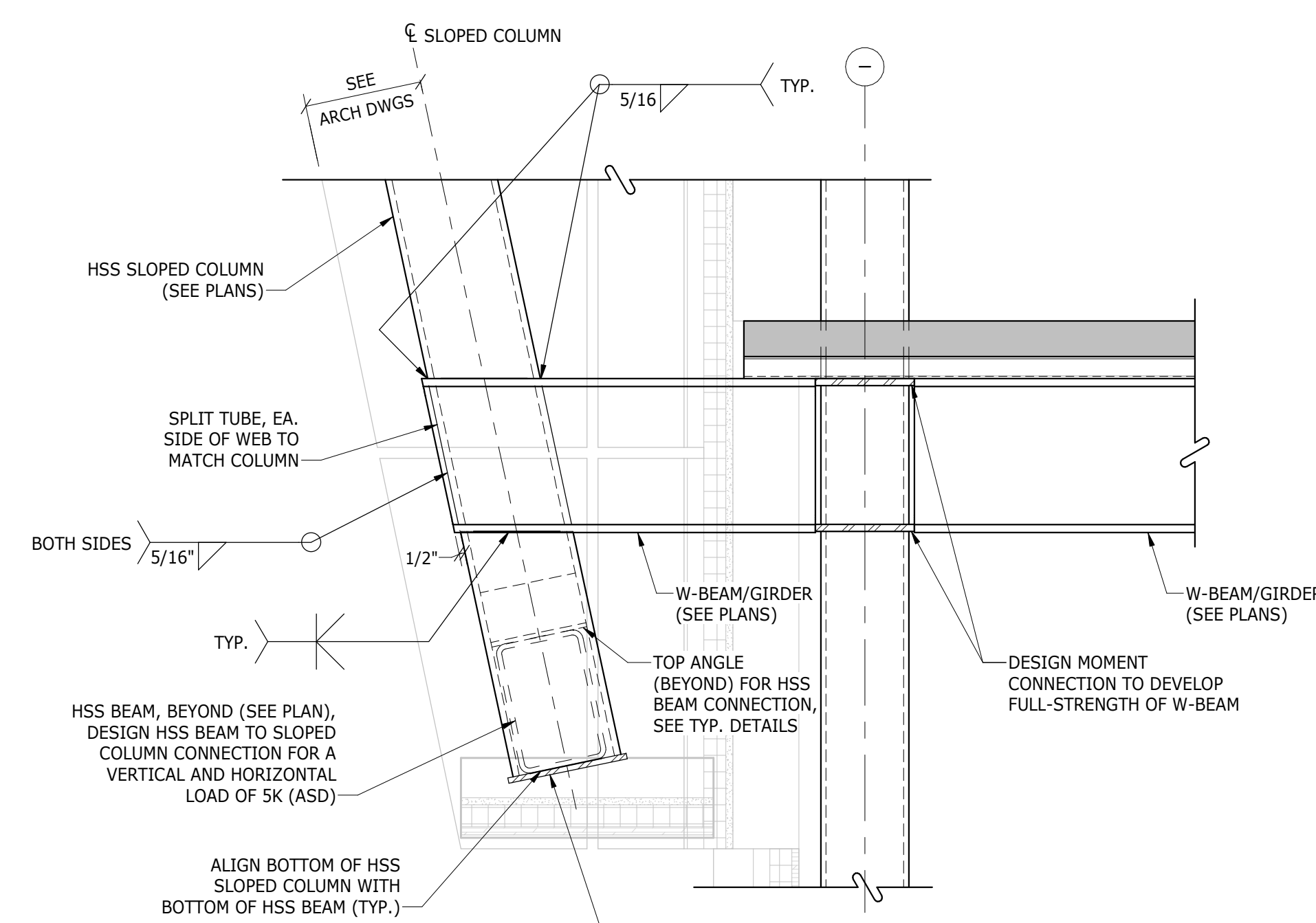
TYPICAL EDGE OF ROOF DECK DETAILS
NOT TO SCALE



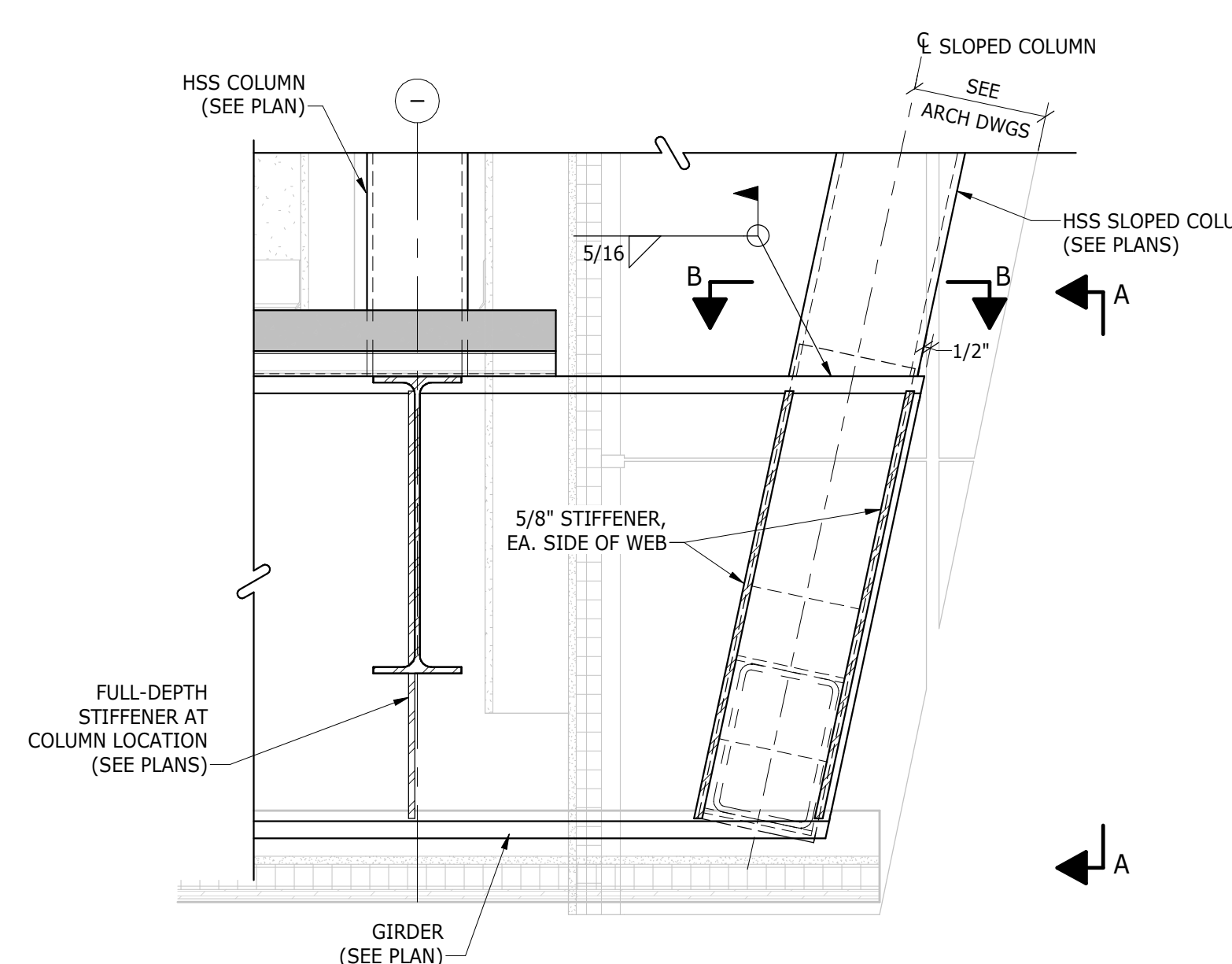
SECTION 1
1" = 1'-0"



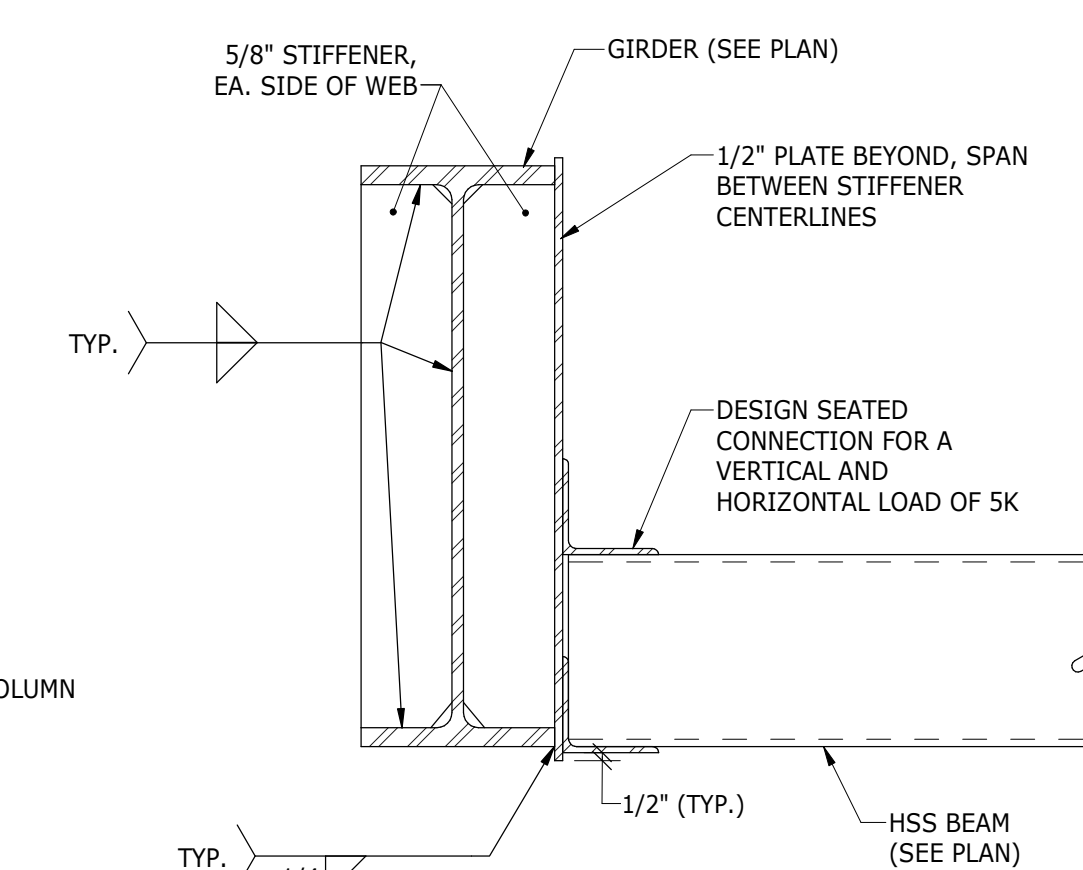
SECTION 2
1" = 1'-0"



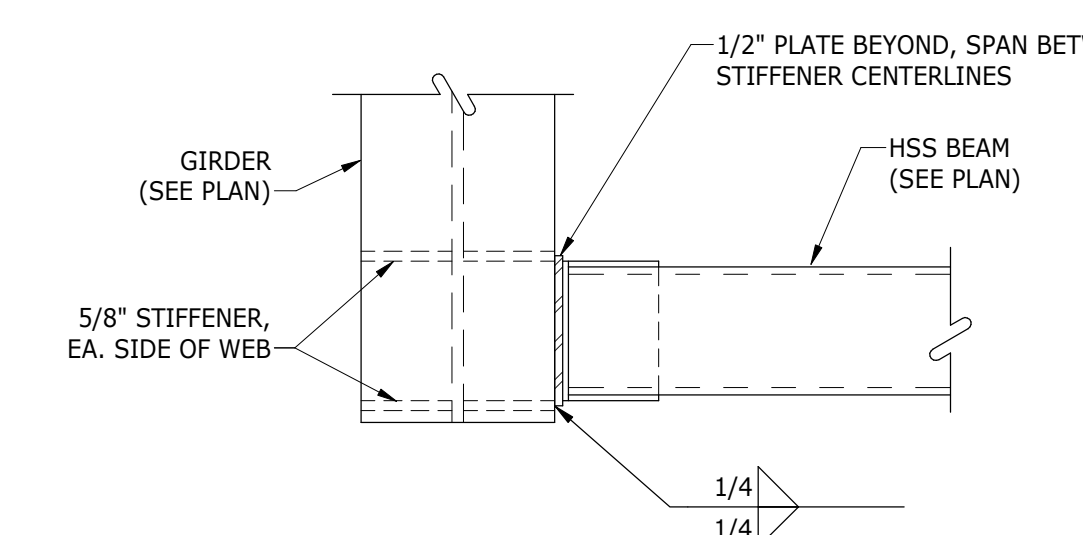
SECTION 3
1" = 1'-0"



SECTION 4
1" = 1'-0"



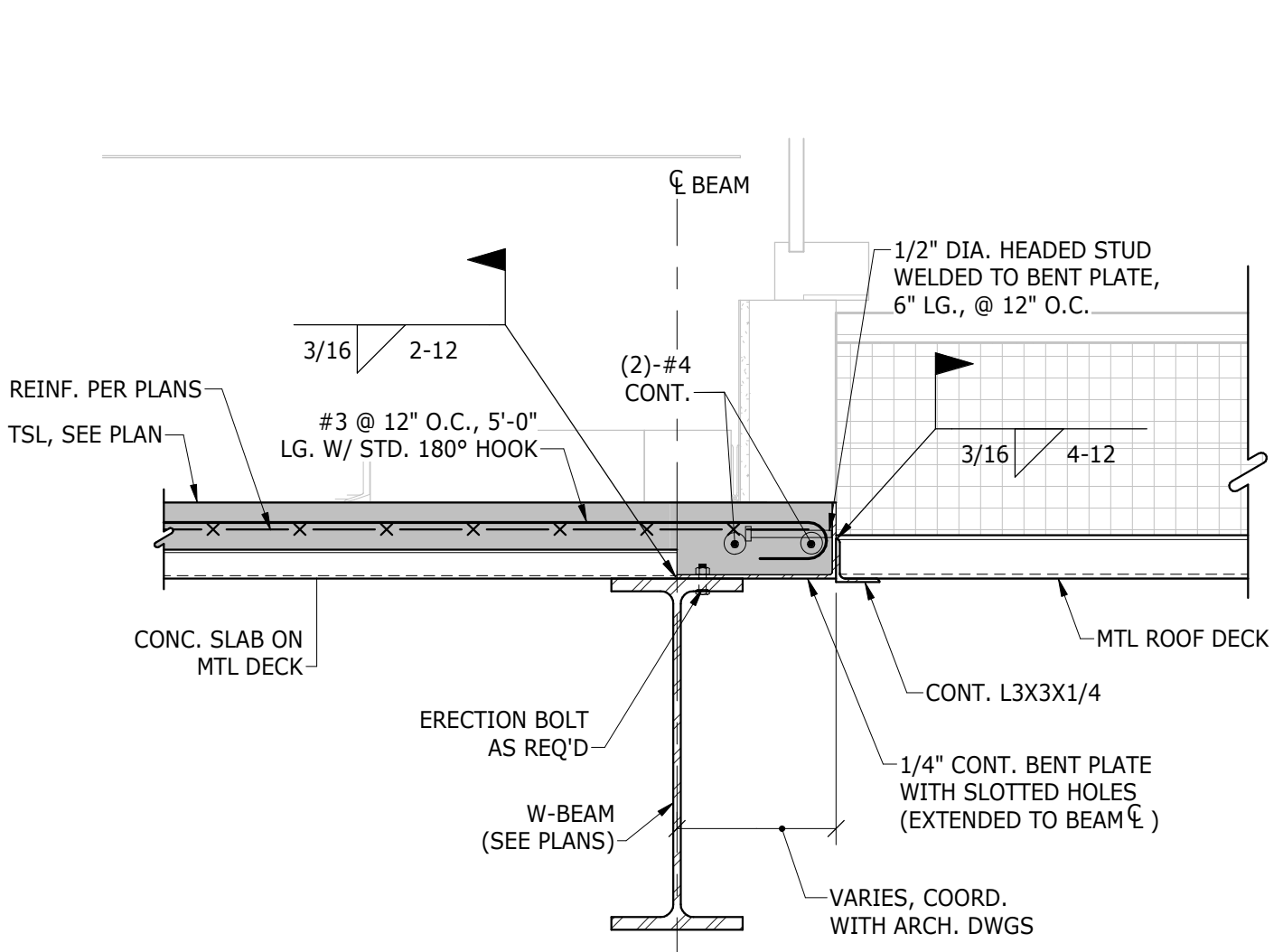
SECTION A-A



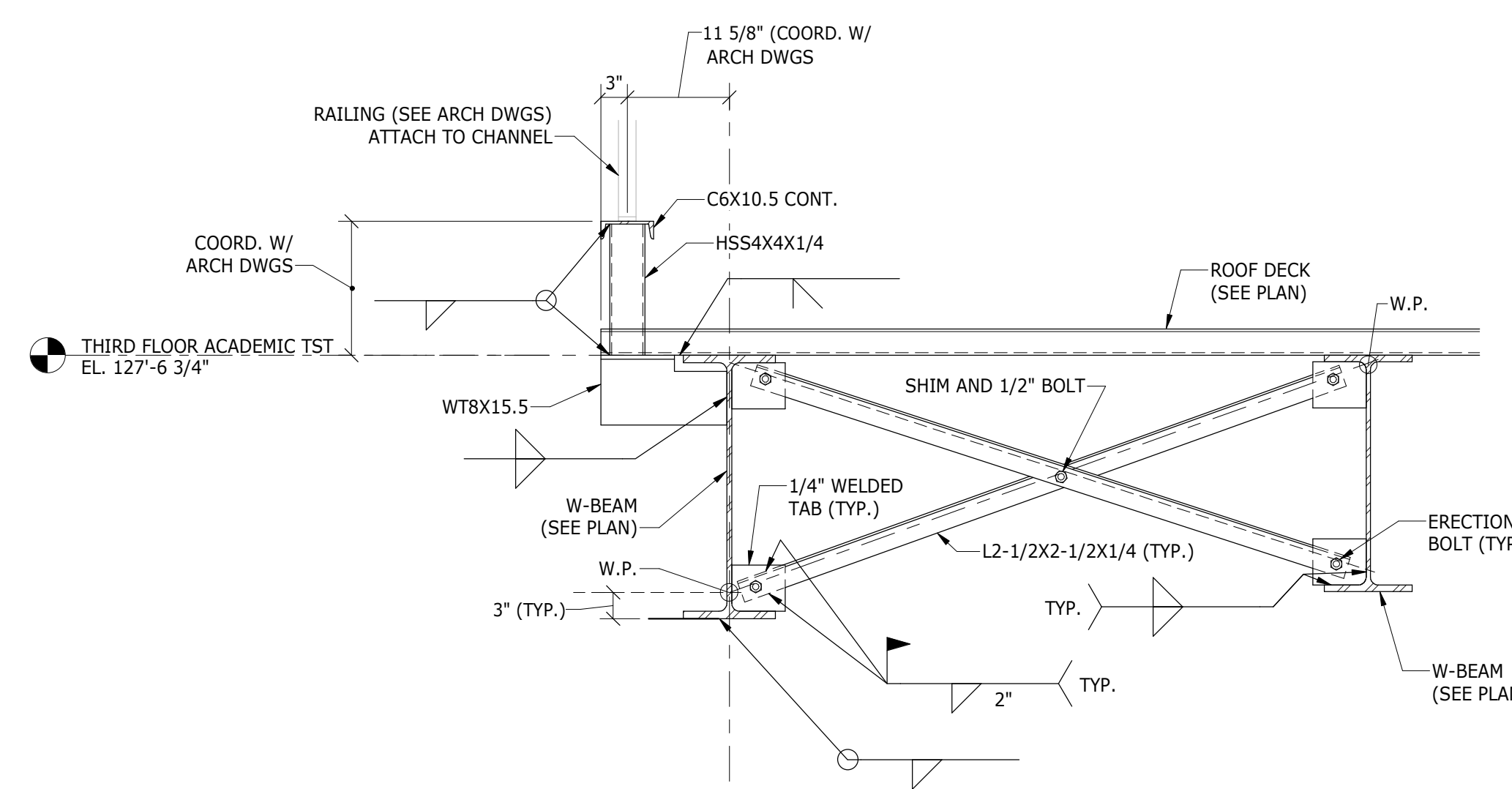
SECTION B-B

NOTE:
DESIGN CONNECTION FOR A VERTICAL LOAD OF 20K, A HORIZONTAL SHEAR FORCE OF 10K, AND A TENSION LOAD OF 7K (ASD). HORIZONTAL SHEAR FORCE SHALL BE APPLIED AT SLOPED COLUMN CENTERLINE AND CONNECTION DESIGN SHOULD ACCOUNT FOR TORSION DUE TO THIS ECCENTRICITY.

1/2" MIN. CAP PLATE, UTILIZE AS BOTTOM SEAT CONNECTION SUPPORT FOR HSS BEAM BEYOND

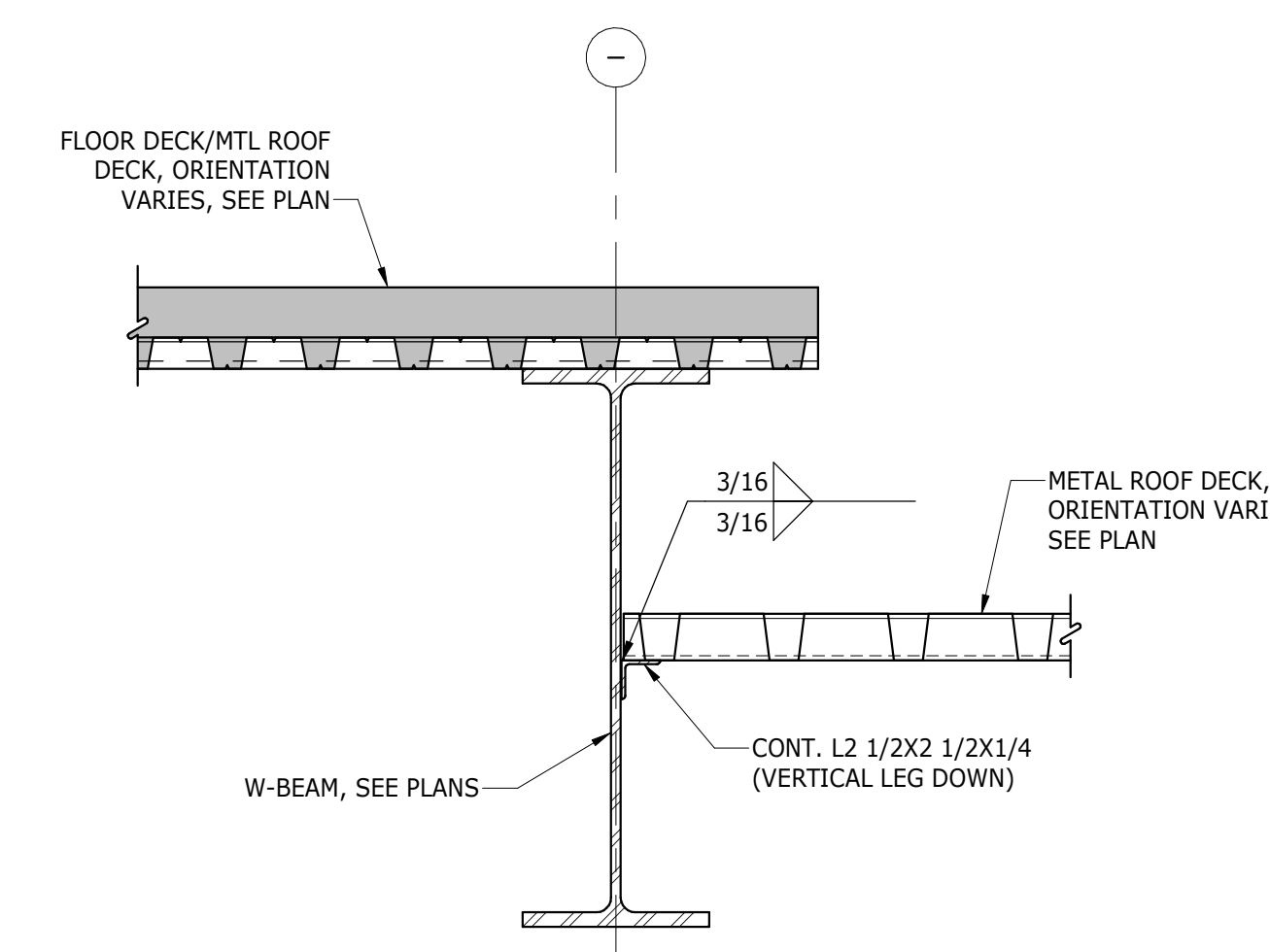


SECTION 5
1" = 1'-0"



SECTION 6
1/2" = 1'-0"

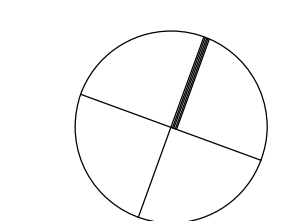
NOTE:
INENT IS TO CONNECT BOX HEADERS TO WF WEBS.



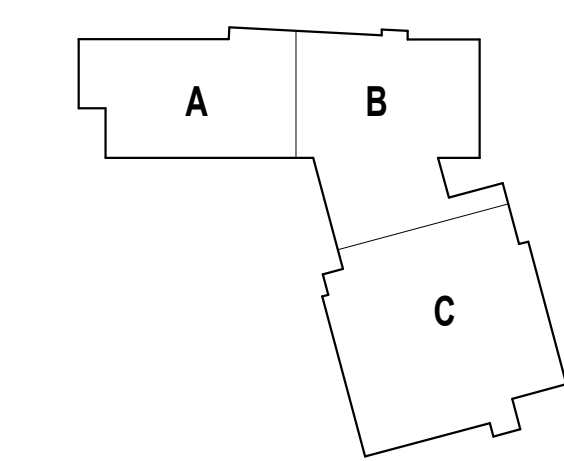
SECTION 7
1" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

STEEL SECTIONS AND DETAILS - 1

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

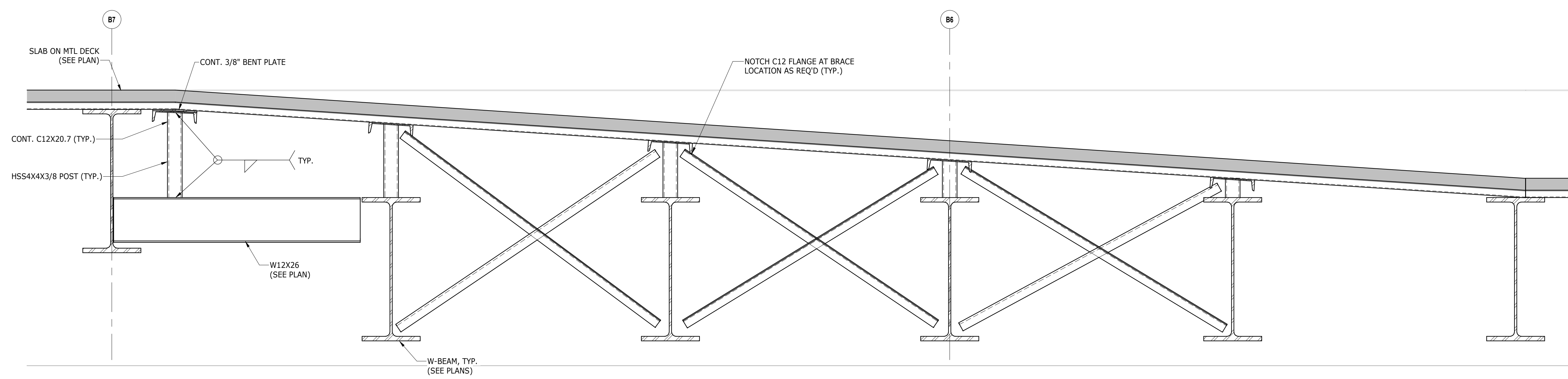
S3.10



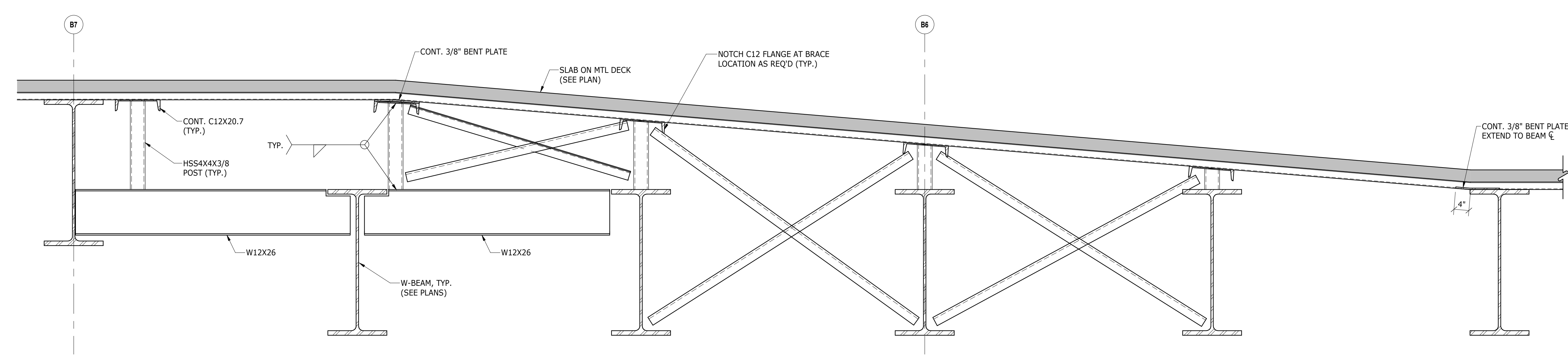
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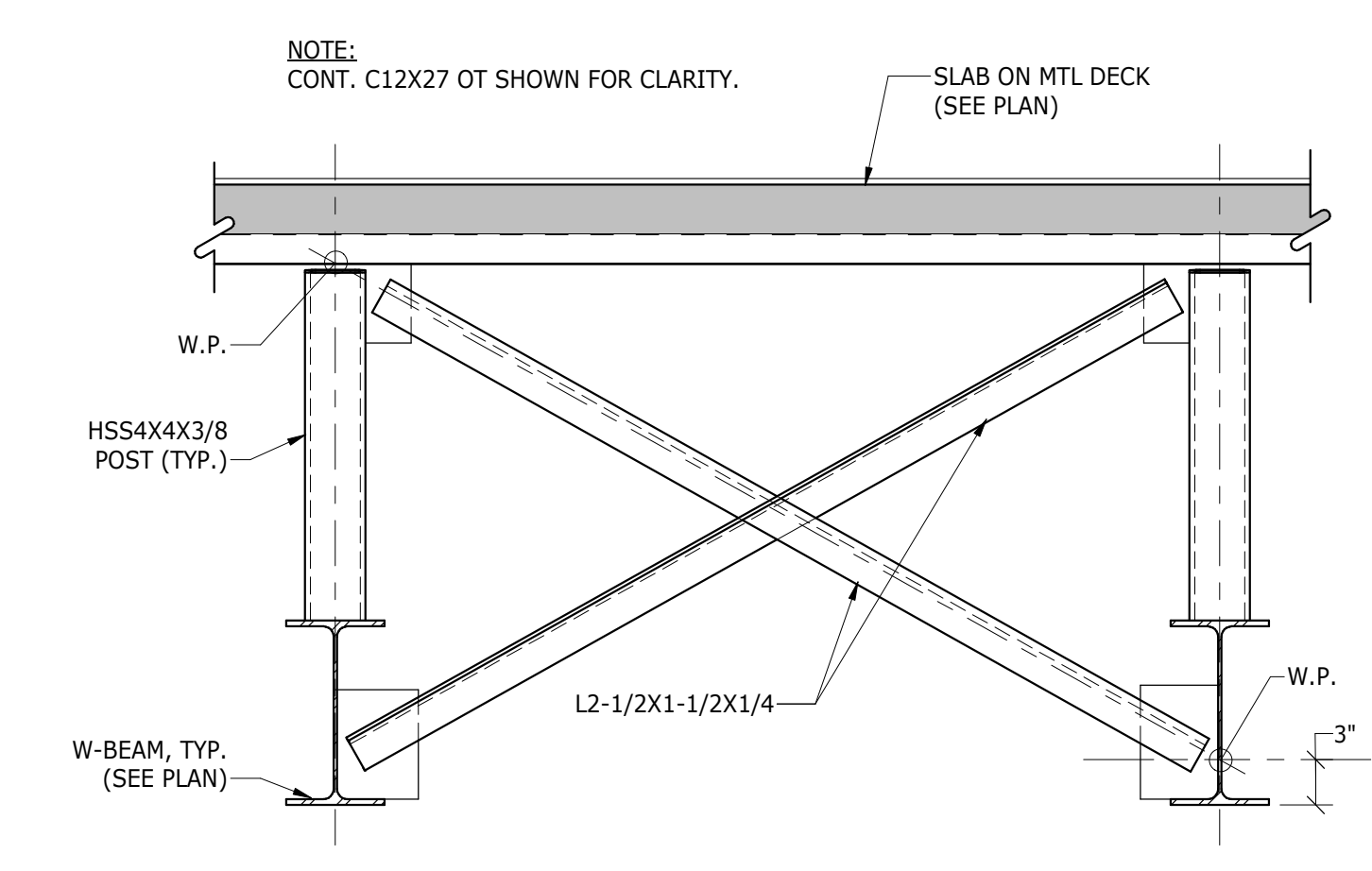
KEYNOTE LEGEND:



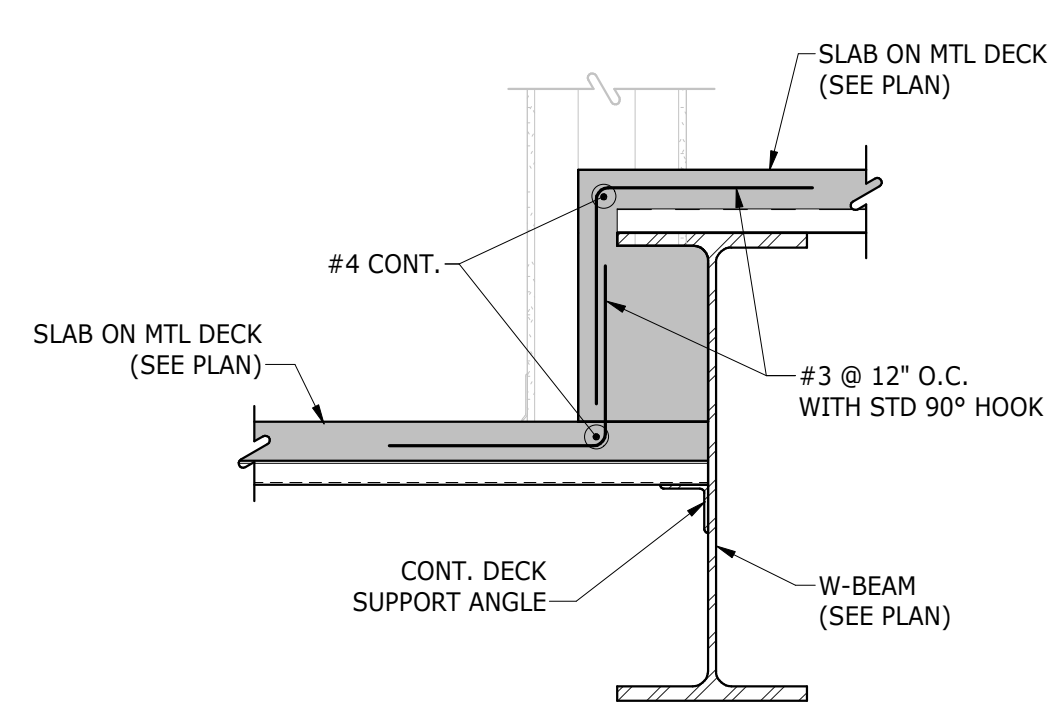
1 SECTION
3/4" = 1'-0"



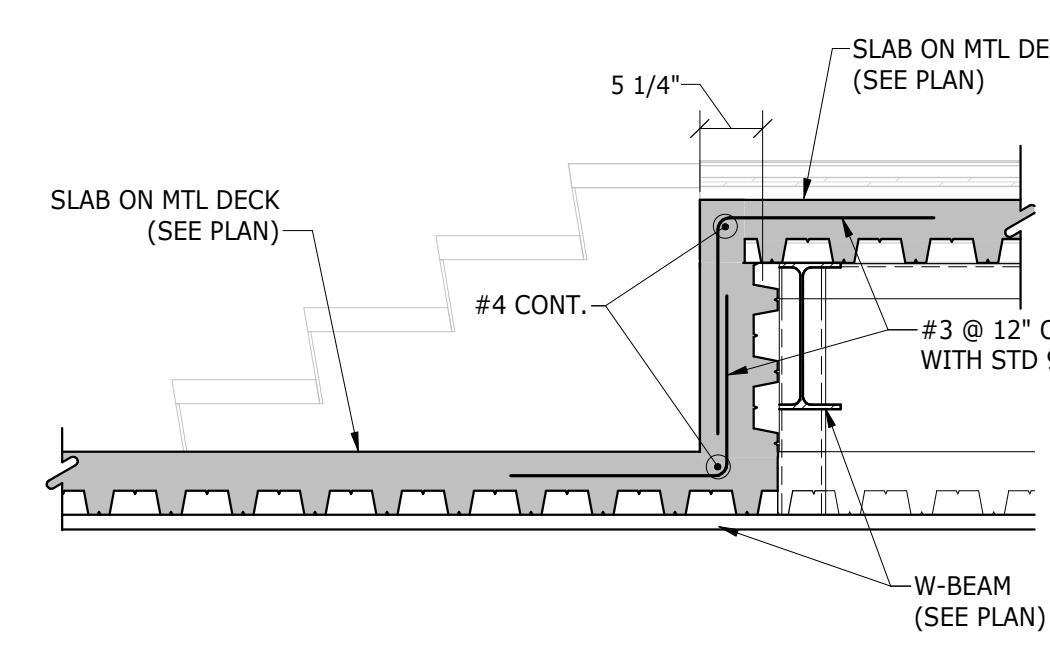
2 SECTION
3/4" = 1'-0"



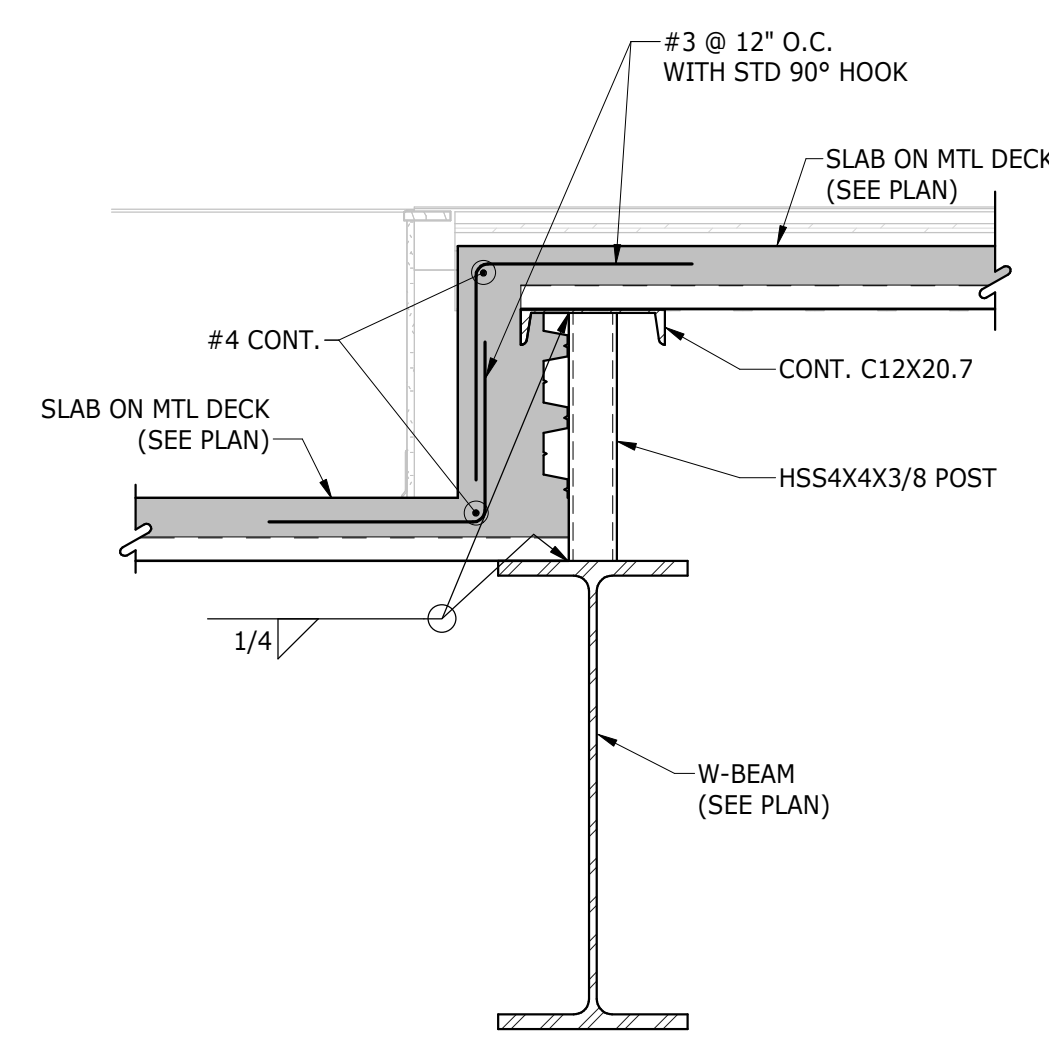
3 TYPICAL CROSS BRACING SECTION
1" = 1'-0"



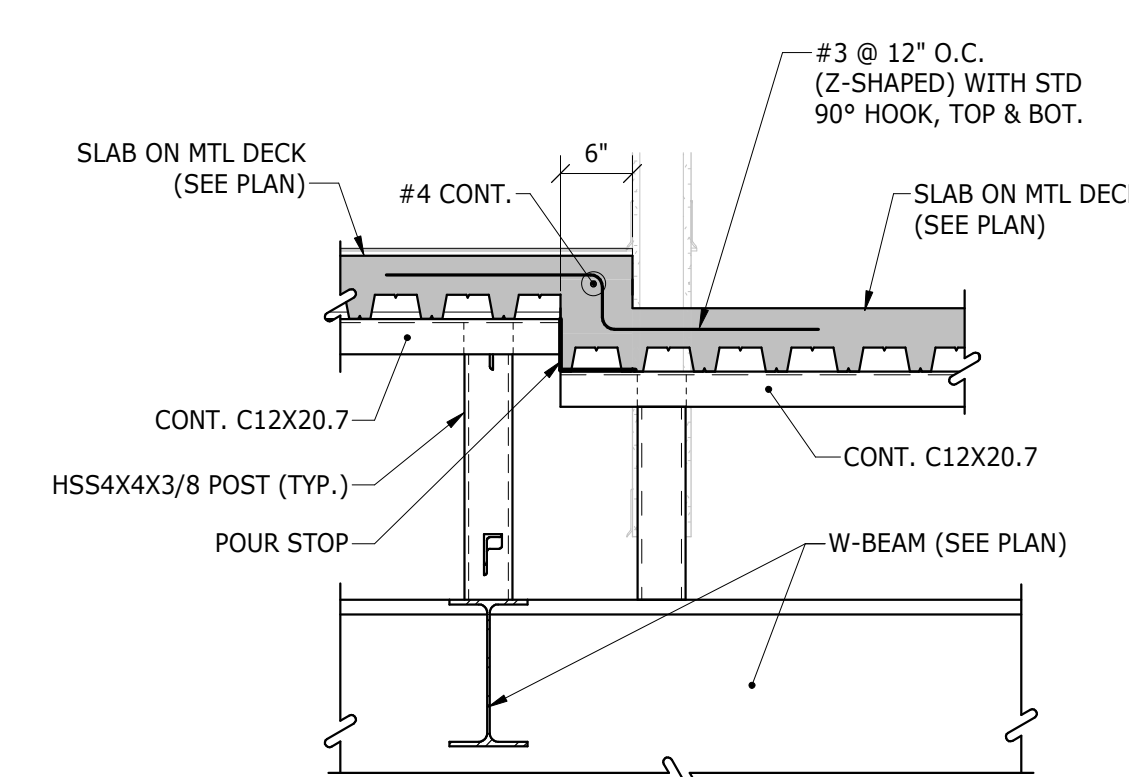
4 SECTION
3/4" = 1'-0"



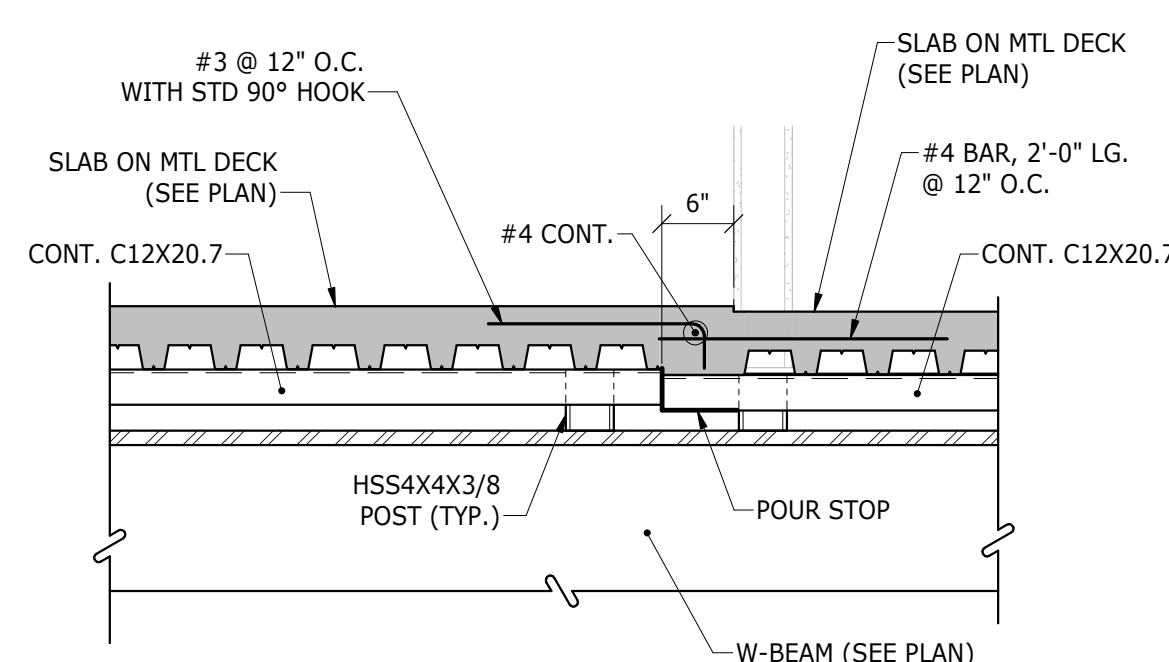
5 SECTION
3/4" = 1'-0"



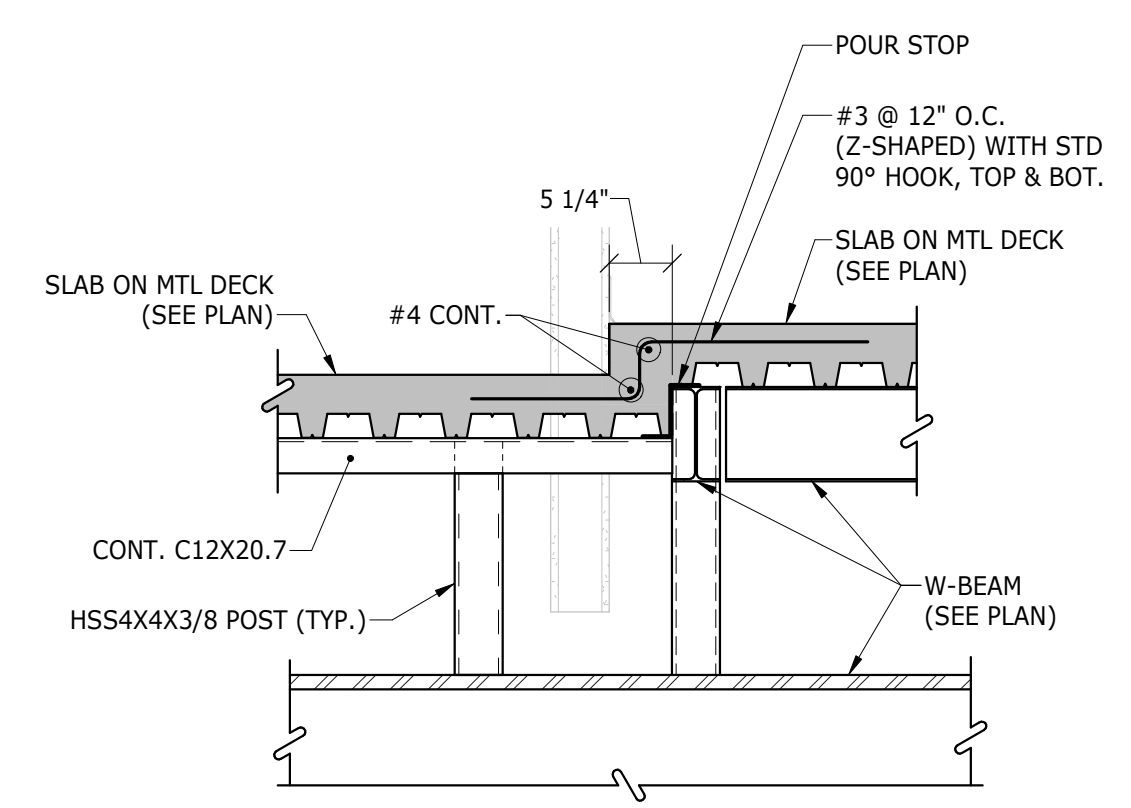
6 SECTION
3/4" = 1'-0"



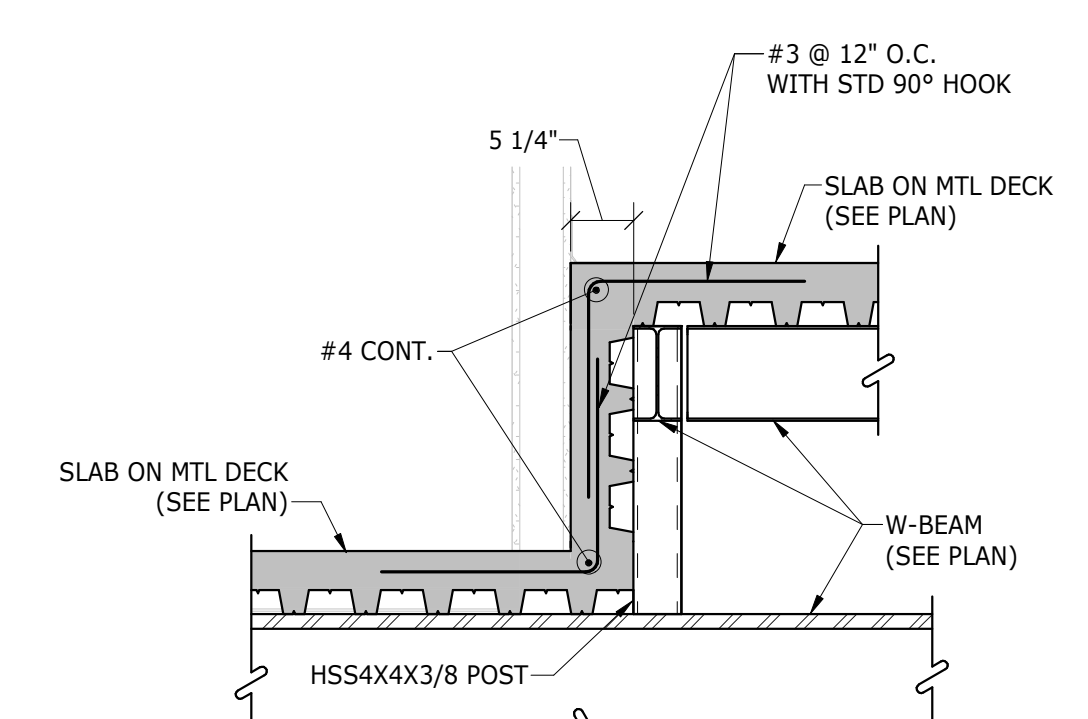
7 SECTION
3/4" = 1'-0"



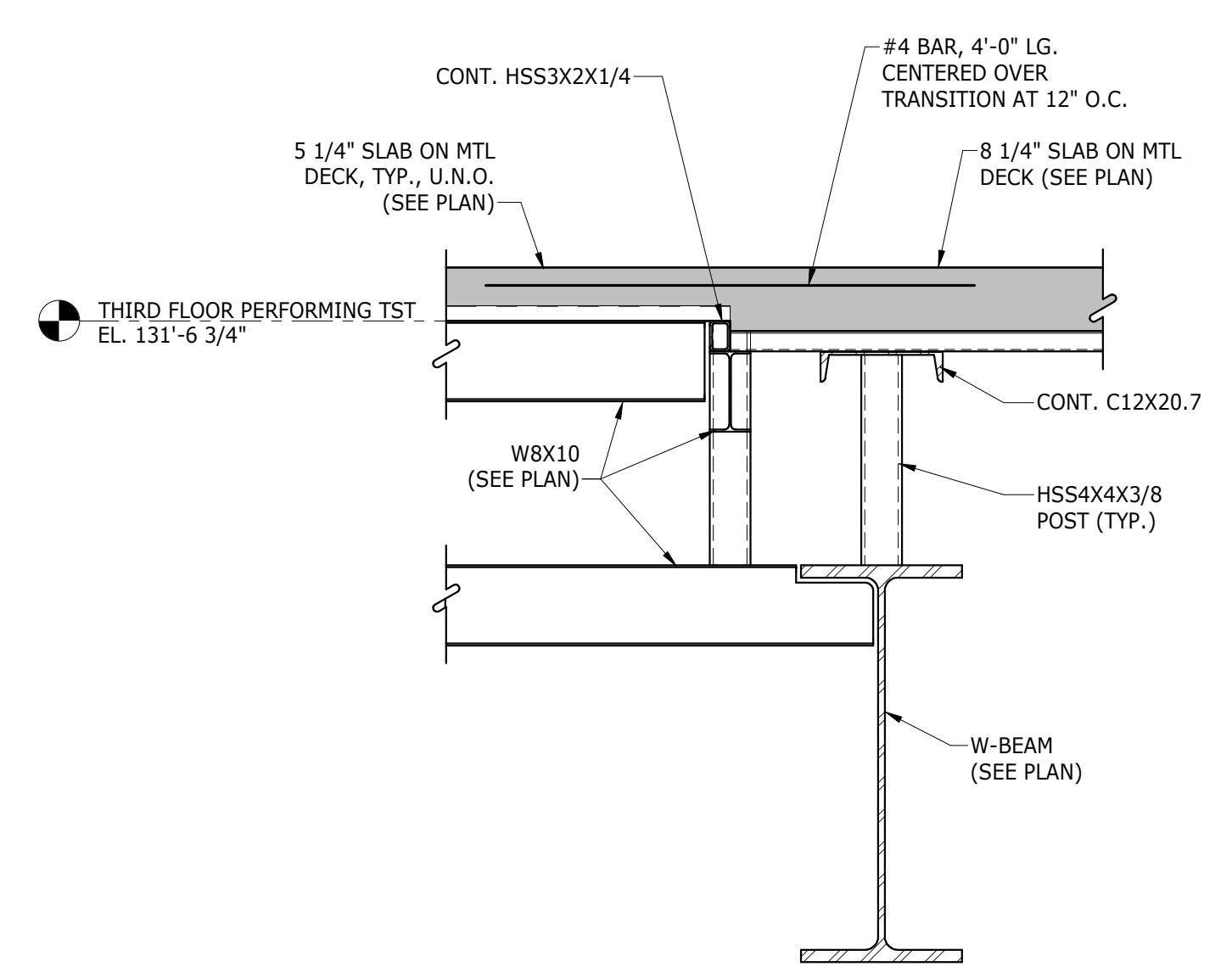
8 SECTION
3/4" = 1'-0"



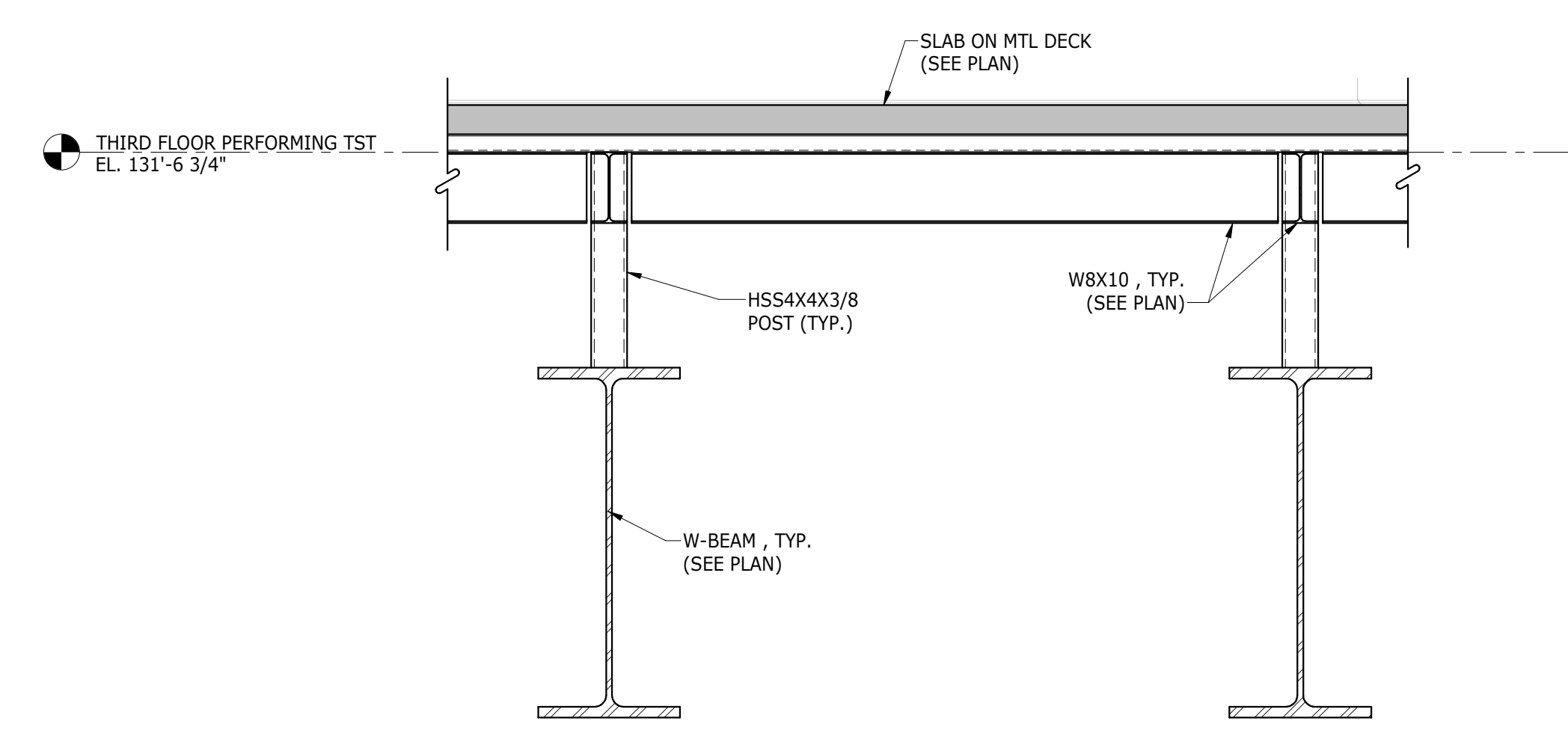
9 SECTION
3/4" = 1'-0"



10 SECTION
3/4" = 1'-0"



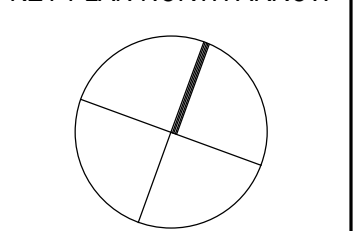
11 SECTION
3/4" = 1'-0"



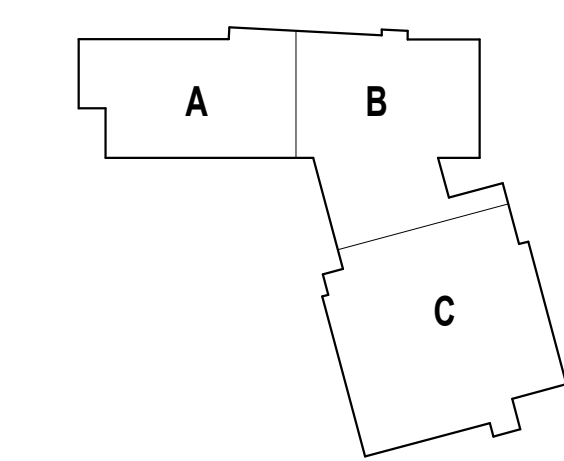
12 SECTION
3/4" = 1'-0"

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

STEEL SECTIONS AND DETAILS - 2

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER:

JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

S3.11

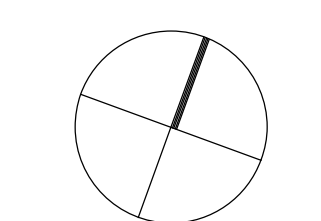


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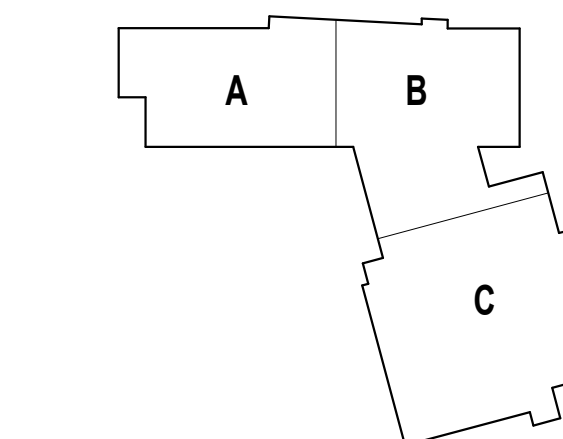
KEYNOTE LEGEND:

100% CONSTRUCTION DOCUMENTS

KEY PLAN NORTH ARROW



KEYPLAN



DRAWING NAME:

STEEL SECTIONS AND DETAILS - 3

DRAWN BY: JDB / MSS

REVIEWED BY: MGM / BP

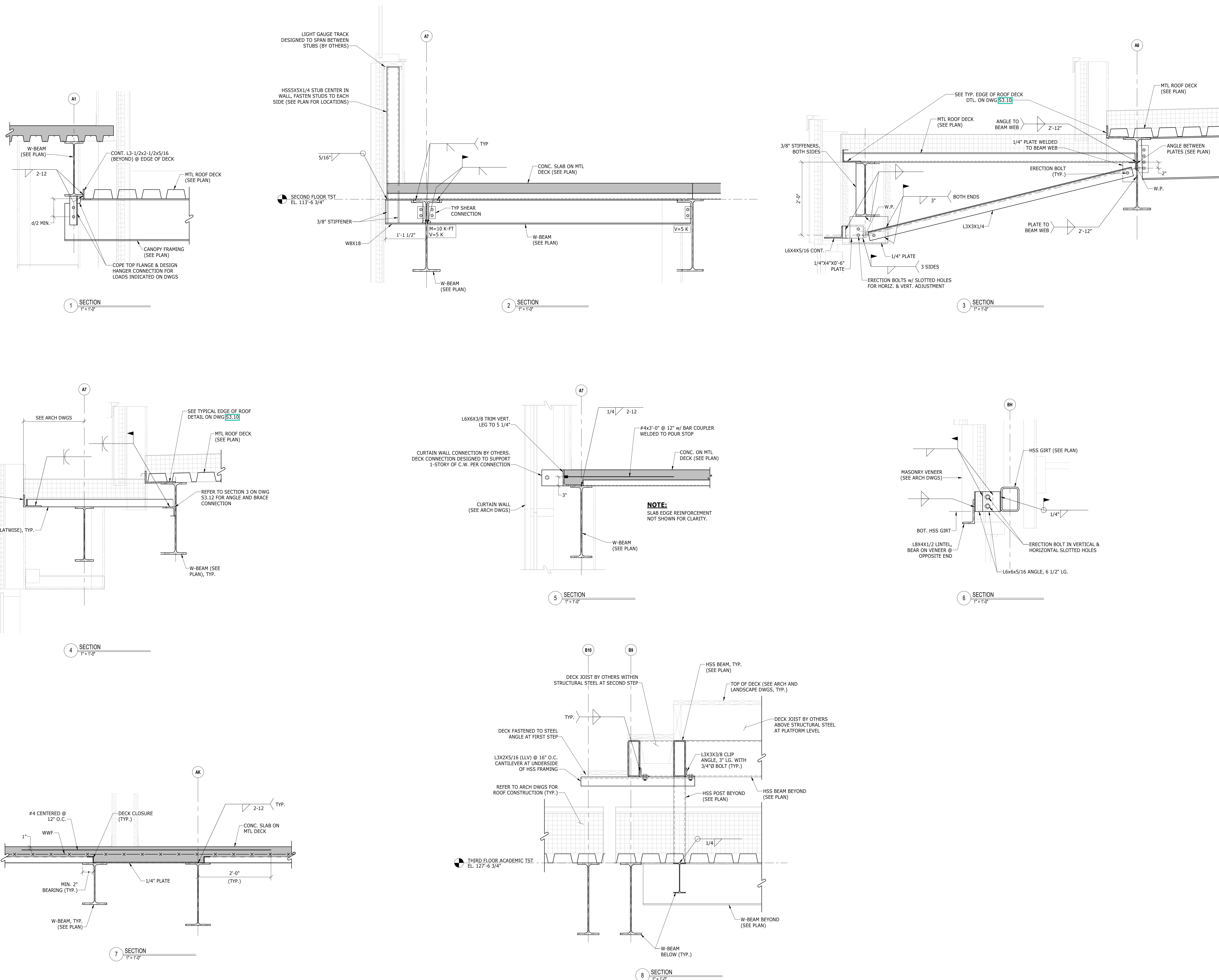
SCALE: AS INDICATED | DRAWING NUMBER:

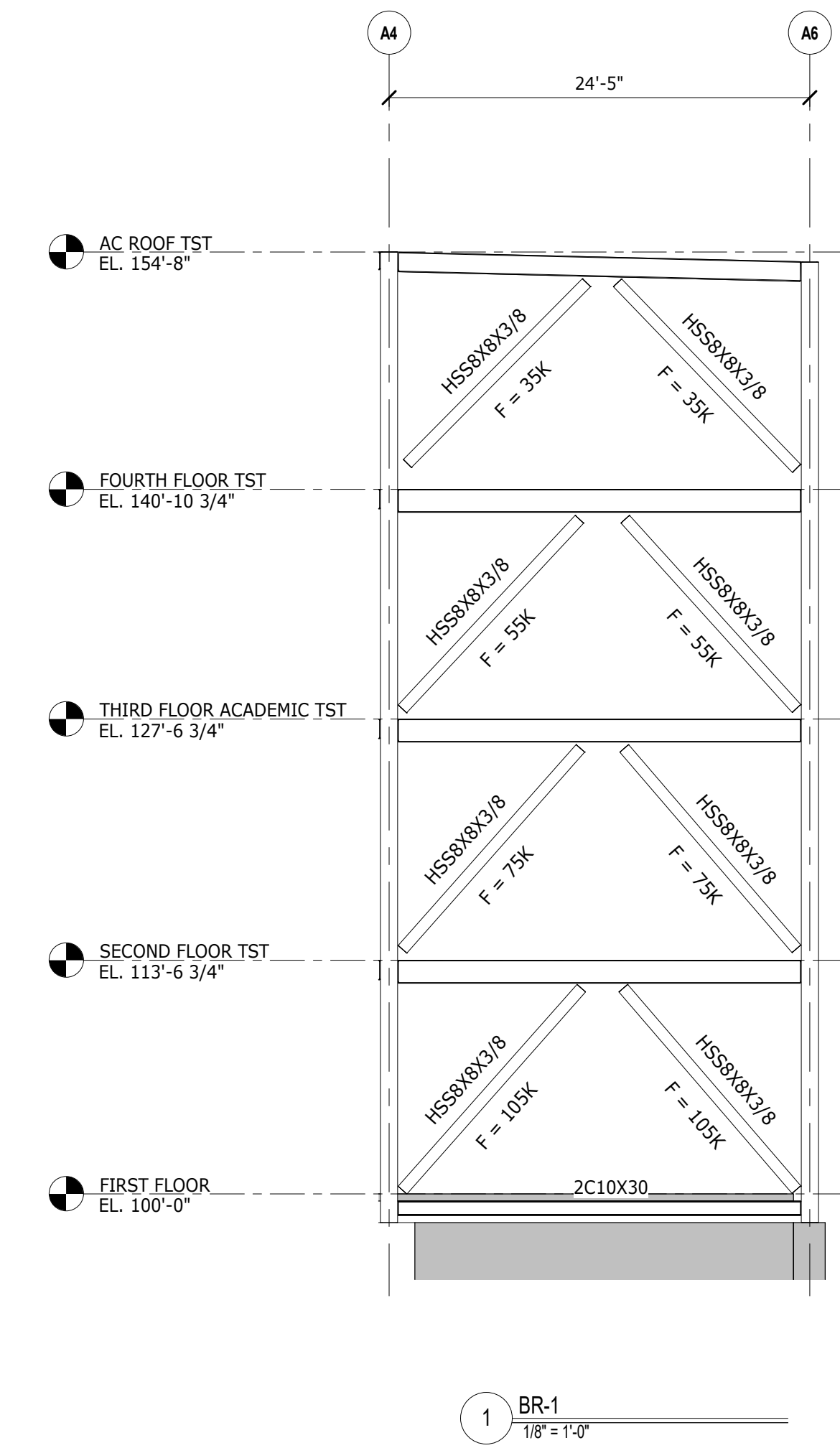
JOB NO.: 2202.02

DATE: OCTOBER 13, 2023

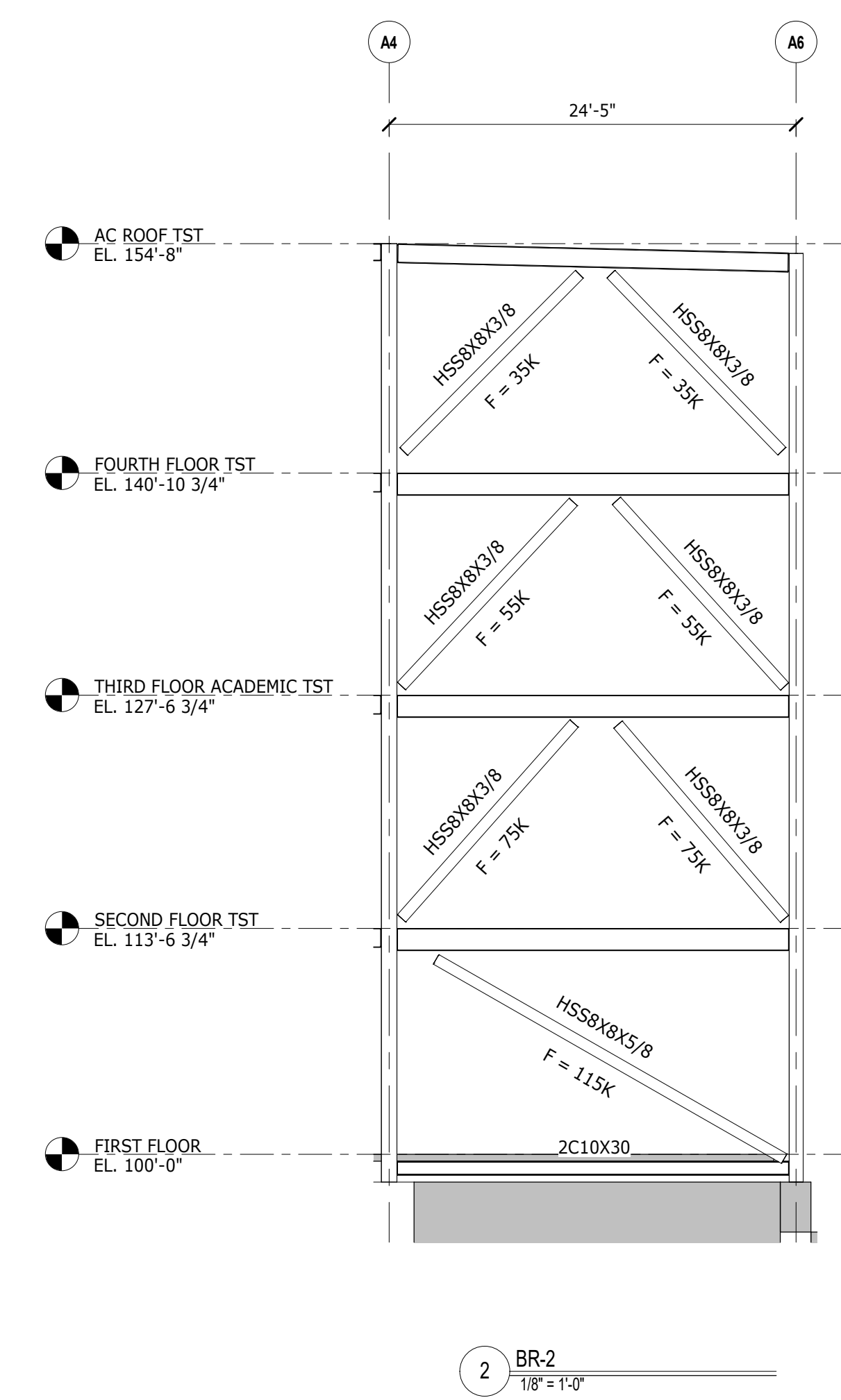
S3.12

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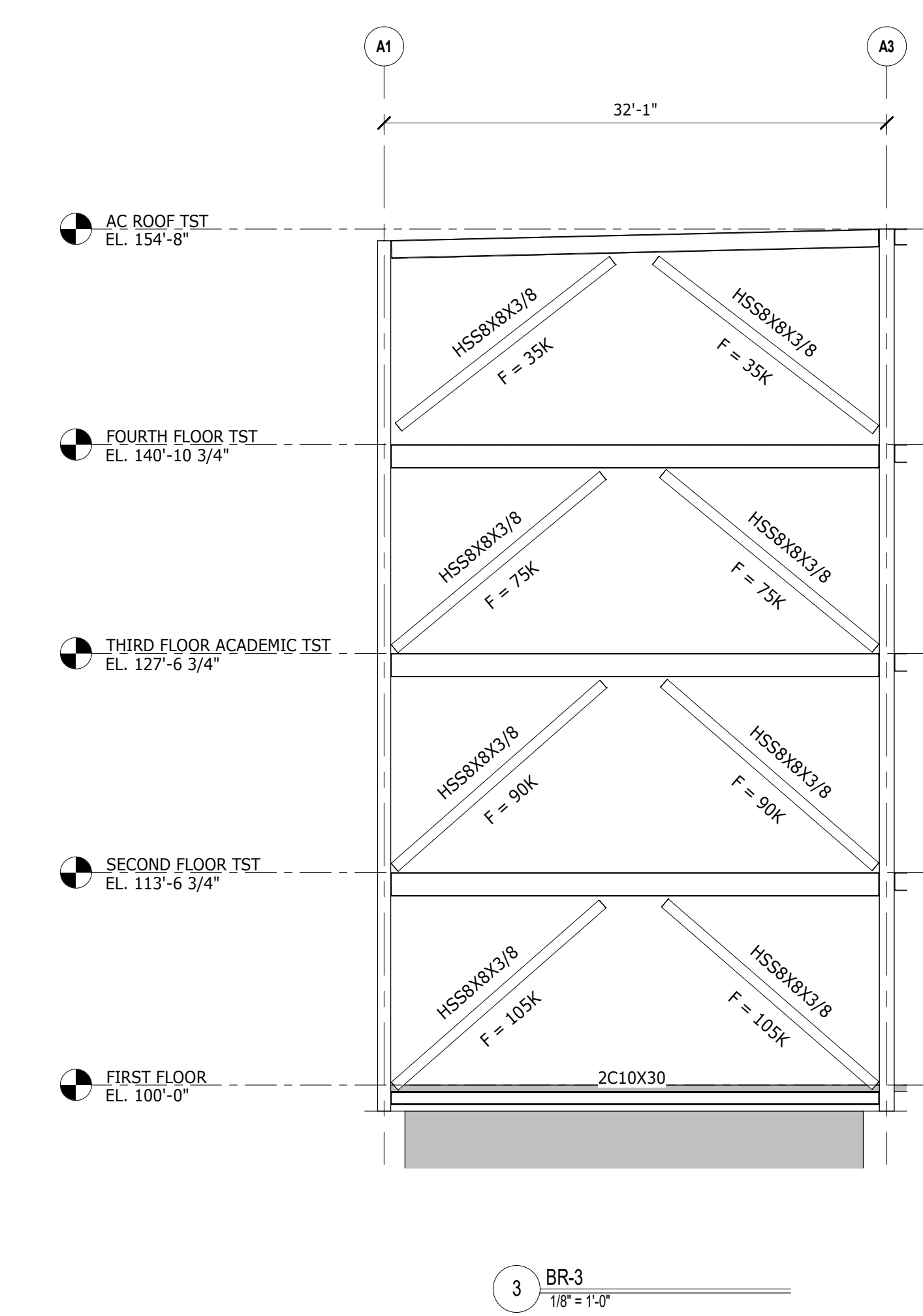




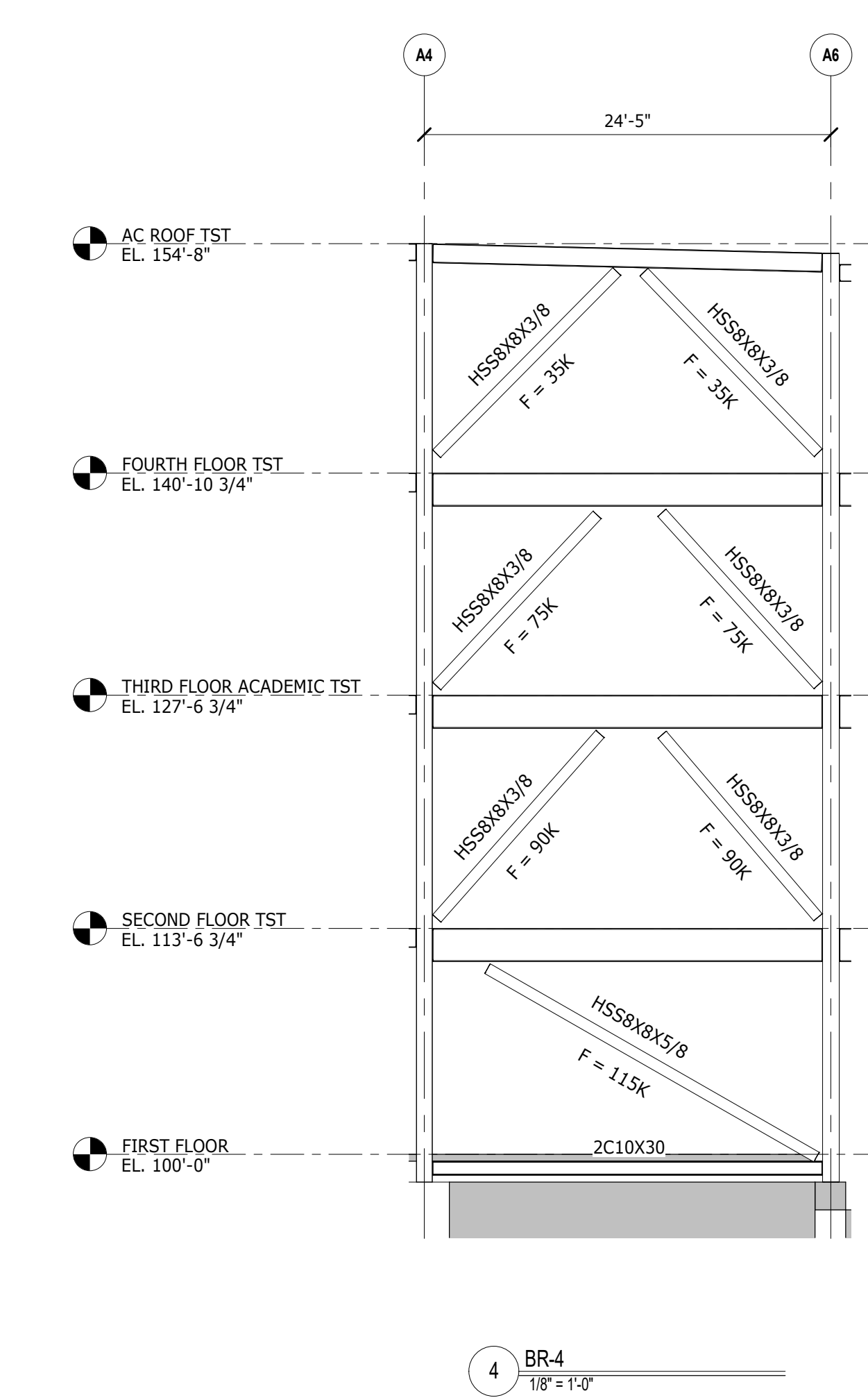
1 BR-1
18' x 1'-0"



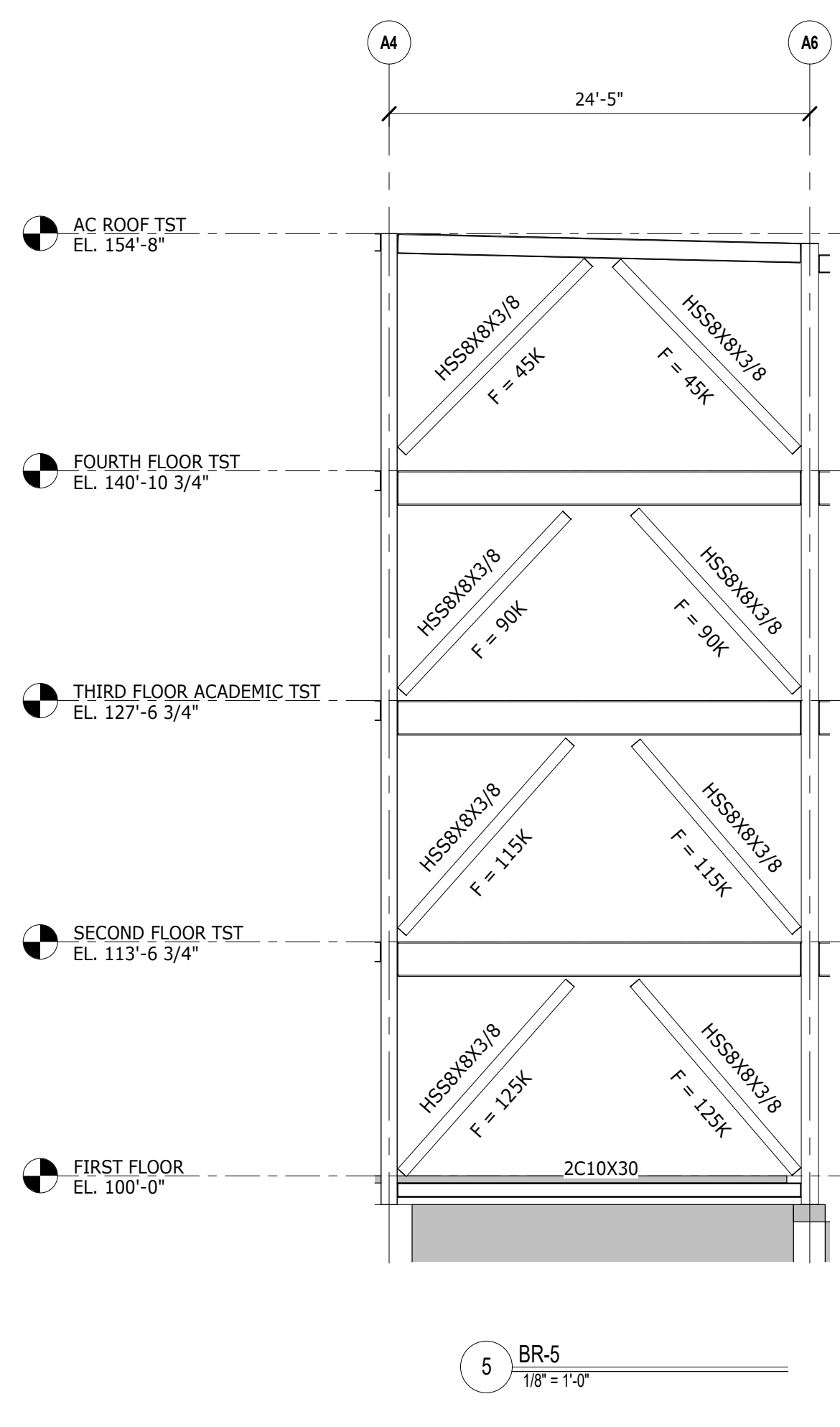
2 BR-2
18' x 1'-0"



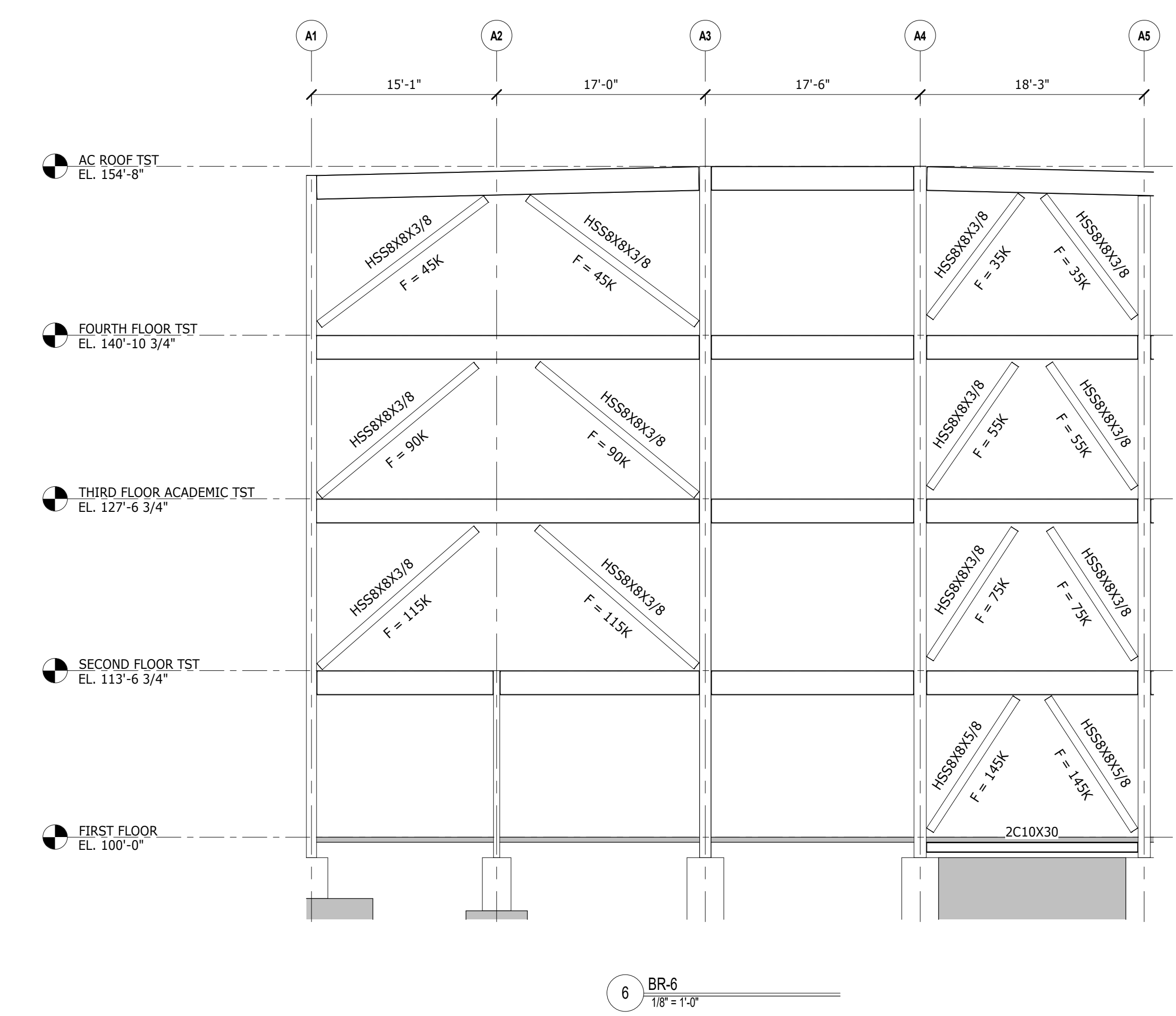
3 BR-3
18' x 1'-0"



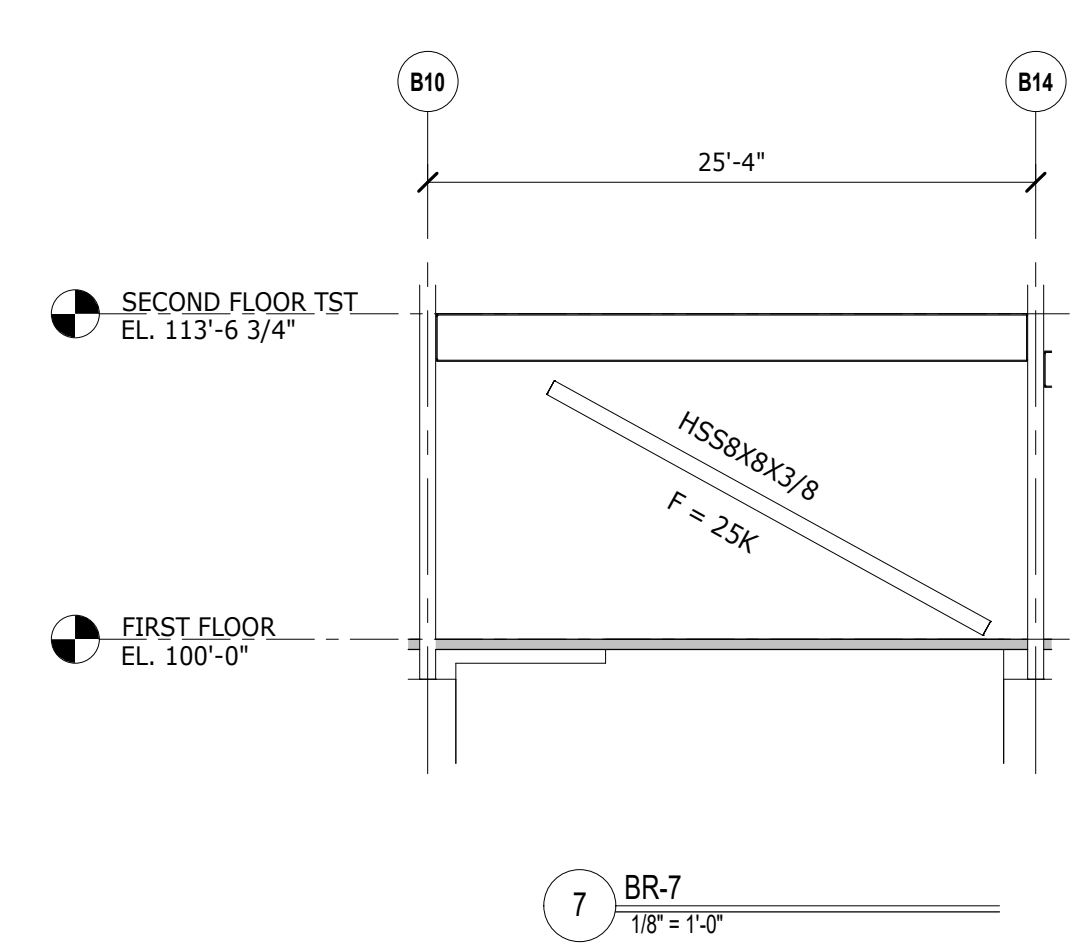
4 BR-4
18' x 1'-0"



5 BR-5
18' x 1'-0"



6 BR-6
18' x 1'-0"

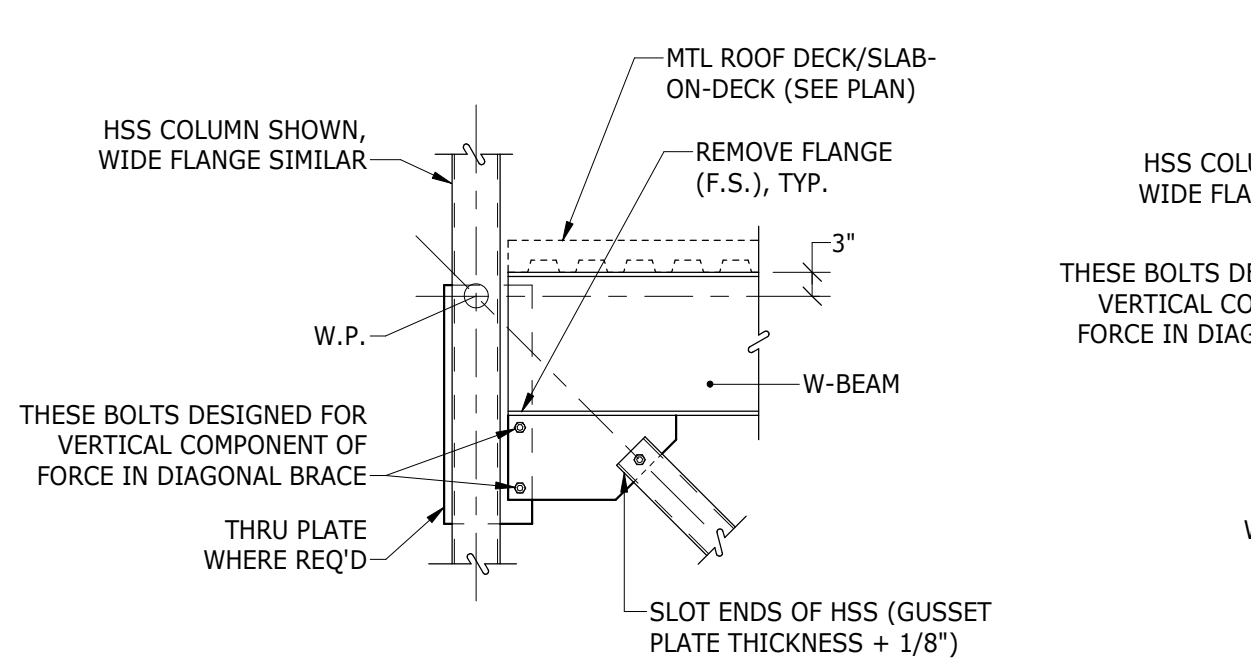
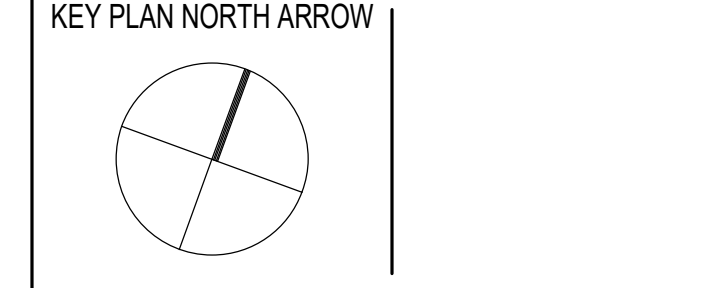


7 BR-7
18' x 1'-0"

BRACING NOTES:

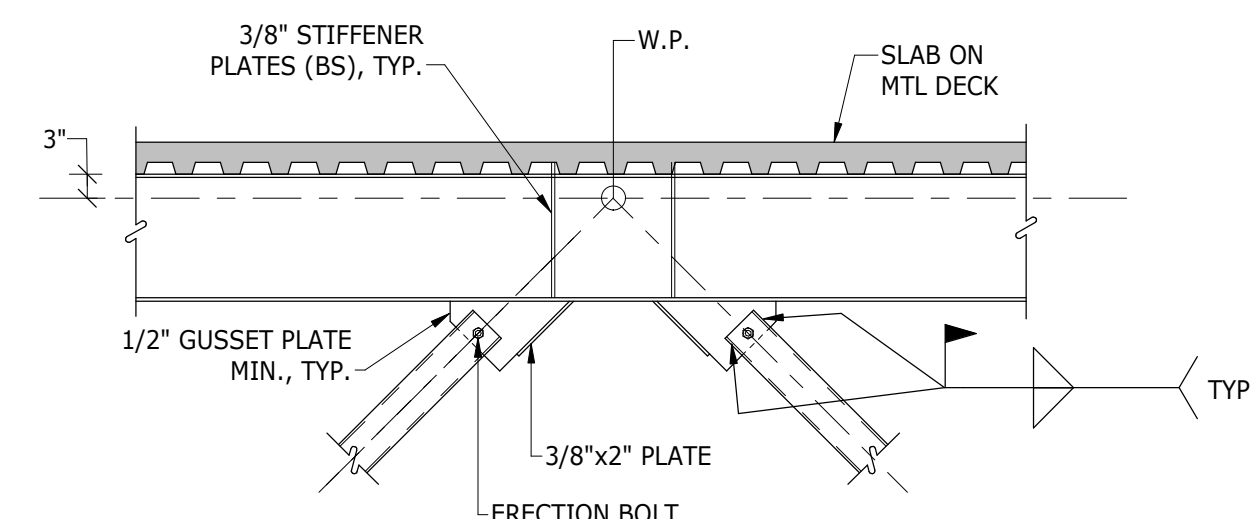
- BRACING ELEVATIONS ARE INTENDED AS SCHEMATIC REPRESENTATION ONLY. REFER TO PLANS, SECTIONS, AND DETAILS FOR FURTHER INFORMATION.
- FORCES SHOWN ON BRACING ELEVATIONS ARE FOR DESIGN OF CONNECTIONS (ALLOWABLE STRENGTH DESIGN).

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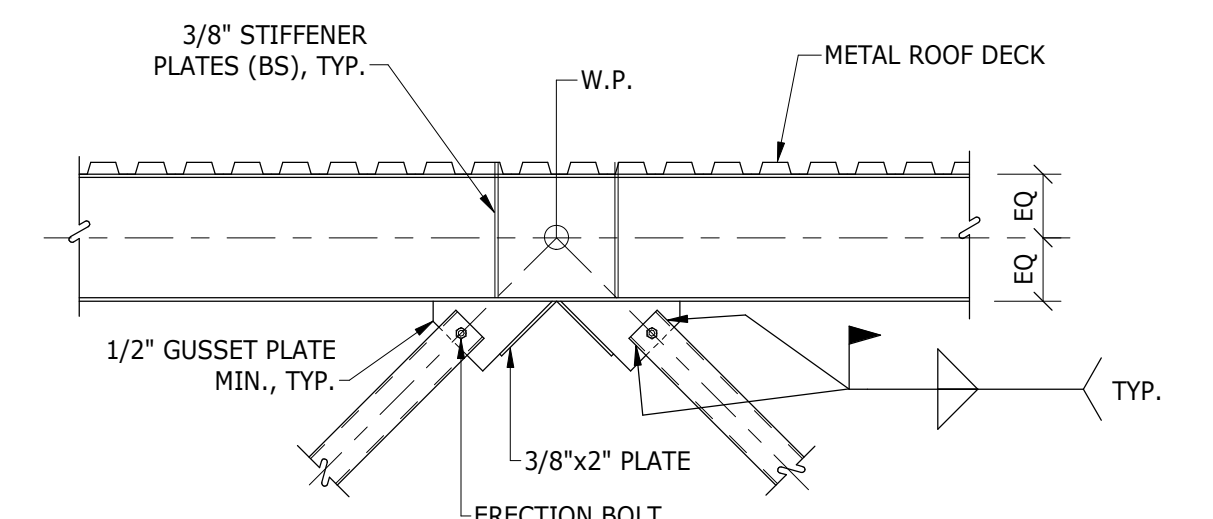


TYPICAL CONNECTION AT COLUMN/BEAM INTERSECTION

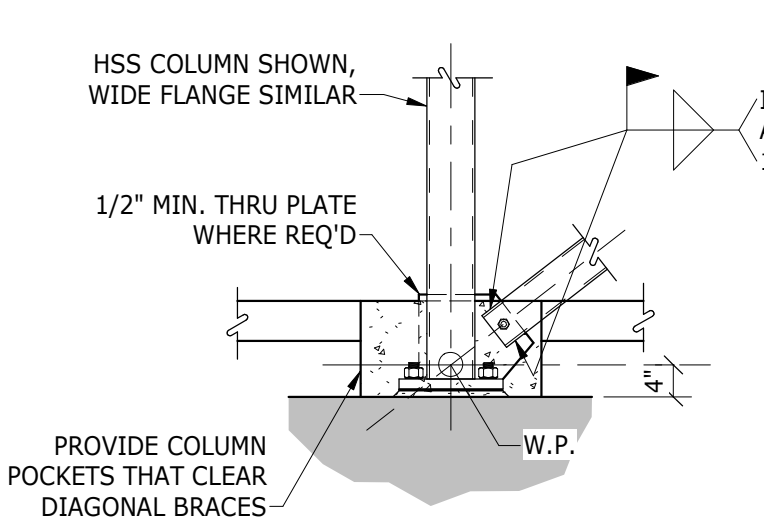
NOTE:
SHIM SLOTTED TUBES TO PROVIDE EQUAL GAPS ON BOTH SIDES OF THE GUSSET PLATE BEFORE FIELD WELDING.



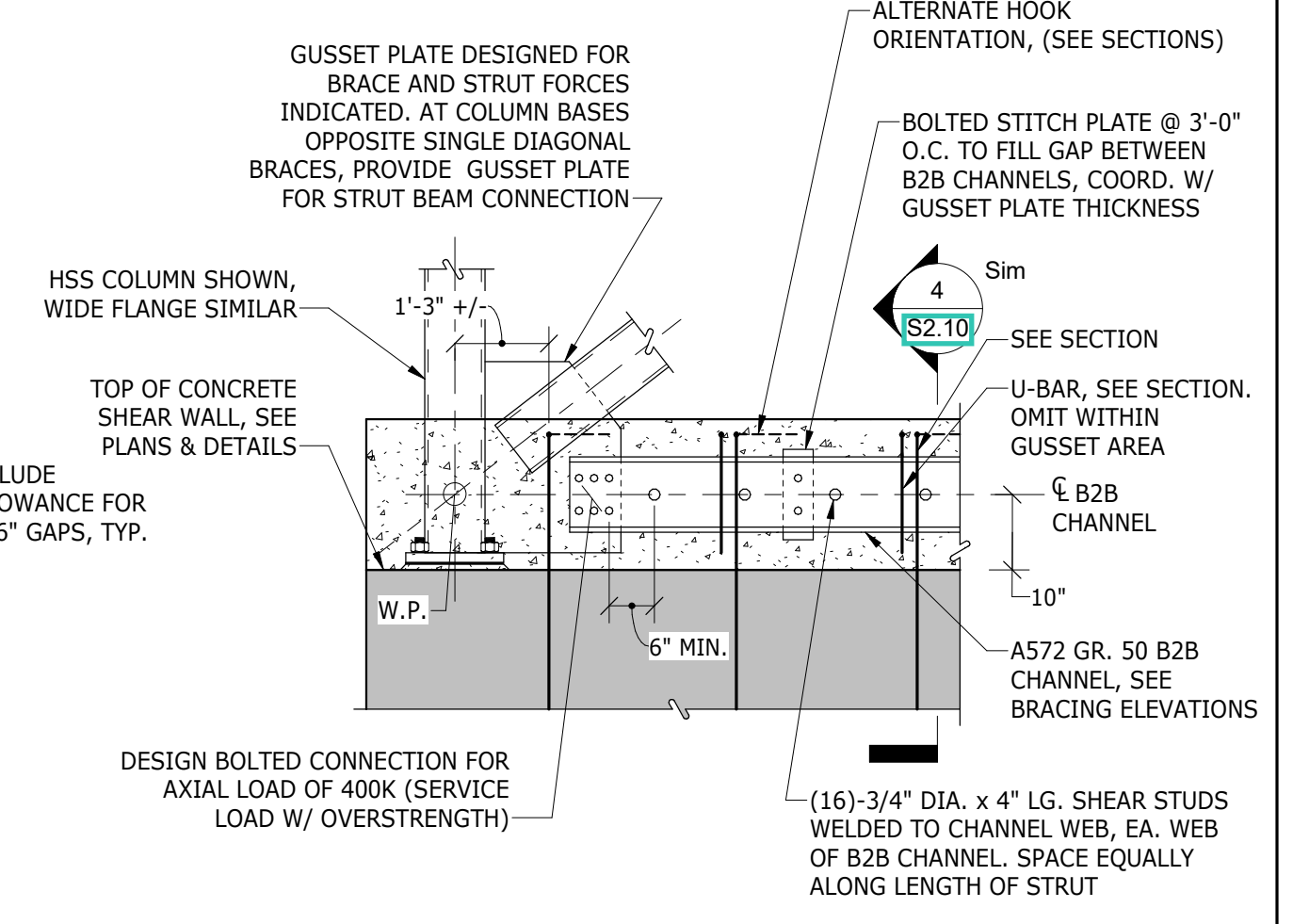
TYPICAL CONNECTION AT FLOOR BEAMS



TYPICAL CONNECTION AT ROOF BEAMS



TYPICAL CONNECTION AT COLUMN BASE



CONNECTION AT COLUMN BASE W/ STRUT BEAM

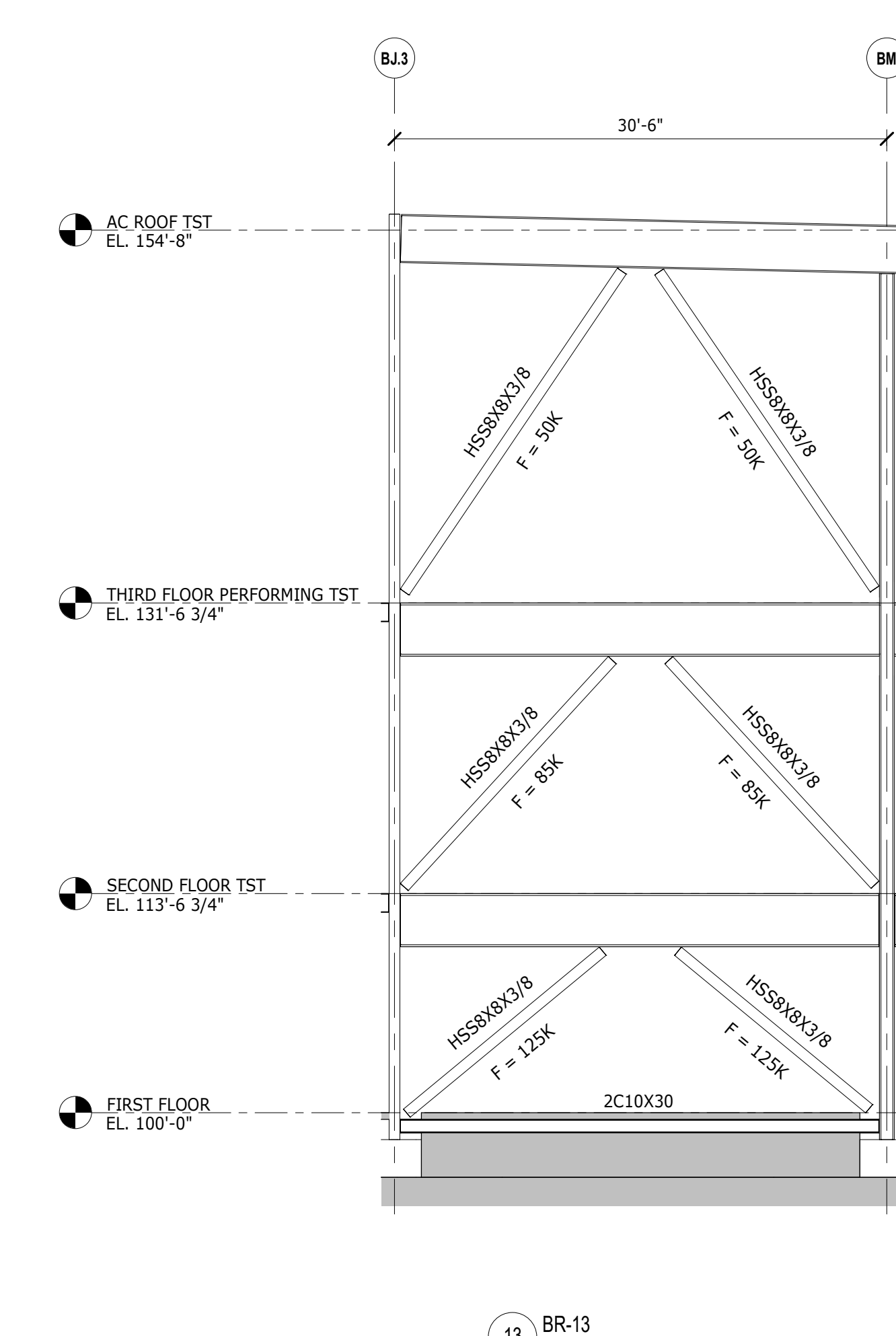
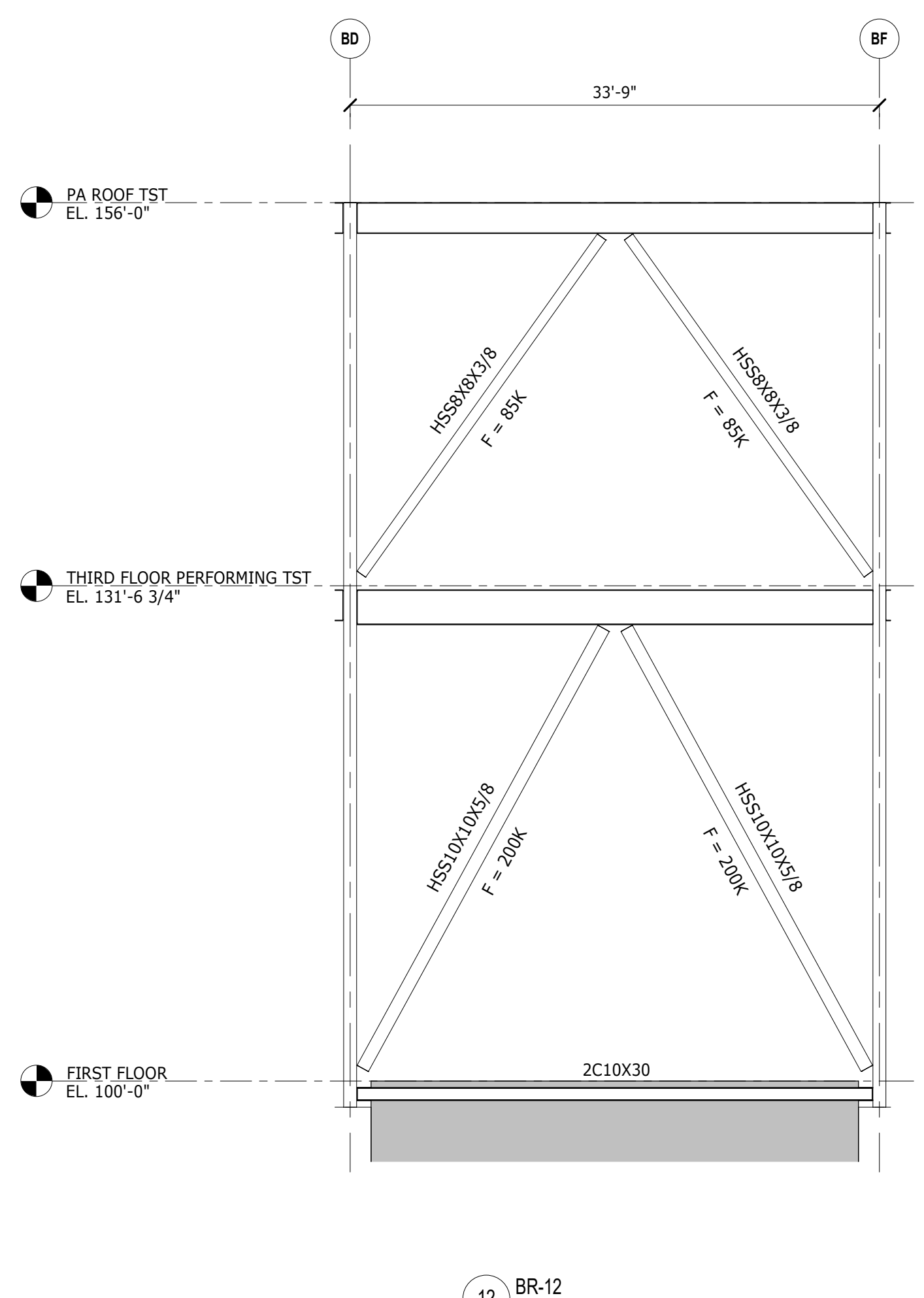
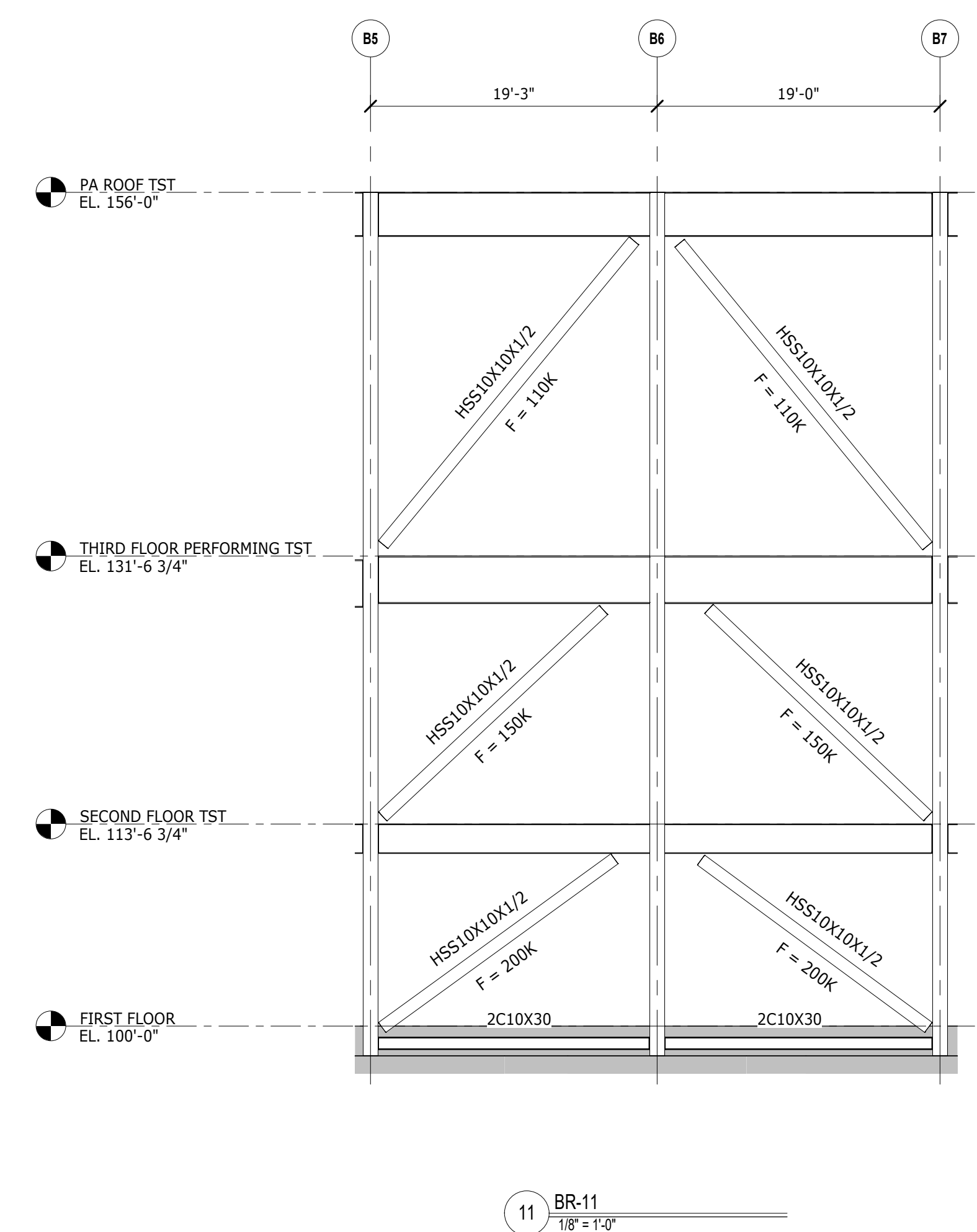
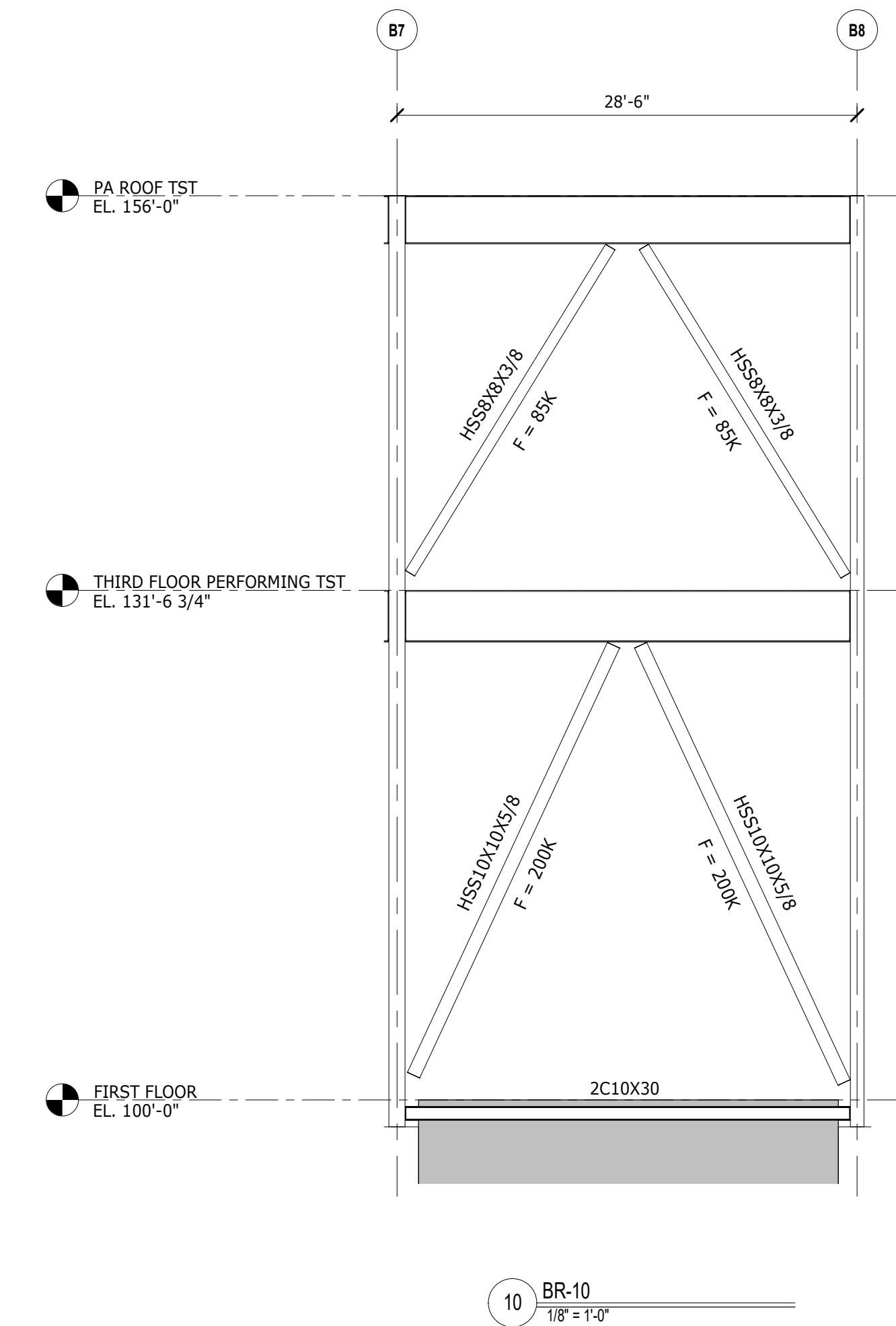
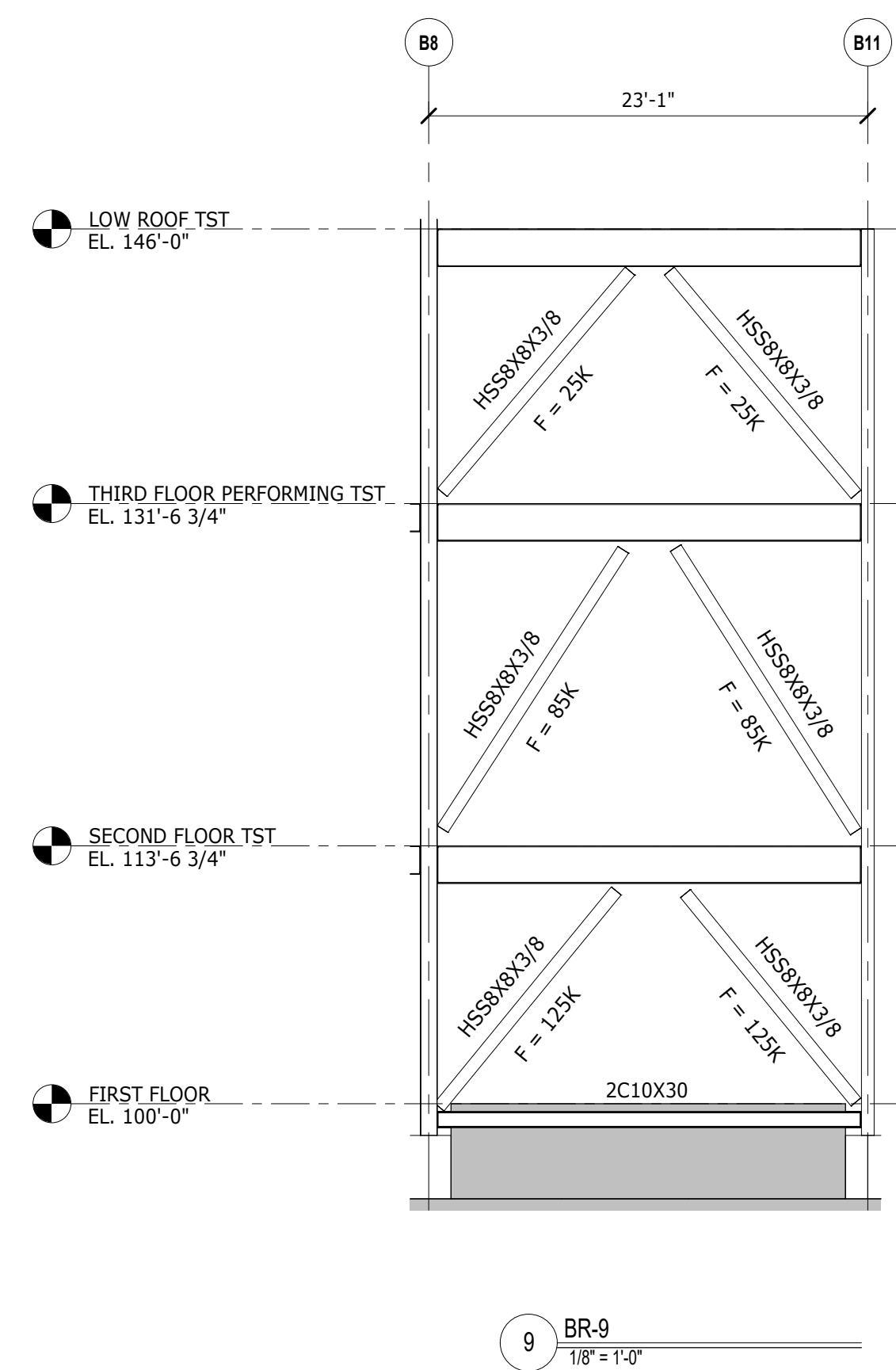
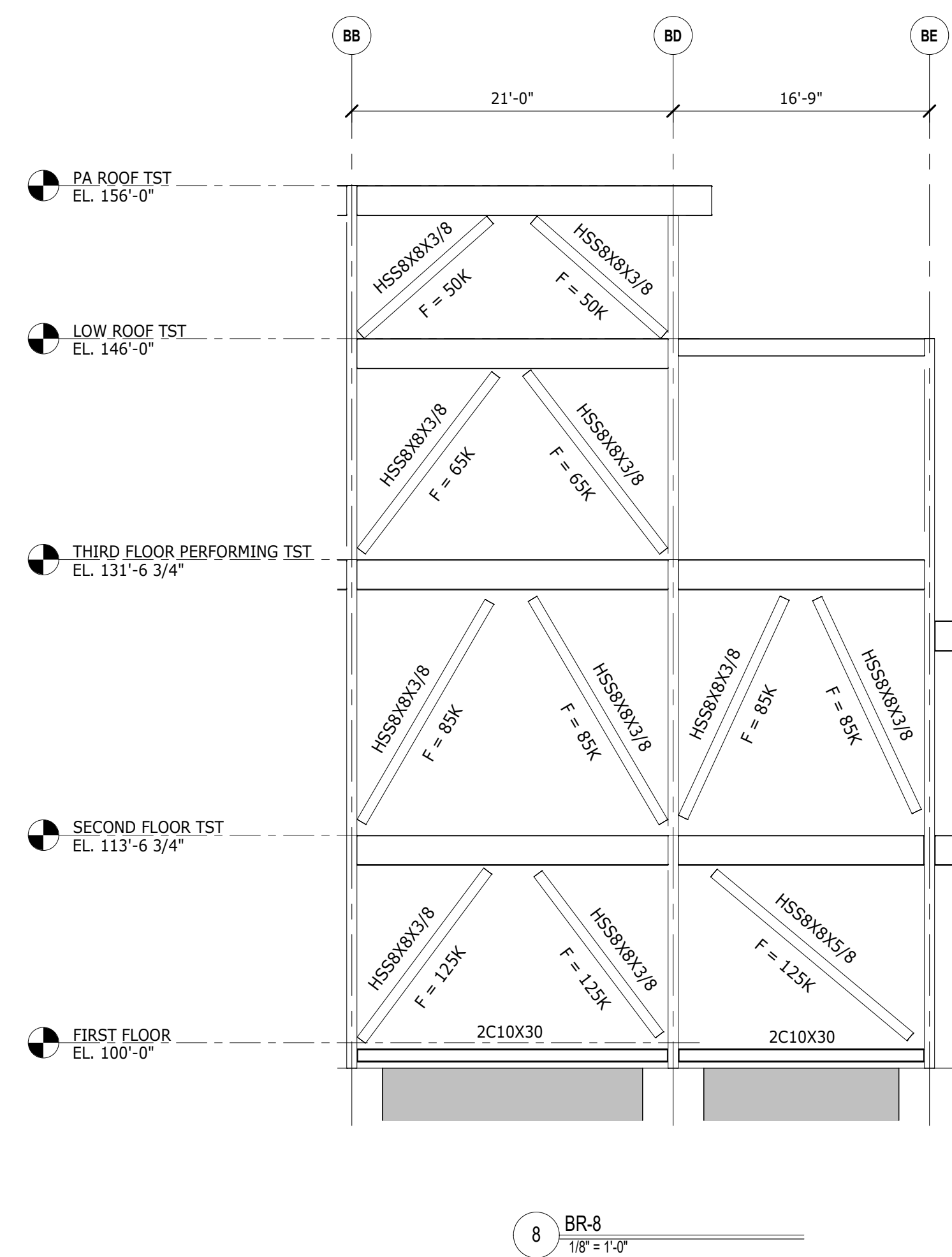
TYPICAL BRACING CONNECTION DETAILS
NOT TO SCALE

DRAWING NAME:
BRACING ELEVATIONS AND DETAILS

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023
DRAWING NUMBER:
S4.10



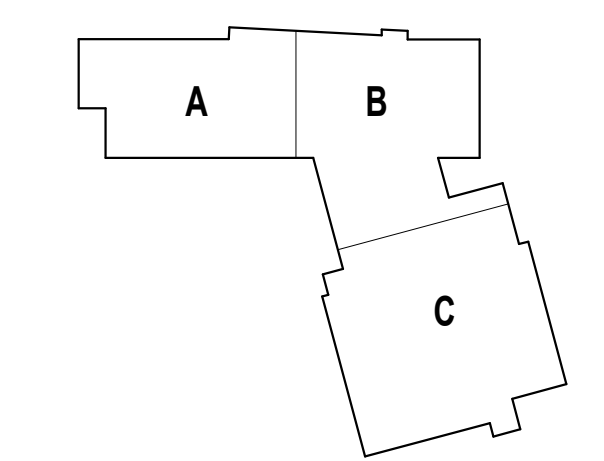
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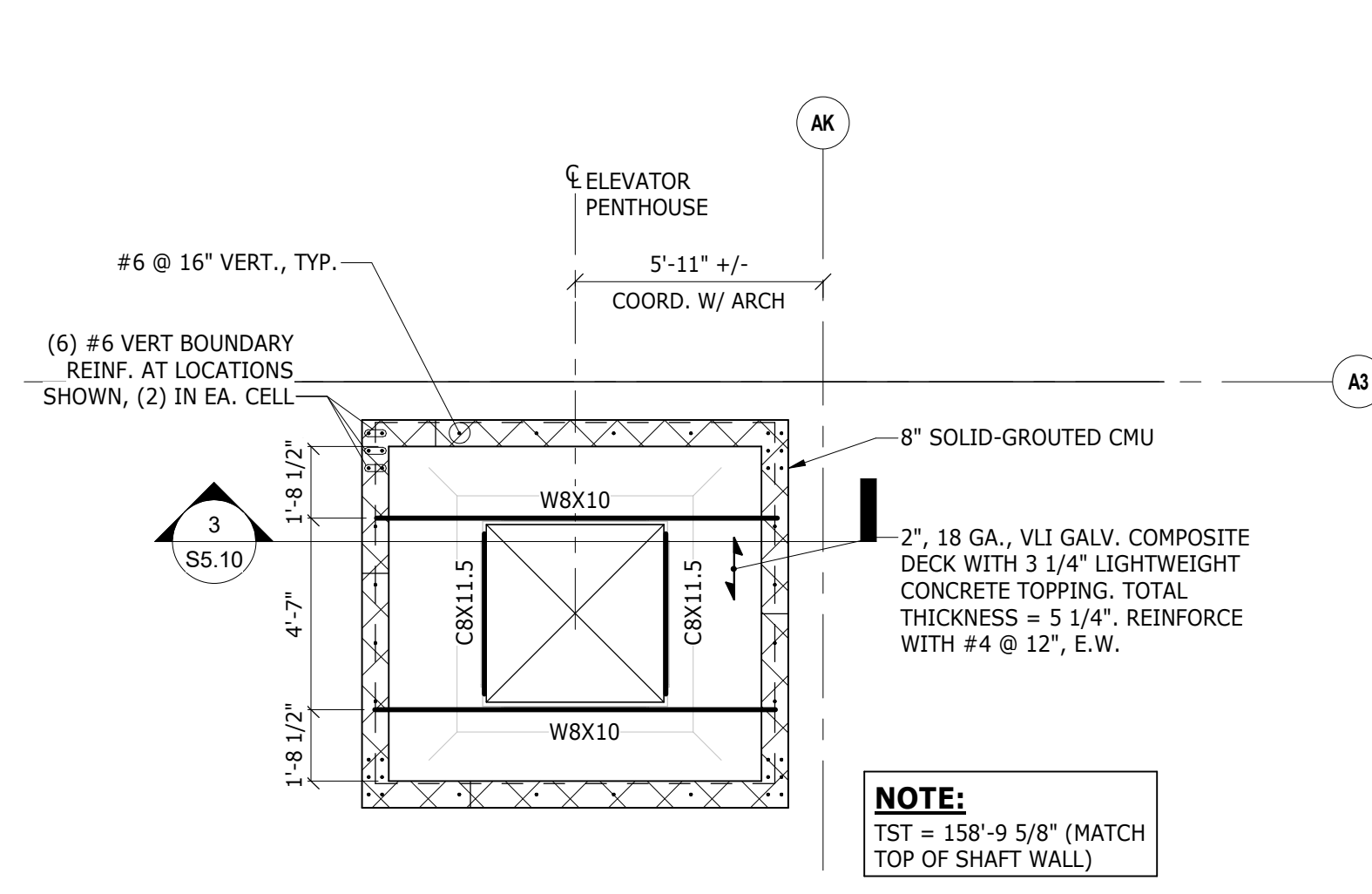
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KEY PLAN NORTH ARROW

KEY PLAN

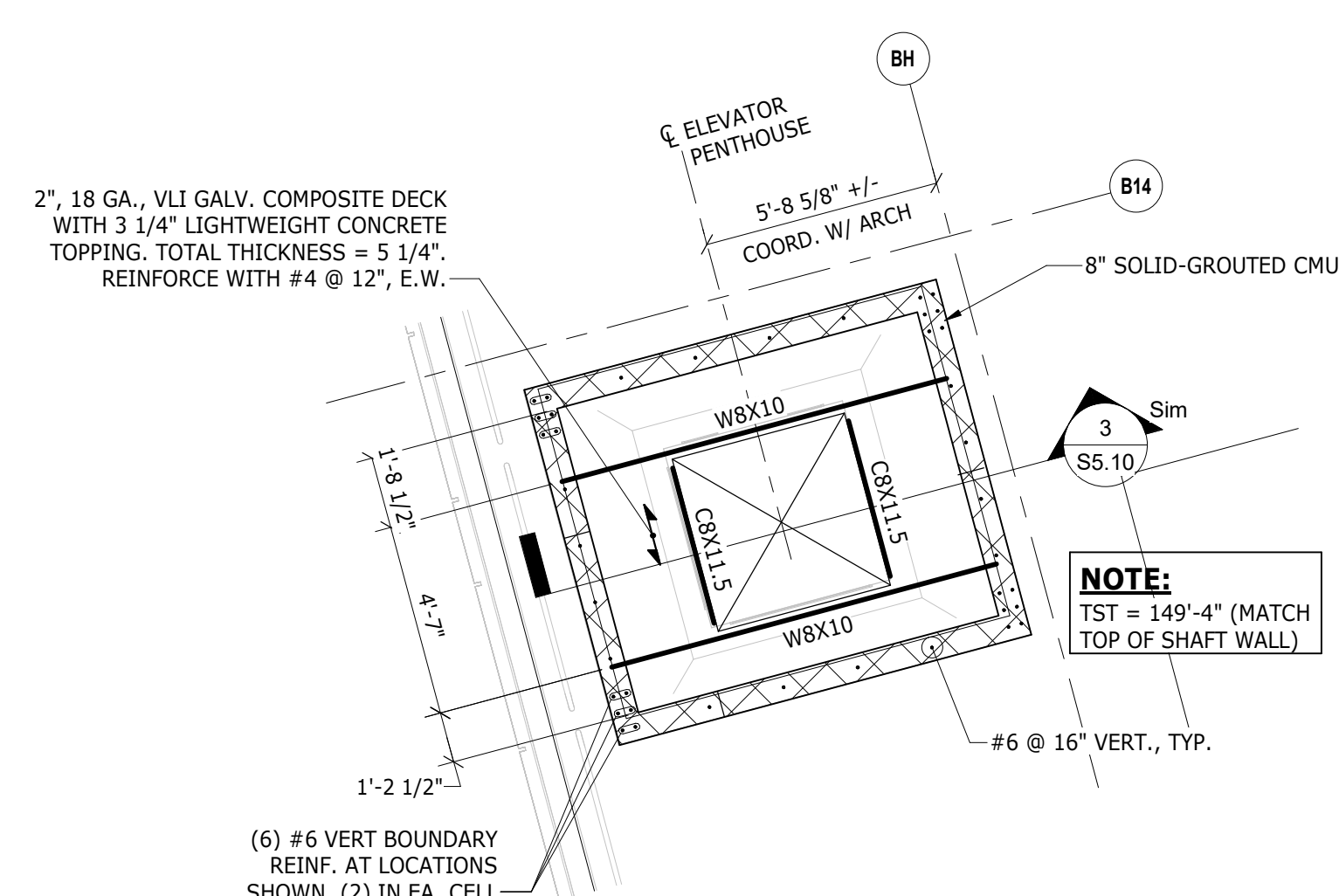


DRAWING NAME:
BRACING ELEVATIONS

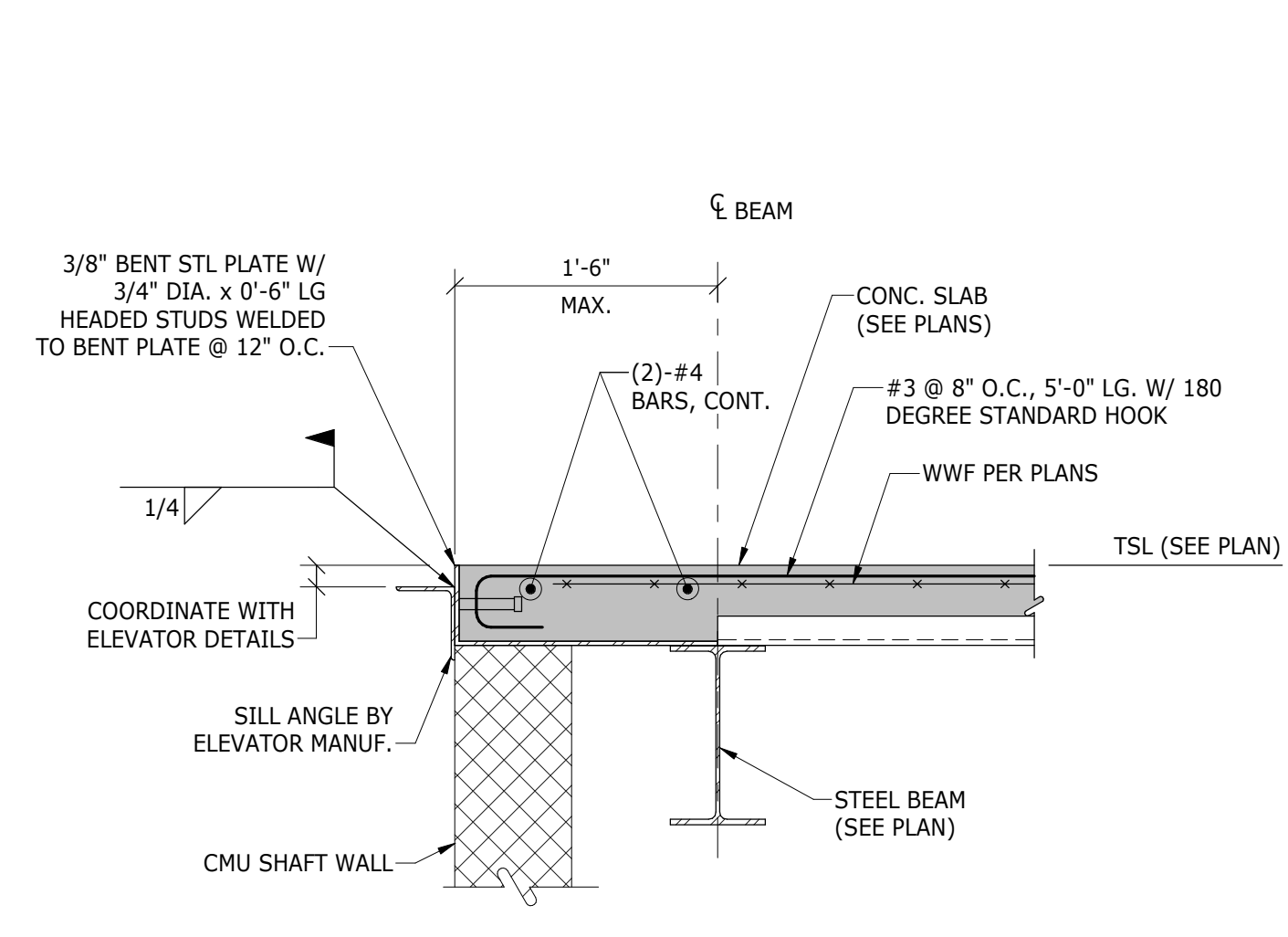
DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP
SCALE: AS INDICATED | DRAWING NUMBER:
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023 **S4.11**



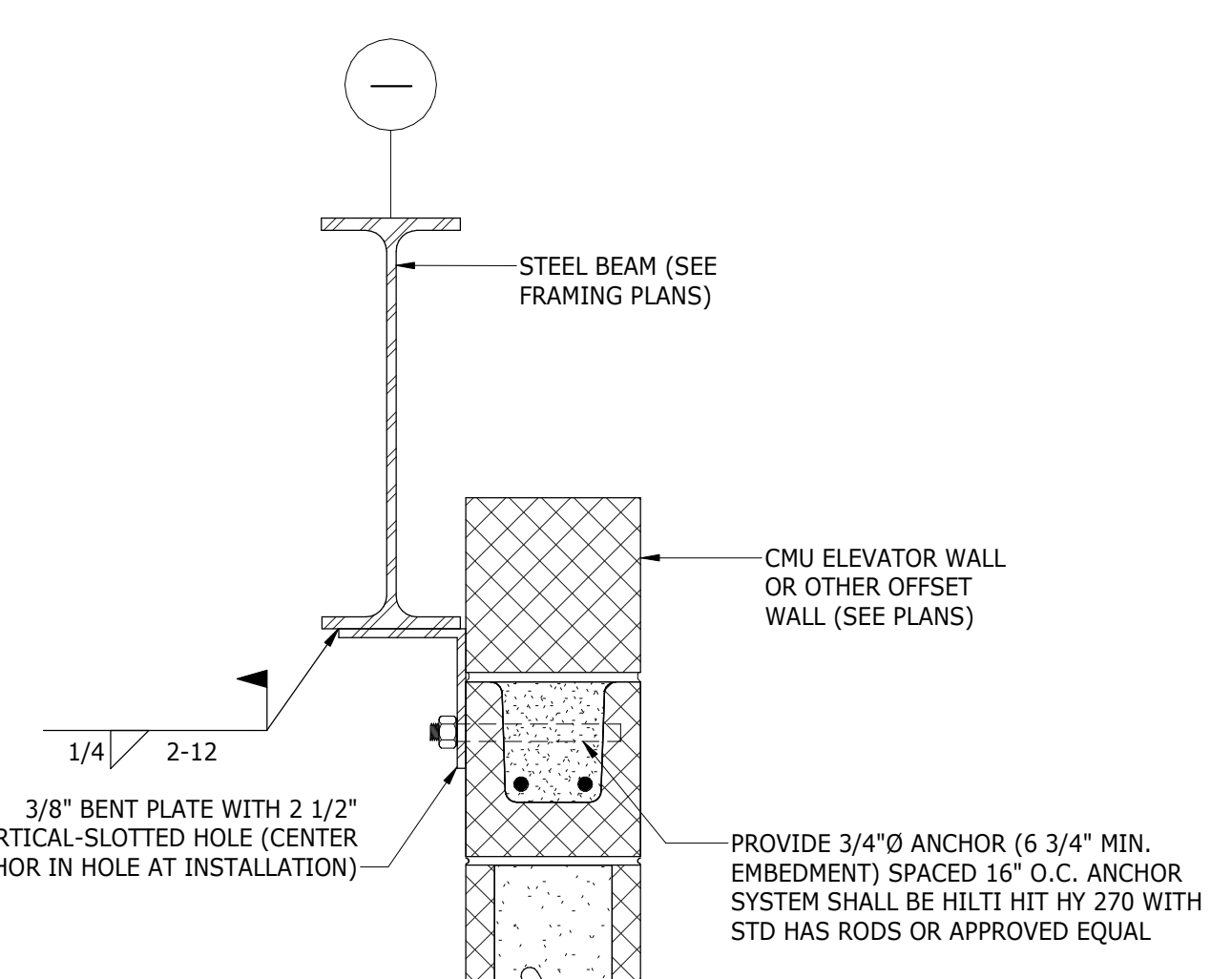
1 TOP OF ELEVATOR 1 SHAFT PARTIAL PLAN
SCALE: 1/4" = 1'-0"



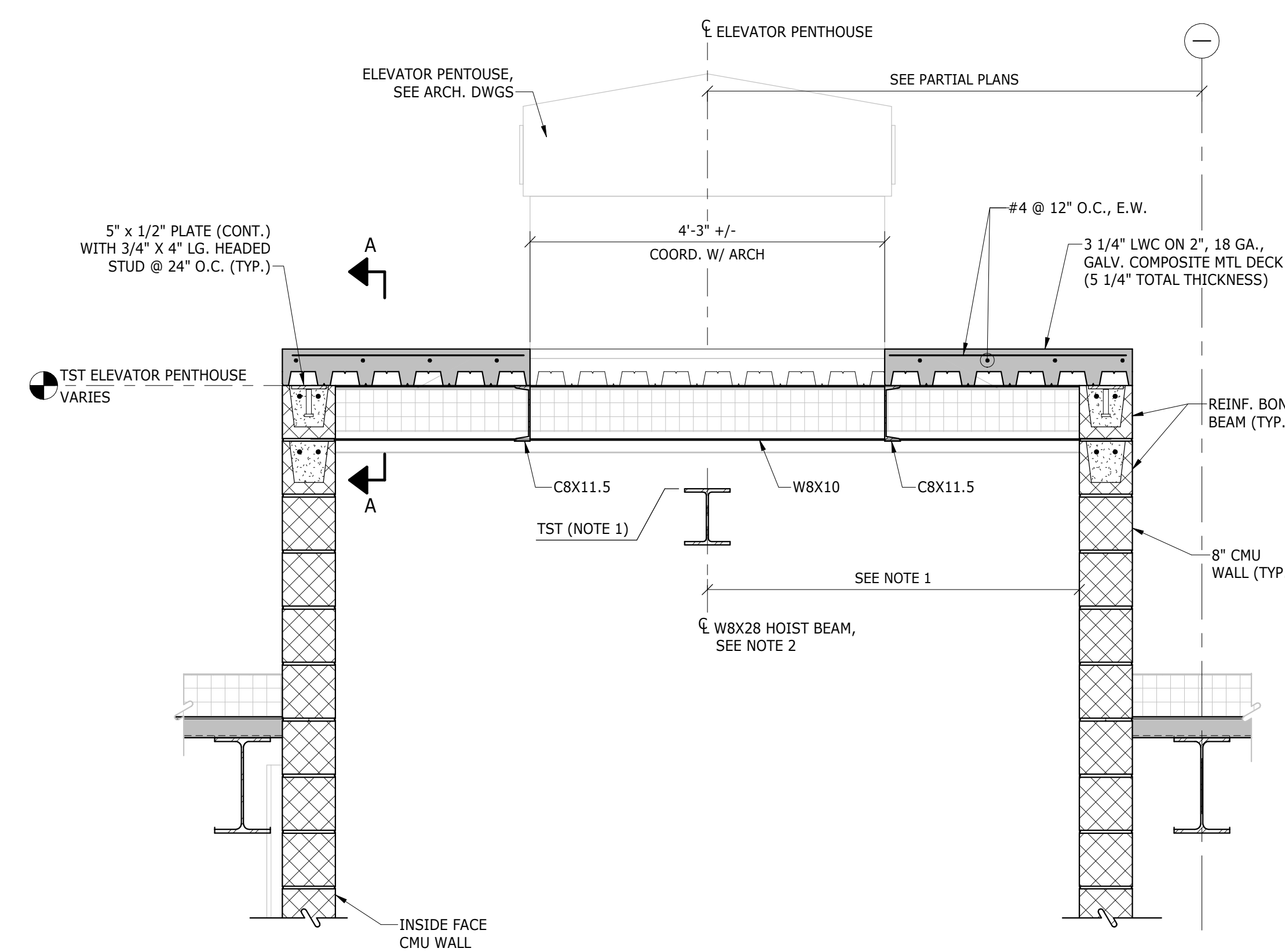
2 TOP OF ELEVATOR 2 SHAFT PARTIAL PLAN
SCALE: 1/4" = 1'-0"



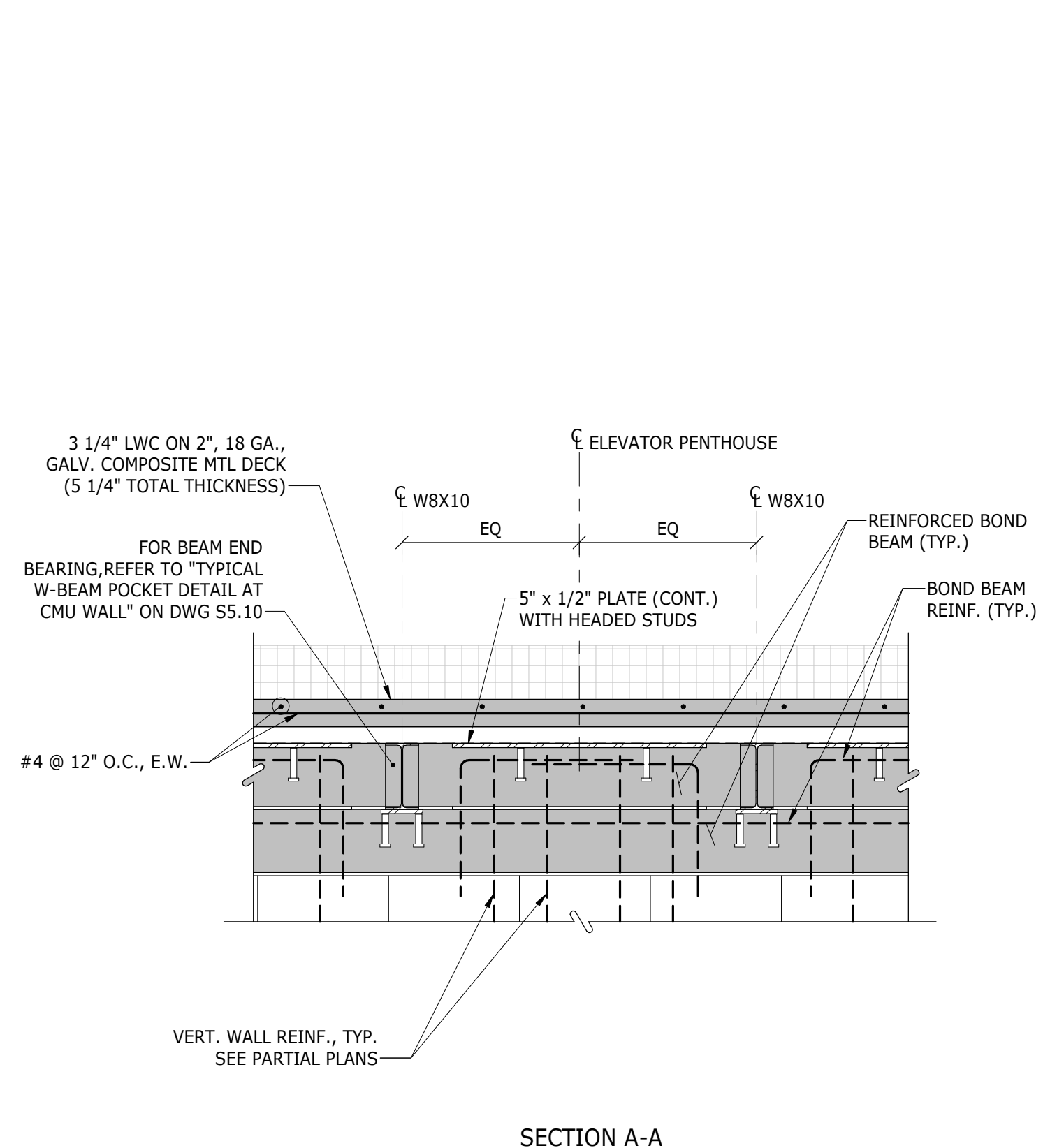
TYPICAL ELEVATOR SLAB DETAIL
NOT TO SCALE



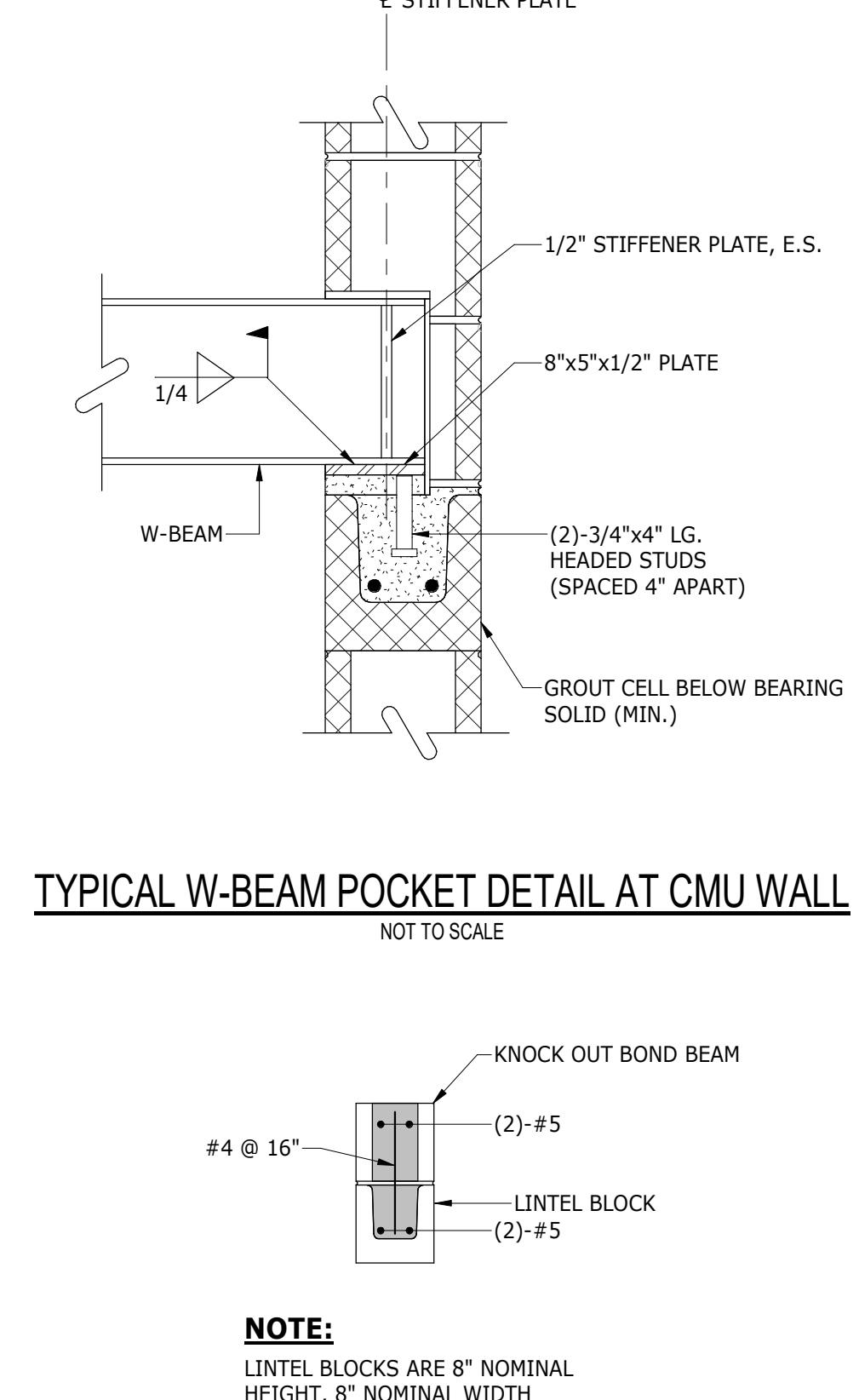
TYPICAL OFFSET ELEVATOR WALL CONNECTION DETAIL
NOT TO SCALE



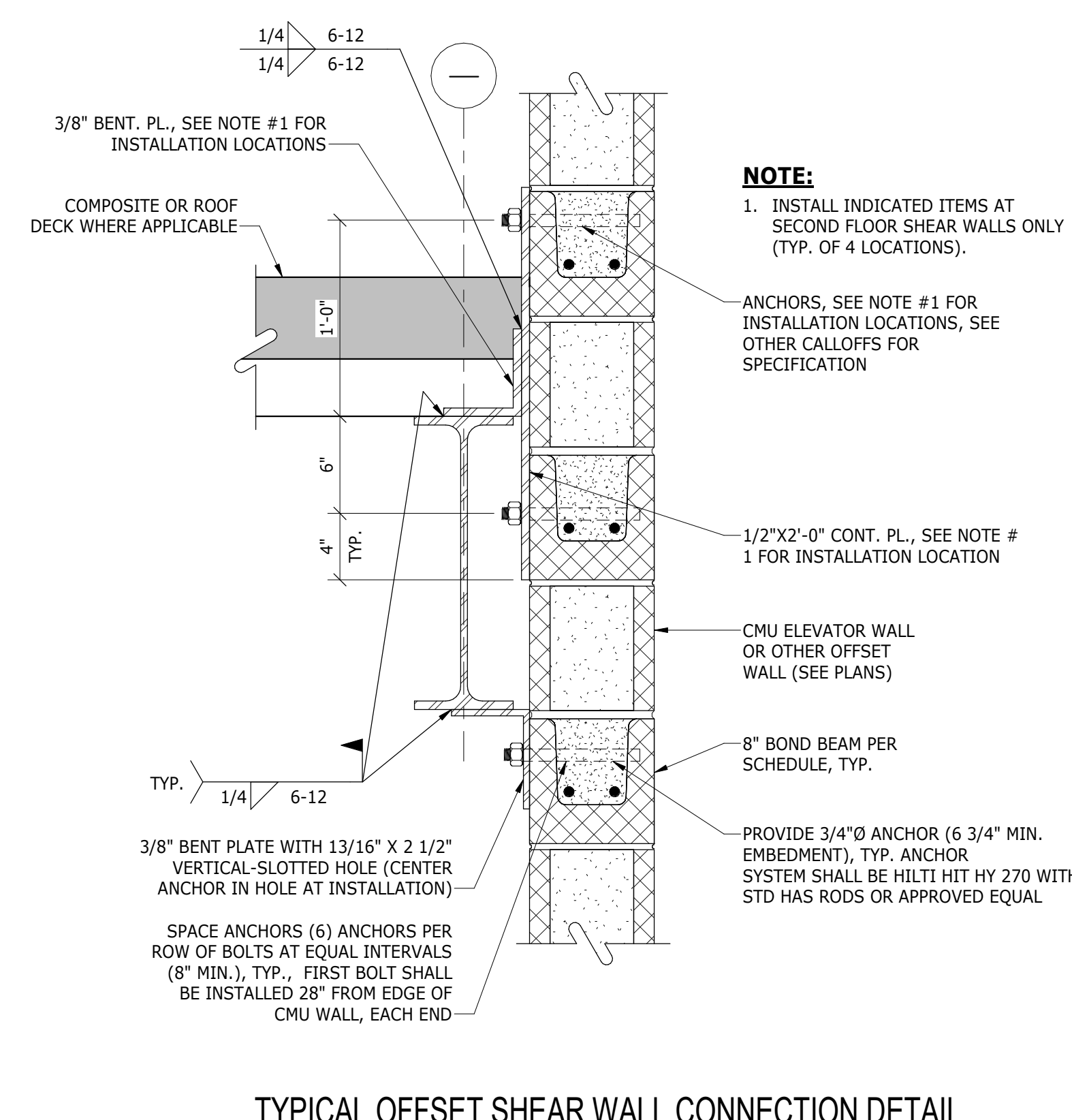
3 SECTION
SCALE: 3/4" = 1'-0"



SECTION A-A



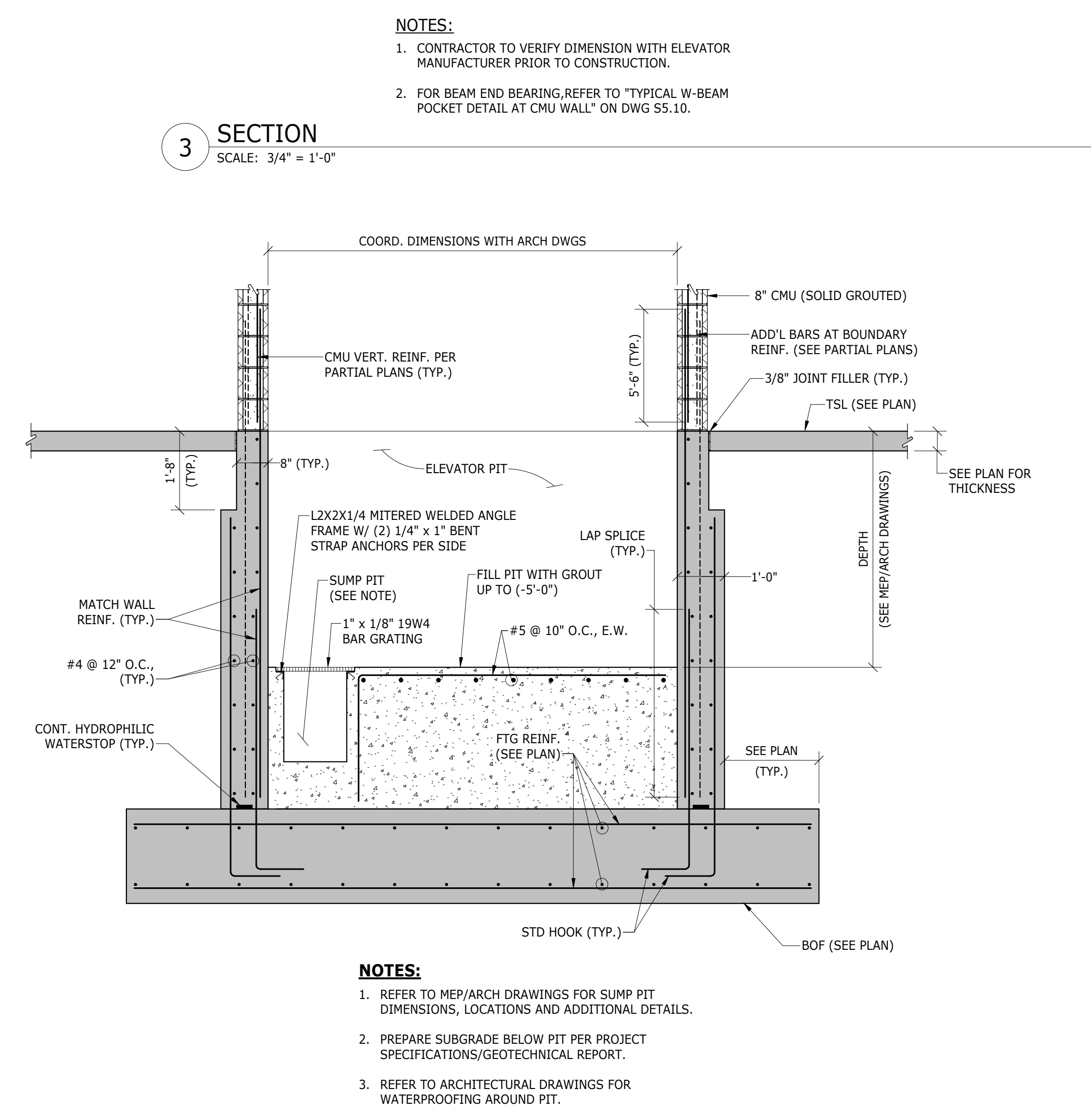
TYPICAL W-BEAM POCKET DETAIL AT CMU WALL
NOT TO SCALE



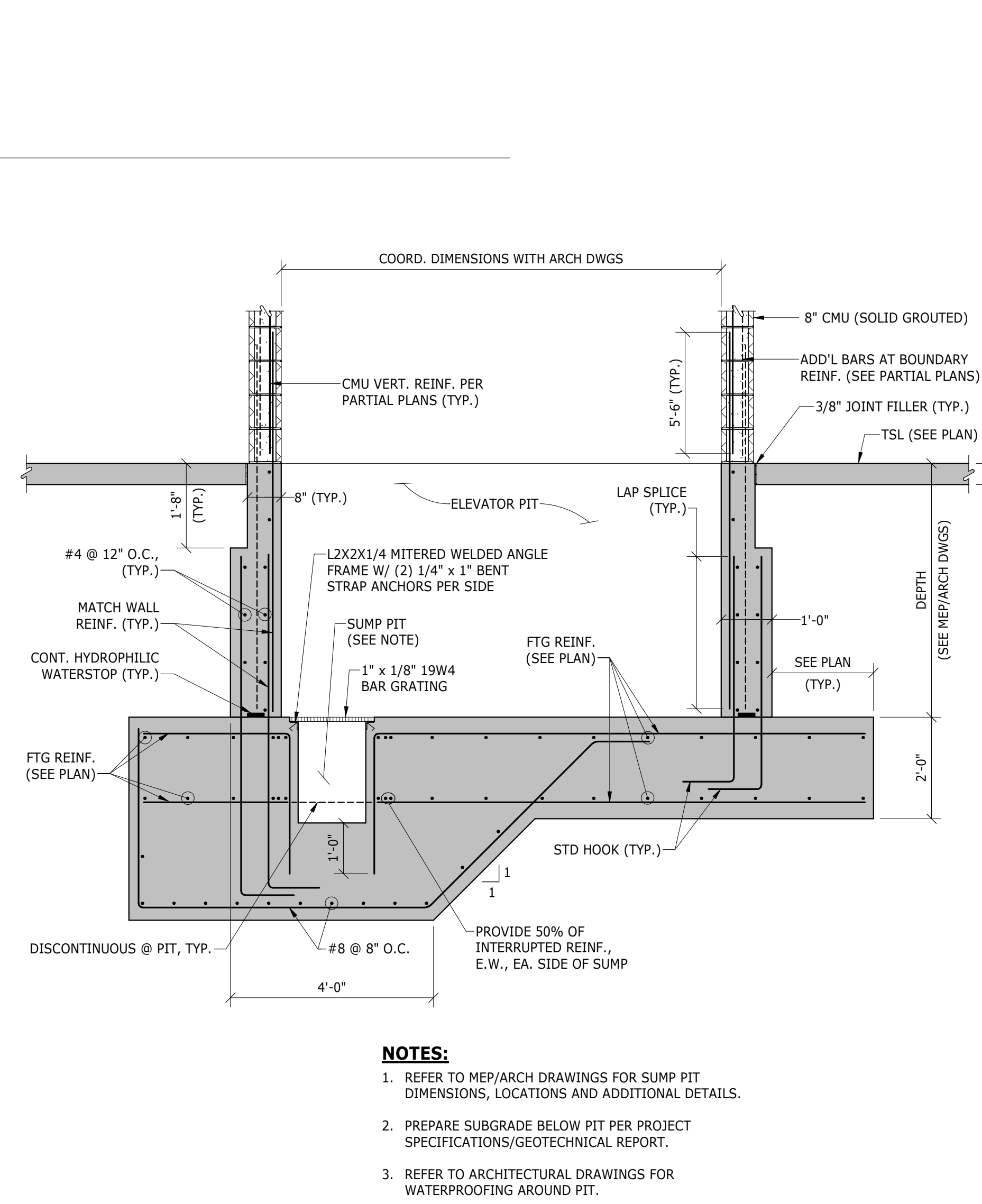
TYPICAL OFFSET SHEAR WALL CONNECTION DETAIL
NOT TO SCALE



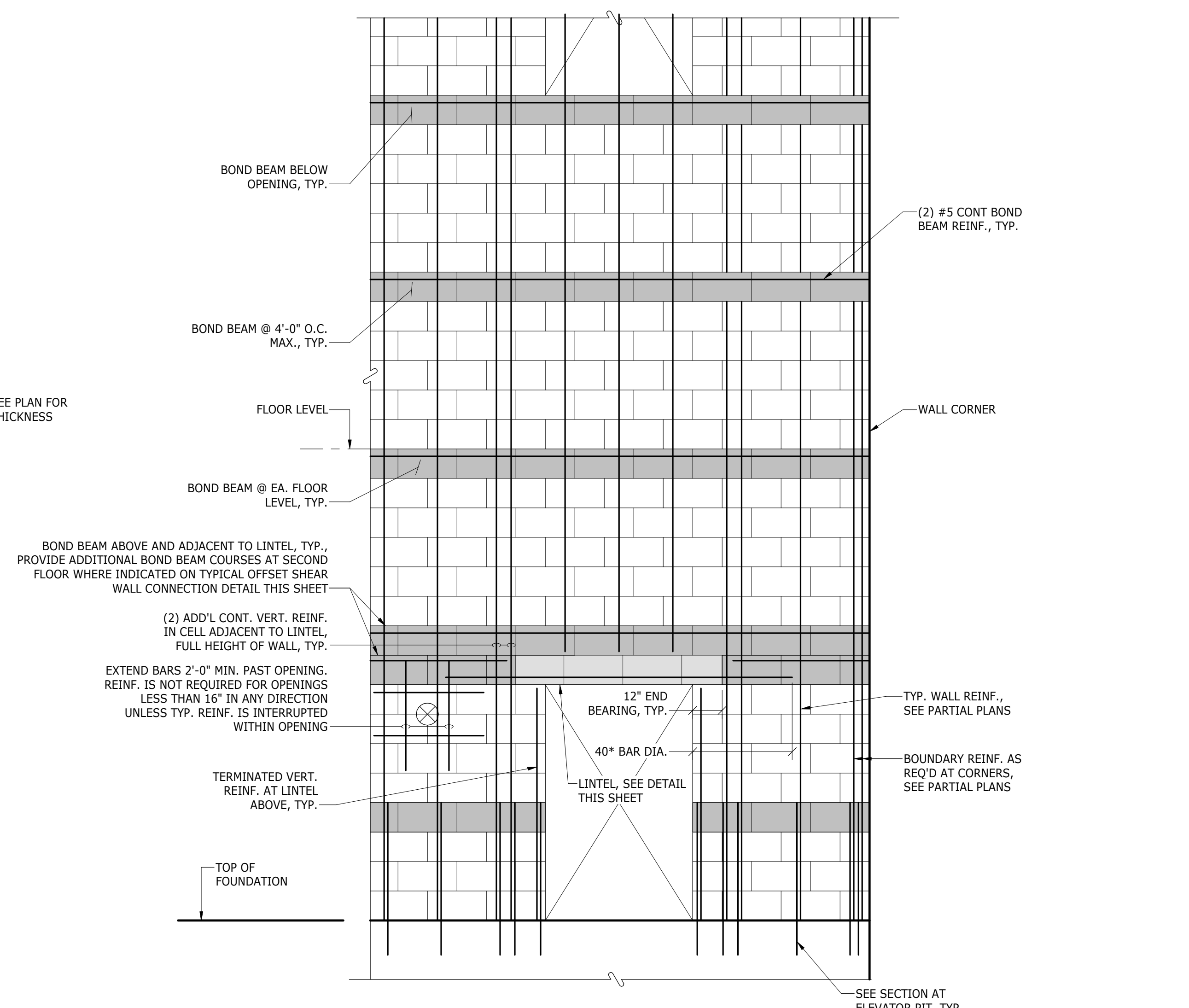
LINTEL DETAIL
NOT TO SCALE



SECTION AT ELEVATOR 1 PIT
NOT TO SCALE



SECTION AT ELEVATOR 2 PIT
NOT TO SCALE

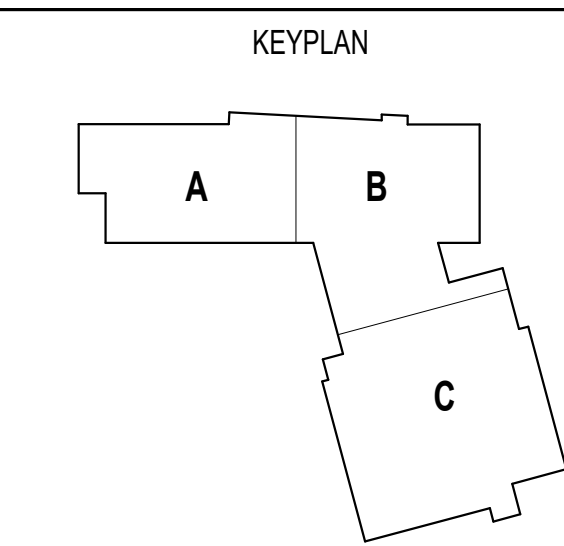
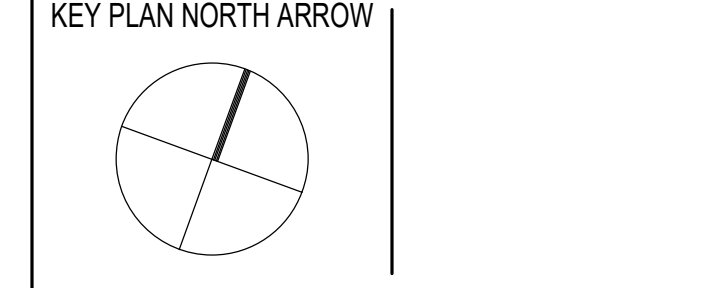


TYPICAL ELEVATOR SHAFT ELEVATION
NOT TO SCALE

- NOTES:**
- REFER TO MEP/ARCH DRAWINGS FOR SUMP PIT DIMENSIONS, LOCATIONS AND ADDITIONAL DETAILS.
 - PREPARE SUBGRADE BELOW PIT PER PROJECT SPECIFICATIONS/GEOTECHNICAL REPORT.
 - REFER TO ARCHITECTURAL DRAWINGS FOR WATERPROOFING AROUND PIT.

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100% CONSTRUCTION DOCUMENTS



DRAWING NAME:
ELEVATOR SHAFT PARTIAL PLANS AND DETAILS

DRAWN BY: JDB / MSS
REVIEWED BY: MGM / BP

SCALE: AS INDICATED | DRAWING NUMBER: S5.10
JOB NO.: 2202.02
DATE: OCTOBER 13, 2023